GENERAL STRUCTURAL NOTES:

1. THE GOVERNING BUILDING CODE IS THE INTERNATIONAL BUILDING CODE (IBC) 2009 EDITION IN ADDITION TO THE 2009 NFPA.
2. THE CONTRACTOR SHALL CROSS VERIFY ALL CONTRACT DOCUMENTS, ELVEVATIONS, DIMENSIONS, AND EXISTING CONDITIONS PRIOR TO STARTING WORK. DISCREPENCIES SHALL BE NOTED TO THE CONSULTANTS IMMEDIATELY FOR REMEDIATION.
3. THE DIMENSIONS, LOCATIONS, AND DETAILS SHOWN ON THE DRAWINGS ARE BASED ON THE BEST AVAILABLE INFORMATION AT THE TIME OF THE DRAWINGS BEING ISSUED. DEVIATIONS WHICH ARE NECESSARY OR WHICH CONFLICT SHALL BE REPORTED TO THE ENGINEER AND ARCHITECT. CONTRACTOR SHALL HAVE FULL RESPONSIBILITY FOR DEVIATIONS NOT APPROVED BY THE ENGINEER.
4. COSTS OF ADDITIONAL DESIGN WORK DUE TO THE SELECTION OF AN OPTION OR DUE TO ERRORS OR OMISSION IN CONSTRUCTION SHALL BE THE RESPONSIBILTY OF THE CONTRACTOR.
5. CONTRACT DOCUMENTS INCLUDE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR OTHER SUBMITTALS BY THE CONTRACTOR.
6. SPECIFIC NOTES AND DETAILS SHALL PRESIDE OVER GENERAL NOTES AND SPECIFICATIONS; WHERE CONFLICTS EXIST G.C. TO CONTACT ARCHITECT AND STRUCTURAL ENGINEER FOR CLARIFICATION ASAP.
7. ALL MATERIALS AND WORKMANSHIP ARE SUBJECT TO THE REVIEW OF THE ARCHITECT AND ENGINEER OF RECORD.
8. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
9. THE STRUCTURAL DRAWINGS REFLECT THE COMPLETED STRUCTURE. BRACING, SHORING, AND PROTECTION DURING CONSTRUCTION IS THE REPSONSIBLITY OF THE CONTRACTOR.
10. THE STRUCTURE SHALL NOT BE LOADED WITH CONSTRUCTION MATERIALS AND EQUIPMENT THAT EXCEEDS THE DESIGN LOADS. CONTACT ENGINEER IF POTENTIAL OVERLOAD CONDITIONS MAY OCCUR.
11. SEE MECHNICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS FOR ALL PENETRATIONS AND EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
12. PENETRATIONS NOT SHOWN ON THE DRAWINGS MUST BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE PLACING THROUGH STRUCTURAL ELEMENTS. CONTRACTOR SHALL PROVIDE A CAST-IN SLEEVE FOR ALL HORIZONTAL ELEMENTS THAT EXTEND THROUGH FOOTING AND FOUNDATION WALL, SUCH AS DRAIN TILE, CONDUIT, PIPING, ETC. COORDINATE SLEEVES WITH ENGINEER.
13. POST INSTALLED ANCHORS NOTED ON PLAN AND/OR DETAILS NOTED SHALL BE AS FOLLOWS (U.O.N.). IF ALTERNATIVE ANCHORS ARE DESIRED, CONTRACTOR MUST SUBMIT PRODUCT DATA FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO ORDERING OF MATERIALS. ANCHORS USED TO TRANSFER STRUCTURAL LOADS MUST HAVE BEEN APPROVED BY METHODS OF ACI 318 APPENDIX D FOR MECHANICAL ANCHORS AND ICC-ES AC308 FOR ADHESIVE AND TORQUE-CONTROLLED ANCHORS. ADHESIVES USED IN COLD WEATHER MUST MEET ALL REQUIREMENTS AND CODE REQUIREMENTS STATED ABOVE.

    INJECTION ADHESIVE ANCHORS: SIMPSON SET-XP ADHESIVE. THREADED RODS TO BE A193 GRADE 87 WITH EMBEDDED END CUT @ 45 DEGREE ANGLE.

    EXPANSION ANCHORS: SIMPSON STRONG BOLT 2

    SCREW ANCHORS: SIMPSON TITEN HD

    POWDER ACTUATED FASTENERS (P.A.F.): 0.145" DIA. x 3/4" STEEL-TO-STEEL 0.145" DIA. x 1 1/4" STEEL TO CONCRETE & MASONRY. 3" MIN. CONCRETE EDGE DISTANCE.
14. ALL ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW OR RECORD SHALL BEAR THE STAMP AND SIGNATURE OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.
15. POST INSTALLED ANCHORSSHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED C.I.P. ANCHORS. CARE SHALL BE TAKEN TO AVOID CONFLICTS WITH EXISTING REINFORCING BARS. HOLES SHALL BE DRILLED AND CLEANED PER ANCHOR MANUFACTURER'S SPECIFICATIONS. ANCHORS AND ADHESIVE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
16. PERIODIC SITE OBSERVATION BY REPRESENTATIVES OF SANDMAN CONSULTING, PC IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH TH ESTRUCTURAL CONTRACT DRAWINGS. THIS LIMITED SITE OBSERVATIONSHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.
17. DEFERRED SUBMITTALS: ALL ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW OR RECORD SHALL BEAR THE STAMP AND SIGNATURE OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.

MASONRY NOTES:

1. MASONRY CONSTRUCTION SHALL CONFORM TO AMERICAN CONCRETE INSTITUTION (ACI) CODES AND SPECIFICATIONS

    ACI 530: "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
    ACI 530.1: "SPECIFICATION FOR MASONRY STRUCTURES"
2. PROVIDE CONCRETE MASONRY UNITS (CMU) OF NORMAL WEIGHT, GRADE N, TYPE I OR II, CONFORMING TO ASTM C90. CMU SHALL HAVE A MINIMUM PRISM COMPRESSIVE STRENGTH OF, f'm=1500 PSI. TESTED IN ACCORDANCE WITH ASTM C140.
3. MORTAR SHALL CONFORM TO ASTM C270. USE TYPE "M" BELOW GRADE AND TYPE "S" ABOVE GRADE.
4. MASONRY GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS.
5. ALL BOND BEAMS SHALL HAVE (2) #5 BARS CONT. W/ CORNER BARS. PLACE BOND BEAMS WHERE SHOWN ON DETAILS AND AT TOP OF WALL. CONTINUE BOND BEAMS THROUGH CONTROL JOINTS.
6. PROVIDE FULL MORTAR BED AT FACE SHELLS AND CROSS WEBS AT GROUTED CORE LOCATIONS. PROVIDE FULL MORTAR BED AT BASE OR BOTTOM WALL COURSE.
7. ALIGN VERTICAL CELLS TO BE FILLED WITH GROUT TO PROVIDE CONT. UNOBSTRUCTED VERTICAL CELLS. REMOVE OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS. ALL MASONRY SHALL BE LAID IN A RUNNING BOND.
8. PROVIDE VERTICAL CONTROL JOINTS MEETING THE FOLLOWING CRITERIA (SEE ARCH DRAWINGS FOR CONTROL LOCATIONS):

    MAX SPACING NOT TO EXCEED 25'-0"
    MAXIUM OF 8'-0" FROM BUILDING CORNERS
    NO CLOSER THAN 2-0" TO OPENING EDGES
    NO CLOSER THAN 1'-4" TO BEAM OR JOIST BEARING LOCATION

    *DEVIATION FROM THESE GUIDELINES REQUIRES APPROVAL FROM ENGINEER AND ARCHITECT.
9. VERTICAL REBAR SHALL BE LATERALLY SUPPORTED IN PROPER POSITION AT TOP AND BOTTOM OF BAR DURING GROUTING OF CELLS WITH WIRE POSITIONERS. SEE ACI SPECIFICATIONS FOR PLACEMENT TOLERANCES.
10. ALL VERTICAL REINFORCMENT, INCLUDING JAMB REINFORCMENT, SHALL BE DOWELED INTO FOUNDATION WALLS OR FOOTINGS.
11. IT IS THE CONTRACTORS RESPONSIBILITY TO BRACE WALLS UNTIL FLOOR OR ROOF CONNECTIONS ARE COMPLETED.
12. MASONRY REINFORCING LAP LENGTHS:

BAR	LAP LENGTH
#3	18"
#4	24"
#5	30"
#6	36"

13. HORIZONTAL JOINT REINFORCING SHALL BE GALVANIZED 9 GA. WITH MAX SPACING OF 16" O.C. VERTICALLY; LAP 8" MIN. ALL CORNERS AND WALL INTERSECTIONS TO BE PREFABRICATED PIECES. JOINT REINFORCMENT SHALL BE FULLY EMBEDDED IN MORTAR. TERMINATE JOINT REINFORCEMENT AT CONTROL JOINTS, U.O.N.
14. PROVIDE (1) #5 EA SIDE OF CONTROL JOINT AND AT END OF WALL SEGMENTS.
15. SHOP DRAWING SUBMITTAL FOR WALL REINF. SHALL SHOW ALL WALL ELEVATIONS W/ REINF, CONTROL JOINTS, AND ALL WALL REINF. RELATED DETAILS.
16. MAX GROUT LIFT IS 5'-0"; UNLESS ALT. ACI METHOD APPROVED BY ENGINEER GROUT SHALL BE CONSOLIDATED BY MECHNICAL MEANS. PLACE 2" COLD JOINT GROUT DEPRESSION AT TOP OF EACH INTERMEDIATE GROUT LIFT.
17. DO NOT PLACE ANY PIPES OR OTHER PENETRATIONS THROUGH MASONRY WITHOUT THE PRIOR APPROVAL THE ENGINEER.

CONCRETE AND STEEL REINFORCMENT NOTES:

1. CONCRETE CONSTRUCTION SHALL CONFORM TO AMERICAN CONCRETE INSTITUTION (ACI) CODES AND SPECIFICATIONS, LATEST EDITION.

ACI 301	"SPECIFICATIONS FOR STRUCTURAL CONCRETE"
ACI 315	"DETAILS & DETAILING OF CONCRETE REINFORCEMENT"
ACI 318	"BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
ACI 306R	"COLD WEATHER CONCRETING"

2. CAST-IN-PLACE CONCRETE STRENGTHS REQUIRED (28 DAY)

FOOTINGS	3000 PSI (AIR ENTRAINED)
EXTERIOR FDN WALLS	3000 PSI (AIR ENTRAINED)
INTERIOR FDN WALLS	3000 PSI
PIERS	3000 PSI
INTERIOR SLABS & TOPPINGS	3000 PSI
EXTERIOR SLABS	4000 PSI (AIR ENTRAINED)

3. CONCRETE MIX DESIGN SHALL BE BY AN INDEPENDENT TESTING LABORATORY.
4. CAST-IN-PLACE CONCRETE SHALL BE SUBJECT TO TESTING BY AN INDEPENDENT TESTING LABORATORY, SEE SPECS AND SPECIAL INSPECTIONS.
5. ALL CONCRETE SHALL BE THOROUGHLY CONSOLIDATED BY MEANS OF A VIBRATOR, ESPECIALLY AROUND REINFORCMENT STEEL AND CORNERS OF FORMWORK.
6. REINFORCING STEEL SHALL BE GRADE 60 DEFORMED, BILLET-STEEL, ASTM A615, U.O.N. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A82 AND A185.
7. WELDED REINFORCING STEEL SHALL BE GRADE 60, LOW CARBON, ASTM A706, WHICH IS SPECIALLY MANUFACTORED TO BE WELDABLE.
8. WELDED WIRE FABRIC SHALL BE PLACED IN THE CENTER OF THE SLAB, U.O.N. LAP JOINTS A MINIMUM OF 6". EXTEND FABRIC TO BE WITHIN 1" OF SLAB EDGES.
9. PROVIDE ADEQUATE BOLSTERS, HIGH CHAIRS, SUPPORT BARS, ETC TO MAINTAIN THE SPECIFIED CLEARANCES FOR THE ENTIRE LENGTH OF ALL REINFORCING STEEL AND WELDED WIRE FABRIC.
10. PROVIDE EXTRA REINFORCEMENT AROUND ALL OPENINGS GREATER THAN 8" SQUARE OR ROUND. PROVIDE (2) #5 BARS @ 3" O.C. FOR EACH MAT OF BARS, AT EACH SIDE AND CORNER OF OPENING EXTENDING MINIMUM 18" PAST CORNER OF THE OPENING. PLACE 2" CLR FROM OPENING.
11. SEE DETAILS FOR REINFORCING LAP SPLICE SCHEDULE, UNLESS OTHERWISE NOTED ON PLAN OR DETAILS.
12. CAST DOWELS, WITH STD 90 DEG HOOK, IN FOOTINGS FOR CONCRETE PIERS AND WALLS ABOVE. DOWELS SHALL BE THE SAME SIZE AND QTY AS THE VERTICAL REINFORCING. (U.0.N.)
13. SUPPLY 100 FEET EXTRA OF #5 REBAR FOR MISC. PLACEMENT AS DIRECTED BY THE ENGINEER. CONTRACTOR SHALL INCLUDE LABOR ALLOWANCE FOR PLACEMENT.
14. EXTERIOR SLABS SHALL DRAIN FREELY AWAY FROM THE BUILDING. SEE CIVIL AND ARCH. DRAWINGS FOR ELEVATIONS.
15. CONTROL SAWCUT JOINTS ARE TO BE EXECUTED AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT AGGREGATE FOR MDISLODGING BY SAW AND PRIOR TO SHRINKAGE STRESS CRACKING. CONTROL JOINTS TO BE SPACED A MAX. OF 10 FOOT SQUARES U.O.N. ON PLAN. A JOINT SHALL BE CUT T OEVERY CORNER OF COLUMN ISOLATION JOINTS.

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED PER AMERICAN NSTITUTE OF STEEL CONSTRUCTION (AISC) REQUIREMENTS (AISC 360-05, 13TH ED.)
2. MATERIAL QUALIFICATIONS, U.N.O. ON DRAWINGS:

SHAPE	Fy	Fu	ASTM STD	NOTES
W,S, & M SHAPES	50 KSI	---	A992	---
STANDARD PIPES	35 KSI	---	A53 GRADE B	---
HSS ROUND	42 KSI	---	A500 GRADE B	---
HSS SQ. & RECTANGULAR	46 KSI	---	A500 GRADE B	---
C, MC, PLATE & ANGLES	36 KSI	---	A36	---
ANCHOR RODS *	36 KSI	---	F1554	A563 HEAVY HEX NUT
THREADED RODS **	---	125 KSI	A193 GRADE B7	A563 HEAVY HEX NUT
SHEAR STUD CONNECTORS	---	65 KSI	A108	---
HIGH STRENGTH BOLTS	---	120 KSI	A325	A563 HEAVY HEX NUT
WASHERS	---	---	F436	---
* STANDARD FOR ANCHOR RODS, U.N.O. IF GRADE 55 IS REQUIRED, IT MUST HAVE S1 CERTIFICATION FOR BEING WELDABLE. GRADE 105 CANNOT BE WELDED.				
** THREADED ROD APPLICATIONS INCLUDE POST-INSTALLED ADHESIVE ANCHORS				

3. WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (AWS) AND AISC. ELECTRODES TO BE E70XX.
4. UNSPECIFIED WELDS SHALL BE CONTINUOUS FILLETS WITH A MINIMUM SIZE PER MATERIAL THICKNESS PER AISC MANUAL.
5. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS EXPERIENCED IN THE TYPE CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.
6. STRUCTURAL STEEL SHOP DRAWINGS SHALL SHOW ALL FIELD AND SHOP WELDING REQUIREMENTS USING AWS SYMBOLS.
7. STRUCTURAL STEEL SHOP DRAWINGS SHALL SHOW ALL FIELD ERECTION DETAILS FOR THE CONTRACTOR TO ERECT THE STRUCTURE ACCORDING TO THE CONTRACT DRAWINGS.
8. COVER STEEL SURFACES WITH ASPHALTIC BASED ROOFING COMPOUND PRIOR TO CASTING CONCRETE AROUND COLUMN BASES. ALL STEEL MEMBERS SHALL RECEIVE A PRIME COAT OF PAINT EXCEPT STEEL SCHEDULED TO BE FIREPROOFED.
9. BOLTED CONNECTIONS SHALL BE MADE USING 3/4" DIA. A325 BOLTS AND WASHERS. BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED. ALL HOLES MUST BE DRILLED OR PUNCHED. FLAME CUTTING OF HOLES IS NOT PERMITTED.
10. FIELD NOTCHING OR COPING OF STRUCTURAL STEEL MEMBERS IS NOT ALLOWED WITHOUT PRIOR APPROVAL FROM THE ENGINEER. ADDITIONAL WELDING, OUTSIDE OF WHAT IS SHOWN ON SPECIFIC DETAIL, IS NOT ALLOWED W/O PRIOR APPROVAL FROM ENGINEER.
11. ROOF PERIMETER STEEL ELEMENTS SUCH AS ANGLES OR BENT PL's, NOTED TO BE CONTINUOUS, SHALL BE FIELD SPLICED WITH A FULL LENGTH SQUARE GROOVE FULL PENETRATION WELD UTILIZING A MINIMUM 1/8" ROOT OPENING.
12. KEEP STEEL FREE OF PAINT AND PRIMER THAT IS TO BE FIELD WELDED.
13. BEAM CONNECTIONS SHALL CONFORM TO AISC MANUAL OF STEEL CONSTRUCTION (ASD), LATEST EDITION. UNLESS NOTED ON PLAN, CONNECTIONS SHALL BE SIZED TO SUPPORT ½ UNIFORM LOAD CAPACITY LISTED IN PART 10 OF THE ASD.
14. PROVIDE SHRINKAGE RESISTANT GROUT CONFORMING TO ASTM C1107 UNDER ALL COLUMN BASE PLATES. GROUT STRENGTH TO HAVE A MINIMUM DESIGN COMPRESSIVE STRENGTH AT LEAST TWICE THE STRENGTH OF THE FOUNDATION CONCRETE.
15. DO NOT MODIFY ANY ANCHOR BOLT SETTINGS OR BASE PLATE HOLES WITHOUT PRIOR APPROVAL FROM STRUCTURAL ENGINEER.
16. PROVIDE 50 FEET OF ∠3x3x¼ FOR MISC BRACING, ETC. AS DIRECTED BY ENGINEER. CONTRACTOR SHALL INCLUDE AN ALLOWANCE FOR INSTALLATION OF MATERIAL.
17. COLUMN BASE PLATES SHALL BE 3/4" THICK AND 6" LARGER IN EACH DIRECTION THAN THE NORMAL DIMENSION OF THE SUPPORTED COLUMN (I.E. 12" SQUARE BASE PLATE FOR 6" DIA. OR 6" SQ. COLUMN AND 10" SQUARE BASE PLATE FOR 4" DIA. OR 4" SQ. COLUMN). ALL BASE PLATES SHALL BE ANCHORED TO PER/FOOTING BELOW WITH (4) 3/4" DIA. x 1'-1" HEADED ANCHOR RODS WITH HEAVY HEX NUTS, TACK WELDED TO ROD, SET INTO FOUNDATION WALL (9" EMBEDMENT+ 4" PROJECTION) UNLESS SPECIFICALLY NOTED OTHERWISE.

STEEL LINTELS

18. LINTELS NOT SHOWN ON PLAN SHALL BE THE FOLLOWING, PROVIDE 8" MIN BEARING LENGTH. EACH ANGLE SHALL BE PROVIDED PER 4" OF WALL THICKNESS. ALL LINTELS TO BE GALVANIZED.

SPAN	SIZE
< 4'-0"	L3 1/2 x 3 1/2 x 1/4
4'-0" TO 6'-0"	L4 x 3 1/2 x 1/4
6'-0" TO 8'-0"	L5 x 3 1/2 x 9/16
> 8'-0"	CONTACT STRUCTURAL ENGINEER

LIGHT GAUGE FRAMING NOTES:

1. LIGHT GAUGE STEEL CONSTRUCTION SHALL CONFORM TO THE FOLLOWING STANDARDS:

    AMERICAN IRON & STEEL INSTITUTE (AISI) – "SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS", LATEST EDITION.

    AMERICAN IRON & STEEL INSTITUTE (AISI) – "STANDARD FOR COLD-FORMED STEEL FRAMING" 2004 EDITION.

    ASTM C-955 – "STANDARD SPECIFICATION FOR LOAD BEARING STEEL STUDS, TRACKS, & BRIDGING."

    ASTM C-1007 – "STANDARD SPECIFICATION FOR INSTALLATION OF LOAD BEARING STEEL STUDS & RELATED ACCESSORIES."
2. WELDED CONNECTIONS SHALL BE MADE IN ACCORDANCE WITH THE LATEST RECOMMENDATION OF:

    AISI – AMERICAN IRON AND STEEL INSTITUTE
    AWS – AMERICAN WELDING SOCIETY
3. TRACKS SHALL BE INSTALLED AT THE TOPS AND BOTTOMS OF ALL VERTICAL STUDS AND SHALL BE THE SAME GAUGE AS THE WALL STUDS (U.O.N.). TRACKS SHALL BE CONNECTED TO THE STUD WITH (1) #8 SCREW (MIN.) AT EACH FLANGE. STUDS TO BE FULLY SEATED IN TRACK THE FULL DEPTH UNLESS NOTED OTHERWISE ON THE PLANS OR DETAILS.
4. DEFLECTION CRITERIA FOR ENGINEERED CURTAIN WALL SYSTEMS SHALL BE L/360 FOR SYSTEMS THAT NOT SUPPORT ANY MASONRY OR BRITTLE FINISH. L/600 SHALL BE USED FOR SYSTEMS THAT SUPPORT MASONRY OR BRITTLE FINISHES.
5. MATERIAL PROPERTIES: 33 ksi - LESS THEN 16 GA. (U.O.N.), 50 ksi – 16 GA. AND HEAVIER. ASTM A653, A875, A792 OR A463, COATINGS SHALL CONFORM TO (G-60 GALV. FINISH, MIN.)
6. ALL WALL STUDS SHALL BE BRACED WITH A COLD ROLLED CHANNEL AT THE STUD PUNCH OUTS. ATTACHED TO STUD WITH A CLIP ANGLE OR WELD. THE COLD ROLLED CHANNELS SHALL BE 16 GA. AND LAPPED MIN 12" AT SPLICE. VERT SPACING OF CHANNELS SHALL NOT EXCEED 4'-0" O.C. SEE STANDARD DETAILS.
7. NOTCHING OF STUDS, JOISTS, BEAMS, OR TRUSS MEMBERS IS NOT PERMITTED WITHOUT PRIOR APPROVAL. NO SPLICES IN STUDS, JOISTS, OR OTHER LOAD CARRYING MEMBERS MAY BE MADE WITHOUT PRIOR APPROVAL FROM ENGINEER.
8. STRUCTURAL LOAD BEARING (GRAVITY, WIND, AND/OR SEISMIC) METAL STUDS NOT SPECIFIED ON CONTRACT DRAWINGS HAVE NOT BEEN DESIGNED BY THE ENGINEER OF RECORD. METAL STUD DESIGN SHALL BE PERFORMED BY A REGISTERED ENGINEER IN THE STATE OF THE PROJECT. SEALED CALCULATIONS & SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL.
9. WEB PUNCH OUTS MUST BE LOCATED A MINIMUM 1'-0" CLEAR FROM ALL BEARING LOCATIONS ON HEADERS AND JOISTS.
10. WEB PUNCH OUTS ON TYP. STUDS AND KING ASSEMBLIES MUST BE LOCATED A MINIMUM OF 12" FROM ATTACHMENT TO BOTTOM TRACK OR ATTACHMENT AT ROOF OR FLOOR LEVELS.
11. JOISTS MUST BE BRACED AGAINST ROTATION @ ALL BRG SUPPORTS BY TRACK OR BLOCKING (U.O.N.). ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICUALR MEMBERS OR AS REQUIRED ON ANGULAR FIT AGAINST ABUTTING MEMBERS.
12. PROVIDE HORIZONTAL BRIDGING AT ALL JOIST MEMBERS AS RECOMMENDED BY THE AISI SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS WITH MINIMUM ROWS AS FOLLOWS: (U.O.N.) SEE DETAILS. BRIDGING MATERIALS ARE TO BE THE SAME GRADE AND GAUGE AS JOIST MEMBERS.

    SPANS UP TO 14'-0" – 1 ROW AT MIDPOINT
    14'-0" UP TO 20'-0" – 2 ROWS @ THIRD POINTS
    20'-0" & GREATER – 1 ROW EVERY 6'-0" O.C.
13. ALL JOISTS, BEAMS, GIRDERS, AND TRUSSES SHALL HAVE FULL BEARING OVER SUPPORT (U.O.N.). PROVIDE WEB STIFFENERS FOR STUD JOISTS AT ALL REACTION POINTS, INTERMEDIATE CONCENTRATED LOADS, AND WHERE INDICATED ON THE DRAWINGS.

PRECAST / PRESTRESSED CONCRETE NOTES:

1. PRECAST CONCRETE UNITS SHALL BE DESIGNED TO SUPPORT THE LOADS DESCRIBED ON THE CONTRACT DOCUMENTS. MEMBER DESIGN SHALL BE PERFORMED ACCORDING TO THE ACI 318 AND PCI LATEST EDITIONS AND UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECT.
2. VERIFY SIZE AND LOCATION OF OPENINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND STRUCTURAL PLANS.
3. ALL PRECAST TO PRECAST CONNECTIONS ARE THE RESPONSIBILITY OF PRECAST SUPPLIER. SHOW FIELD WELDS AND CONNECTION MATERIAL REQUIREMENTS ON SHOP DRAWINGS SUBMITTALS.
4. PRECAST MEMBERS HAVE BEEN INDICATED ON THE DRAWINGS BY GENERAL SIZE AND DEPTH. THE STRUCTURAL DESIGN OF THESE MEMBERS AND THEIR LIFTING ACCESSORIES SHALL BE BY A REGISTERED ENGINEER IN THE STATE OF THE PROJECT BY THE PRECAST MANUFACTURER.

    A. DESIGN DEVIATIONS WILL BE PERMITTED AFTER THE ENGINEERS APPROVAL OF THE MANUFACTURER'S PROPOSED DESIGN SUPPORTED BY CALCULATIONS AND DRAWINGS.

    B. DESIGN DEVIATIONS MUST PRODUCE AN INSTALLATION EQUIVALENT TO THE BASIC INTENT WITHOUT INCURRING ADDED COSTS.
5. ALL SUBMITTAL DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
6. HEADERS AT OPENINGS THROUGH PRECAST PLANK SHALL BE SUPPLIED BY THE PRECAST SUPPLIER. ADJACENT PRECAST UNITS SHALL BE DESIGNED FOR THE LOADS AT THE HANGER BEARING LOCATION.
7. PRECAST SUPPLIER SHALL BE RESPONSIBLE FOR OPENINGS 8" AND LARGER IN SIZE THROUGH PRECAST MEMBERS. HOLES LESS THAT 8" SHALL BE CUT BY THE TRADE WITH PRIOR APPROVAL OF THE PRECAST SUPPLIER.
8. PRECAST MEMBERS SHALL BE ERECTED SIMULTANEOUSLY ON EACH SIDE OF THE SUPPORTING WALLS AND BEAMS TO MAINTAIN STABILITY.
9. VERIFY WITH ALL TRADES IF ADDITIONAL EMBEDS ARE REQUIRED FOR TRADE COORDINATION AND INSTALLATION.
10. CONTRACTOR RESPONSIBLE TO CONTACT PRECAST SUPPLIER IF COMPONENTS ARE ATTACHED TO PRECAST EMBEDS INCORRECTLY OR MISALIGNED. CONTRACTOR TO WORK WITH PRECAST ENGINEER TO DEVELOP APPROPRIATE FIELD FIX, IF NECESSARY. ALL CORRESPONDENCES TO BE COPIED TO STRUCTURAL ENGINEER OF RECORD.

STEEL DECK:

1. STEEL DECK SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE TO STEEL DECK INSTITUTE STANDARD(SDI).
2. STEEL DECK MANUFACTURER TO PROVIDE DETAILED SHOP DRAWINGS FOR REVIEW AND APPROVAL BY ARCHITECT AND STRUCTURAL ENGINEER.
3. STEEL DECK SIZES (MIN. SIZE) U.O.N. ON PLAN/DETAILS:

ROOF DECK-1½"	TYPE B,	22 GA.
FLOOR DECK-3"	TYPE VL1,	186A
STOOP DECK-1½"	TYPE C,	20 GA. (GALV).

FOUNDATION NOTES:

1. FOOTINGS ARE DESIGNED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF 3,000 FOR STRIP FOOTINGS AND PAD FOOTINGS. THESE VALUES ARE PER THE GEOTECHNICAL REPORT #13-006 BY INDEPENDENT TESTING TECHNOLOGYS ISSUED ON JANUARY 25, 2013.
2. PROTECT FOOTING EXCAVATIONS FROM WATER, MOISTURE, OR FROST INFILTRATION. PRIOR TO PLACEMENT OF FOOTING CONCRETE, CLEAN FOOTING EXCAVATIONS OF SNOW, WATER, MUD, DIRT, AND DEBRIS.
3. FROST COVER FOR FOOTINGS SHALL BE A MINIMUM OF 5'-0" TO THE BOTTOM OF THE FOOTING. SEE PLAN FOR TOP OF FOOTING ELVATIONS AND DETAILS FOR FOOTING STEP REQUIREMENTS.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE CONSULTANTS IMMEDIATELY OF ANY SPECIAL SOIL OR WATER CONDITIONS THAT EXIST ON SITE.
5. BACKFILL & COMPACTION SHALL BE INSPECTED AND TESTED BY A LICENSED GEOTECHNICAL ENGINEER OR QUALIFIED FIELD TECH. SUBMIT REPORTS TO THE ENGINEER.
6. BACKFILL SHALL BE COMPACTED BY MECHNICAL MEANS. WATER INFILTRATION SHALL NOT BE ALLOWED. BACKFILL SHALL BE PLACED IN ALTERNATIVE LIFTS ON EACH SIDE OF THE FOUNDATION WALLS FOR STABILITY.
7. UNLESS SPECIFICALLY PRESCRIBED IN A GEOTECHNICAL REPORT, BACKFILL SHALL BE PLACED AND COMPACTED IN LOOSE LIFT THICKNESSES OF 6" OR LESS. MOISTURE CONTENT AT THE TIME OF COMPACTION SHOULD BE +/- 3% OF OPTIMUM MOISTURE AND IT IS RECOMMENDED ALL ENGINEERED FILL BELOW FOOTINGS BE COMPACTED TO A MINIMUM OF 95% OF THE STANDARD PROCTOR MAX DENSITY (ASTM D698-00a).

SHOP DRAWING NOTES:

1. CONTRACTOR TO FURNISH COMPLETE AND DETAILED SHOP DRAWINGS FOR REVIEW BY THE STRUCTURAL ENGINEER.
2. SHOP DRAWING & SUBMITTALS REQUIRED ON THIS PROJECT FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER INCLUDE:

SHOP DRAWING OR SUBMITTAL	NOTES/COMMENTS
CONCRETE MIX DESIGN	---
CONCRETE REINFORCEMENT SHOP DRAWINGS	---
STRUCTURAL STEEL SHOP DRAWINGS	---
STEEL DECK SHOP DRAWINGS	---
CMU, MORTAR, & GROUT MATERIAL SPECS	---
CMU REINFORCEMENT SHOP DRAWINGS	---
PRECAST CONCRETE SHOP DRAWINGS	DEFERRED SUBMITTAL: SEE NOTE 5
COLD-FORMED METAL FRAMING SHOP DRAWINGS	---
LUMBER MATERIAL GRADE SUBMITTAL	---
WOOD TRUSS SHOP DRAWINGS	DEFERRED SUBMITTAL: SEE NOTE 5

3. CONTRACT DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. ALL SHOP DRAWINGS MUST BEAR THE REVIEW STAMP OF THE CONTRACTOR BEFORE THEY ARE REVIEWED BY THE ENGINEER.
4. SHOP DRAWINGS SHALL SHOW ALL FIELD DETAILS AND ADDITIONAL INFORMATION NEEDED FOR THE CONTRACTOR TO CONSTRUCT THE BUILDING PER THE CONTRACT DOCUMENTS.
5. SHOP DRAWINGS COMPONENTS DESIGNATED AS A DEFERRED SUBMITTAL "DESIGNED BY OTHERS" OR "PRE-ENGINEERED" SHALL INCLUDE A CALCULATION PACKAGE THAT IS TO BE STAMPED AND SIGNED BY THE COMPONENT DESIGN ENGINEER PRIOR TO SUBMITTAL FOR APPROVAL TO THE E.O.R.
6. SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING TIMES OF INSPECTION AND SHALL BE CLEARLY INDICATED THEY HAVE BEEN REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
7. REVIEW OF SUBMITTALS AND SHOP DRAWINGS BY THE ARCHITECT AND STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLEY RESPONSIBLE FOR THE ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

DESIGN CRITERIA:

1. FLOOR LOADS

FLOOR LOADS	LIVE	DEAD
1ST FLOOR:	100 psf	127 psf
2ND FLOOR:	100 psf	127 psf

    * INCLUDES WEIGHT OF TOPPED 8" HC= 81 psf
2. ROOF LOADS:

    GROUND SNOW LOAD Pg = 50 psf
    FLAT ROOF (SNOW) * Pf = 35 psf
    ROOF DEAD LOAD DL = 73 psf +

    SNOW IMPORTANCE FACTOR Is = 1.0
    EXPOSURE FACTOR Ce = 1.0
    THERMAL FACTOR Ct = 1.0 (TYP)

    * PLUS DRIFTING AS REQD
    + ADD'L 5 PSF DL @ OVERBUILD
3. WIND LOADS:

    BASIC WIND SPEED (3 SEC. GUST) 90 mph
    WIND EXPOSURE CATEGORY 'B'
    WIND IMPORTANCE FACTOR Iw = 1.0
    INTERNAL PRESSURE COEFFICIENT GCpi = +/- 0.18

FLAT ROOF - 1:12 SLOPE

COMPONENT AND CLADDING WIND LOADS (PSF)							
ZONE	EFFECTIVE AREA (SF)	PRESSURE	SUCTION	ZONE	EFFECTIVE AREA (SF)	PRESSURE	SUCTION
1	10	10.0	14.6	3	100	10.0	15.8
1	100	10.0	13.3	4	10	14.6	15.8
2	10	10.0	24.4	4	100	12.4	13.6
2	100	10.0	15.8	5	10	14.6	19.5
3	10	10.0	36.8	5	100	12.4	15.1
ALL LOADS ARE GROSS SUPERIMPOSED LOADS							
CORNER VALUES TO BE USED WITHIN 13'-0" OF BUILDING'S EXTERIOR CORNERS							
DEFLECTION CRITERIA FOR OUT-OF-PLANE LOADS ON WALLS = L/240							

6.5:12 - 12:12 SLOPE

COMPONENT AND CLADDING WIND LOADS (PSF)							
ZONE	EFFECTIVE AREA (SF)	PRESSURE	SUCTION	ZONE	EFFECTIVE AREA (SF)	PRESSURE	SUCTION
1	10	13.3	14.6	3	100	12.1	14.6
1	100	12.1	12.1	4	10	14.6	15.8
2	10	13.3	17.0	4	100	12.4	13.6
2	100	12.1	14.6	5	10	14.6	19.5
3	10	13.3	17.0	5	100	12.4	15.1
ALL LOADS ARE GROSS SUPERIMPOSED LOADS							
CORNER VALUES TO BE USED WITHIN 3'-0" OF BUILDING'S EXTERIOR CORNERS							
DEFLECTION CRITERIA FOR OUT-OF-PLANE LOADS ON WALLS = L/600							

4. SEISMIC LOADS:

    SEISMIC DESIGN CATEGORY: "A"
    SEISMIC SITE CLASS: "D" (ASSUMED)
    MAPPED SPECTRAL RESPONSE ACCELERATIONS: S_S = 0.079 S_1 = 0.022
    SPECTRAL RESPONSE COEFFICIENTS: S_{DS}= 0.085 S_{D1}= 0.035
    OCCUPANCY CATEGORY: I
    SEISMIC IMPORTANCE FACTOR: I = 1.0
    BASIC SEISMIC FORCE RESISTANCE SYSTEM:
    ASCE 7-05: TABLE 12.2-1 SECTION "C": "ORDINARY CONCRETE MOMENT FRAMING" DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS"
    RESPONSE MODIFICATION COEFFICIENT, R 3.0
    SEISMIC RESPONSE COEFFICIENT, Cs 0.028
    ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE PROCEDURE

WOOD STRUCTURAL PANEL WALL SHEATHING (WOOD FRAMING):

1. ALL PANELS SHALL HAVE A GRADE STAMP BY AN AMERICAN PLYWOOD ASSOCIATION (APA APPROVED AGENCY MEETING THE PROJECT REQUIREMENTS. ALL PANELS SHALL BE EXPOSURE 1, MINIMUM.

2. NOMINAL THICKNESS = 19/32", SPAN RATING 24/0.

3. PANEL JOINTS SHALL BE AT STUD CENTERLINE AND SHALL BE OFFSET FROM ADJACENT PANELS. EDGES TO BE BUTT TIGHT @ JOINTS. PANELS LESS THAN 12" WIDE SHALL NOT BE USED.

4. FASTENERS SHALL BE SPACED @ 6" O.C. ALONG ALL PANEL EDGES & @ 12" O.C. IN THE FIELD OF THE PANELS. FASTENERS TO BE LOCATED NOT LESS THAN 3/8" IN FROM THE EDGE OF THE PANEL

5. MINIMUM FASTENERS SHALL BE: 8d COMMON NAILS, WITH A MINIMUM 13/8" PENETRATION, FLUSH DRIVEN, U.N.O.

6. IN SHEARWALL APPLICATIONS, IF PRE-FABRICATED PANELS ARE USED, WALL SHEATHING MUST SPLICE @ STUD CENTERLINE AND NOT AT A JOINT BETWEEN WALL PANELS. IF SPLICE DOES HAPPEN BETWEEN WALL PANELS, AN ALTERNATE CONNECTION OF PANELS IS TO FASTEN END STUDS OF PANELS TOGETHER W/ 16d NAILS @ 6" O.C. STAGGERED FROM BOTH SIDES. IN BOTH CASES, FIELD INSTALL UPPER TOP PLATE FOR WALL CONTINUITY PER STANDARD DETAILS.

7. IN SHEARWALL APPLICATIONS: PANEL EDGES TO BE UNBLOCKED, U.N.O. PANELS SHALL NOT BE LESS THAN 4'x8', EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING. WALL OPENINGS, SUCH AS WINDOWS AND DOORS, DO NOT CONSTITUTE CHANGES IN FRAMING.

8. FASTENING REQUIREMENTS SHALL APPLY TO ALL STUDS, TOP & BOTTOM PLATES, & BLOCKING.

9. MINIMUM SPACING OF NAILS TO BE 3" O.C. FOR 2X LUMBER.

10. PROPOSED PENETRATIONS THROUGH SHEARWALL SHEATHING NEEDS TO BE SUBMITTED TO E.O.R. FOR APPROVAL PRIOR TO CUTTING IN THE FIELD.

WOOD TRUSSES:

1. METAL PLATE CONNECTED WOOD TRUSSES SHALL BE ENGINEERED BY A REGISTERED ENGINEER IN THE STATE OF THE PROJECT. A SEALED COVER SHEET SHALL BE SUBMITTED WITH THE SHOP DRAWINGS AT THE TIME OF SUBMITTAL. SEE PLAN FOR LOADS AND BEARING LOCATIONS.

    SHOP DRAWINGS SUBMITTAL SHOULD INCLUDE:
    -STRUCTURAL DESIGN DATA
    -LOCATION, CONFIGURATION AND SPACING FOR EACH TYPE OF TRUSS
    -LUMBER SIZE, SPECIES AND GRADE FOR EACH MEMBER
    -SIZE AND CAPACITY OF TRUSS TO TRUSS CONNECTERS DESIGNED BY SUPPLIER
    -CONNECTOR PLATE TYPE, SIZE, GAGE AND LOCATION
    -DETAILS FOR FIELD CONNECTIONS, SPLICES, AND FASTENING OF MULTI-PLYS
    -BEARING DETAILS AT TRUSS SUPPORTS
    -LOCATIONS AND DETAILS OF REQUIRED PERMANENT LATERAL TRUSS BRACING
    -BRACING FOR WEB MEMBERS OF GABLE END TRUSSES, U.O.N. ON DRAWINGS
    -SUPPORT REACTIONS FOR ALL LOAD CASES
    -DEFLECTION OR DEFLECTION RATIO FOR LIVE AND TOTAL LOAD

2. WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", AFPA, AND "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION."

3. SEE DESIGN LOAD NOTES FOR ROOF, FLOOR, AND WIND LOADS TO BE USED IN TRUSS DESIGN. DESIGN TRUSSES FOR LOAD CASE REQUIREMENTS OF REFERENCED BUILDING CODE. COORDINATE WEIGHT AND LOCATION OF MECHANICAL UNITS WITH MECH. AND ARCH.

4. TRUSSES TO BE DESIGNED TO SATISFY THE FOLLOWING DEFLECTION REQUIREMENTS. TRUSS SUPPLIER TO PROVIDE CAMBER AS NECESSARY TO COUNTER DL DEFLECTIONS.
    ROOF TL = L/240 * FLOOR TL = L/240 *
    ROOF SL = L/360 FLOOR LL = L/360
    * NET TL DEFLECTION AFTER CAMBER, NOT TO EXCEED 3/4". PARTITION WALLS BELOW TRUSSES TO BE FRAMED TO ALLOW FOR THIS EXPECTED DEFLECTION.

5. TRUSS SUPPLIER TO MAKE EVERY EFFORT TO FOLLOW FRAMING SCHEME AS THE LOADS HAVE BEEN DISTRIBUTED TO THE FOUNDATION ACCORDINGLY. IF REVISED FRAMING DIRECTIONS ARE DESIRED BY SUPPLIER, PLAN MUST BE SUBMITTED FOR APPROVAL PRIOR TO FOUNDATION CONSTRUCTION.

6. UNLESS OTHERWISE NOTED ON DRAWINGS, ENGINEER OF RECORD HAS NOT PROVIDED STRUCTURAL SHEATHING BENEATH THE BOTTOM CHORD OF ROOF OR FLOOR TRUSSES FOR BRACING.

7. ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC) REQUIRED FOR CONNECTIONS BETWEEN TRUSSES SHALL BE DESIGNED AND SUPPLIED BY THE TRUSS ENGINEER AND SUPPLIER.

8. UNLESS NOTED OTHERWISE, ROOF TRUSSES SHALL BE ATTACHED TO THE TOP PLATE AT ALL BEARING CONDITIONS W/ SIMPSON H2.5T CLIPS INSTALLED PER MANUFACTURER'S INSTRUCTIONS. GIRDERS AND ROOF BEAMS SHOULD BE ATTACHED TO BEARING SUPPORTS WITH (2) H2.5T CLIPS.

9. INSTALL CONTINUOUS BLOCKING BETWEEN TRUSSES AT BEARING WALLS.

10. DO NOT CUT OR REMOVE TRUSS MEMBERS OR MAKE FIELD ALTERATIONS TO THE TRUSSES.

11. LAYOUT AND SPACING GUIDELINES ON PLAN ARE FOR REFERENCE ONLY UNLESS SPECIFICALLY DIMENSIONED OR DETAILED.

12. THE METAL PLATE CONNECTED WOOD TRUSS SUPPLIER SHALL SUPPLY AN HIB-91 PUBLICATION OUTLINING THE PROPER HANDLING, ERECTING, AND BRACING OF TRUSSES.

13. SEE METAL PLATE CONNECTED WOOD TRUSS SHOP DRAWINGS FOR PERMANENT WEB AND CHORD BRACING LOCATIONS AND REQUIREMENTS.

14. GIRDER TRUSSES SHALL BE SUPPORTED BY SAME NUMBER OF STUDS AS TRUSS PLIES (MIN OF 2 STUDS) WITH CONTINUOUS SOLID BEARING TO THE FOUNDATION. FASTEN STUDS TOGETHER WITH (1) ROW OF 10d NAILS @ 6" O.C. STAGGERED.

15. AVOID BEARING GIRDER TRUSSES OVER WALL OPENINGS, UNLESS OTHERWISE DIMENSIONED ON PLAN.

GYPSUM WALLBOARD/GYPSUM SHEATHING (LIGHT GA. FRAMING) NOTES:

1. 5/8" GYPSUM WALL BOARD SHALL BE USED. REFER TO ARCH. DRAWINGS FOR ADDITIONAL REQUIREMENTS OF GYPSUM SHEATHING.
2. PANEL JOINTS SHALL BE AT STUD CENTERLINE AND SHALL BE OFFSET FROM ADJACENT PANELS. EDGES TO BE BUTT TIGHT AT JOINTS. PANELS LESS THAN 12" WIDE SHALL NOTUSED.
3. FASTENERS SHALL BE SPACED AT 8" O.C. ALONG ALL PANEL EDGES & AT 12" O.C. IN THE FIELD OF THE PANELS. FASTENERS TO BE LOCATED NOT LESS THAN 3/8" IN FROM THE EDGE OF THE PANEL.
4. MINIMUM FASTENERS SHALL BE NO. 6 x 1 3/4" WAFER HEAD SELF DRILLING IN ACCORDANCE W/ ASTM C 954. SCREWS SHALL HAVE PENETRATION INTO THE STEEL FRAMING MEMBERS WITH A MINIMUM OF (3) EXPOSED THREADS.
5. ALL PANEL EDGES SHALL BE UNBLOCKED, U.N.O. WHERE HORIZONTAL STRAP BLOCKING IS USED, IT SHALL BE A MINIMUM 2" WIDE & SAME MATERIAL & THICKNESS AS THE FRAMING MEMBERS. SOLID BLOCKING IS REQUIRED BEHIND THE HORIZONTAL JOINT BETWEEN THE FIRST TWO END STUDS.
6. IN SHEARWALL APPLICATIONS, IF PRE-FABRICATED PANELS ARE USED, WALL SHEATHING MUST SPLICE @ STUD CENTERLINE AND NOT AT A JOINT BETWEEN WALL PANELS.
7. PANEL SHEETS SHALL BE APPLIED LONG DIRECTION PERPENDICLAR TO STUDS WITH EDGES BLOCKED AS INDICATED AS PLAN OR APPLIED VERTICALLY WITH ALL EDGES ATTACHED TO FRAMING MEMBERS. PANELS LESS THAN 4'-0" WIDE NEED TO BE APPLIED HORIZONTAL TO FRAMING MEMBERS.
8. FASTENING REQUIREMENTS SHALL APPLY TO ALL STUDS, TRACKS & BLOCKING.

PRE-ENGINEERED LIGHT-GAUGE TRUSSES:

1. LIGHT GAUGE TRUSSES SHALL BE ENGINEERED BY A REGISTERED ENGINEER IN THE STATE OF THE PROJECT. A SEALED COVER SHEET SHALL BE SUBMITTED WITH THE SHOP DRAWINGS AT THE TIME OF SUBMITTAL. SEE PLAN FOR LOADS AND BEARING LOCATIONS.

    SHOP DRAWINGS SUBMITTAL SHOULD INCLUDE:
    -STRUCTURAL DESIGN DATA - DESIGN ASSUMPTIONS
    -SLOPE, DEPTH, SPAN AND SPACING OF EACH TRUSS
    -BEARING LOCATIONS AND MINIMUM BEARING LENGTHS
    -NOMINAL REACTIONS FORCES AND DIRECTIONS
    -LOCATION OF ALL TRUSS CONNECTIONS
    -GUSSET PLATE LOCATIONS, SIZE AND MATERIAL SPECIFICATIONS
    -FASTENER TYPE, SIZE QUANTITIES AND LOCATIONS
    -SHAPE AND MATERILA SPECIFICATION FOR EACH COMPONENT
    -MAXIMUM NOMINAL COMPRESSIVE FORCES IN ALL TRUSS MEMBERS
    -LOCATION OF REQUIRED PERMANENT TRUSS MEMBER BRACING
    -CONNECTION REQUIREMENTS FOR
        -TRUSS-TO-TRUSS GIRDER
        -TRUSS PLY-TO-PLY
        -FIELD ASSEMBLY OF TRUSSES
    -DEFLECTION RATION AND/OR MASIMUM DEFLECTION FOR LIVE AND TOTAL LOAD
2. LIGHT FAUGE STEE TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING STANDARDS
    -AMERICIAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS", LATEST EDITION
    -AMERICAN IRON AND STEEL INSTITUTE (AISI) "STANDARD FOR COLD-FORMED STEEL TRAMING: TRUSS DESIGN", LATEST EDITION
    -AMERICIAN IRON AND STEEL INSTITUTE (AISI) "STANDARD FOR COLD-FORMED STEEL FRAMING: GENERAL PROVISIONS". LATEST EDITION.
3. SEE DESIGN LOAD NOTES FOR ROOF, FLOOR, AND WIND LOADS TO BE USED IN TRUSS DESIGN. DESIGN TRUSSES FOR LOAD CASE REQUIREMENTS AND DEFLECTION LIMITS OF REFERENCED BUILDING CODE. COORDINATE WEIGHT AND LOCATION OF MECHANICALUNITS WITH MECH. AND ARCH.
4. TRUSS SUPPLIER TO MAKE EVERY EFFORT TO FOLLOW FRAMING SCHEME AS THE LOAHAVE BEEN DISTRIBUTED TO THE FOUNDATION ACCORDINGLY. IF REVISED FRAMING DIRECTIONS ARE DESIRED BY SUPPLIER, PLAN MUST BE SUBMITTED FOR APPROVAL PRIOR TO FOUNDATION CONSTRUCTION.
5. UNLESS OTHERWISE NOTED ON DRAWINGS, ENGINEER OF RECORD HAS NOT PROVIDED STRUCTURAL SHEATHING BENEATH THE BOTTOM CHORD OF ROOF OR FLOOR TRUSSES FOR BRACING.
6. ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC) REQUIRED FOR CONNECTIONS BETWEEN TRUSSES SHALL BE DESIGNED AND SUPPLIED BY THE TRUSS ENGINEER AND SUPPLIER.
7. INSTALL CONTINUOUS BLOCKING BETWEEN TRUSSES AT BEARING WALLS.
8. DO NOT CUT, NOTCH OR REMOVE TRUSS MEMBERS OR MAKE FIELD ALTERATIONS TO THE TRUSSES.
9. LAYOUT AND SPACING GUIDELINES ON PLAN ARE FOR REFERENCE ONLY UNLESS SPECIFICALLY DIMENSIONED OR DETAILED.
12. THE LIGHT GAUGE TRUSS SUPPLIER SHALL SUPPLY SPECIFICATIONS OUTLINNING THE PROPER HANDLING, ERECTING, AND BRACING OF TRUSSES
13. SEE LIGHT GAUGE STEEL TRUSS SHOP DRAWINGS FOR PERMANENT WEB AND CHORD BRACING LOCATIONS AND REQUIREMENTS.

No | REVISION | DATE

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Project Number

JODE NICHOLSON, PE MN 44104
DATE: 12-18-2013

GENERAL NOTES

RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

12.20.13
AS NOTED
656-12-339
28/9 JN KK

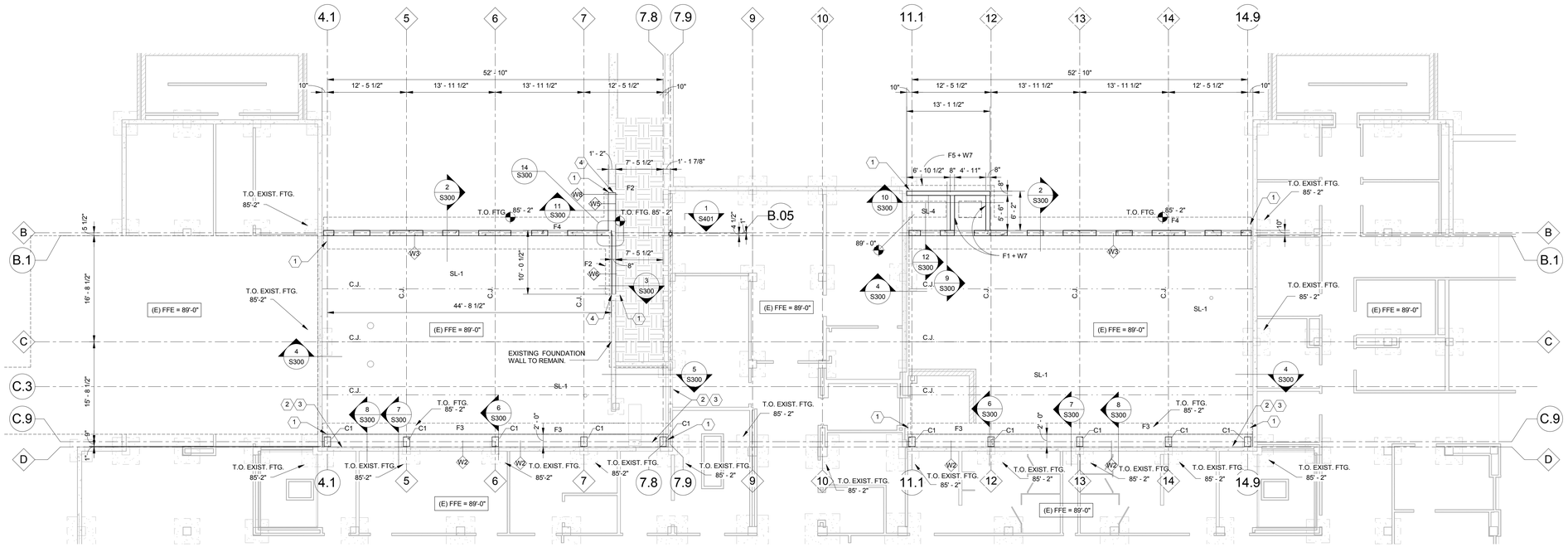
ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

S001

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

KEYNOTES:

1. DRILL LONG. FOOTING REINF. INTO (E) FOOTING W/ #5 x 2'-0" DOWELS. EMBED INTO EXISTING TO BE 8". SEE S001 NOTES FOR ADHESIVE. STEP FOOTING AS REQ'D TO MATCH EXISTING
2. TIE THE NEW SLAB TO THE EXISTING THRU THE DOOR. CUT EXISTING FDN WALL DOWN 8" BELOW FFE, TIE THE NEW SLAB TO EXISTING w/ 3/4" DIA SMOOTH BAR DOWELS @ 12" O.C.. EMBED INTO EXISTING SLAB 5" MINIMUM.
3. SAWCUT NEW OPENING THROUGHT EXISTING WALL. SEE DETAIL 7/S401 FOR LINTEL.
4. DRILL INTO (E) FOOTING FOR ATTACHMENT OF VERTICAL WALL DOWELS. EMBED INTO EXISTING TO BE 8". SEE S001 NOTES FOR ADHESIVE.



STRUCTURAL FOOTING & SLAB SCHEDULE	
TYPE	DESCRIPTION
F1	24" x 12" STRIP FTG. W/ (2) #5 BARS CONT. LONG. & #4 BARS @ 48" O.C., TRANSVERSE
F2	30" x 12" STRIP FTG. W/ (3) #5 BARS CONT. LONG. & #4 BARS @ 48" O.C., TRANSVERSE
F3	48" x 18" STRIP FTG. W/ (6) #5 BARS CONT. LONG. & #6 BARS @ 15" O.C., TRANSVERSE
F4	60" x 12" STRIP FTG. W/ (5) #5 BARS CONT. LONG. & #5 BARS @ 12" O.C., TRANSVERSE
F5	39" x 12" STRIP FTG. W/ (3) #5 BARS CONT. LONG. & #4 BARS @ 48" O.C., TRANSVERSE
SL-1	4" CONC. SLAB W/ #4 BARS @ 18" O.C. A TOP 4" FINE GRANULAR FILL, A TOP 15 MIL VAPOR RETARDER. A TOP COMPACTED GRADE PER GEOTECH REQUIREMENTS.
SL-2	2" CONC. TOPPING OVER 12" HOLLOW CORE PLANK
SL-3	8" HOLLOW CORE PLANK
SL-4	6 1/2", 4,000 PSI CONC. SLAB W/ #4 BARS @ 12" O.C. ATOP 1.5" DEEP, 20 GA. DECKING

WALL SCHEDULE		
TYPE MARK	TYPE	TYPE COMMENTS
W2	24" CONC.	REINF. W/ #6 BARS @ 14" O.C., VERT. E.F. & #5 BARS @ 12" O.C., HORIZ. E.F.
W3	10" CONC. BASE LEVEL 8" CONC. ELEV. LEVELS	PRECAST WALL PANEL PER MANUF.
W4	8" CMU	REINF. W/ #4 BARS @ 48" O.C.IN RMG
W5	14" CONC.	REINF. W/ #4 BARS @ 18" O.C., VERT. E.F. & #5 BARS @ 16" O.C., HORIZ. E.F.
W6	8" CONC.	#4 BARS @ 18" O.C., VERT. & #4 BARS @ 16" O.C., HORIZ.
W7	8" CONC.	#5 @ 18" O.C., O.F., VERT . & HORIZ.
W8	8" CMU	UNREINFORCED

COLUMN SCHEDULE	
Type	Description
4x4	
C1	18" DEEP x 12" WIDE PRECAST (BY OTHERS)
C2	HSS 6x6x1/2"

FOUNDATION PLAN NOTES:

1. ALL WALL FOOTINGS ARE TO BE CENTERED ON WALLS U.N.O. ALL PAD FOOTING ARE TO BE CENTERED ON COLUMNS, U.N.O.
2. CONTRACTOR TO VERIFY ALL EXISTING/NEW DIMENSIONS AND NOTIFY ARCHITECT/ENGINEER IF ANY DISCREPANCIES EXIST.
3. ALL EXISTING FOUNDATION INFORMATION SHOWN ON PLAN/DETAILS ARE FOR GRAPHICAL REPRESENTATION ONLY, U.N.O.. ALL EXISTING FOUNDATION INFORMATION WAS OBTAINED FROM EXISTING DRAWINGS ISSUED BY U.S. VETERANS BUREAU, DATED 03/15/1929. G.C. TO CONTACT STRUCTURAL ENGINEER WITH ADDITIONAL INFORMATION AS IT IS VERIFIED IN THE FIELD SO DESIGN ASSUMPTIONS CAN BE VERIFIED.
4. SEE 1/S300 FOR GENERAL FOUNDATION DETAILS. SEE SHEET S001 FOR GENERAL CONCRETE AND FOUNDATION NOTES.
5. SEE ARCH FOR DIMENSIONING OF FOUNDATION WALL HOLD-OUTS, LOCATION OF STOOPS, SLAB RECESS LOCATIONS, AND SLOPED SLAB AREAS.
6. CONTRACTOR TO REFER TO GEOTECHNICAL REPORT FOR OVER-EXCAVATION & STRUCTURAL BACKFILL PROCEDURES.
7. TYPICAL SLAB ON GRADE CONTROL JOINTS TO BE CUT INTO SQUARES SPACED BE SPACED @ 10'-0" MAX U.N.O. ON PLAN. SEE DETAIL 1/S300
8. SEE 13/S300 FOR TYPICAL MECHANICAL EQUIPMENT & HOUSEKEEPING PADS. SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION.

1 / S100 FOUNDATION PLAN
1/8" = 1'-0"

FINISHED FLOOR ELEV. = +/- 89'-0" MATCH EXISTING

No	REVISION	DATE

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Project Number

STAMP/SEAL:
I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.
MN 44104
JOE NICHOLSON, PE REG. NO.
DATE: 12-18-2013

Drawing Title: BUILDING 28 FOUNDATION PLAN

APPROVED: PROJECT DIRECTOR DATE

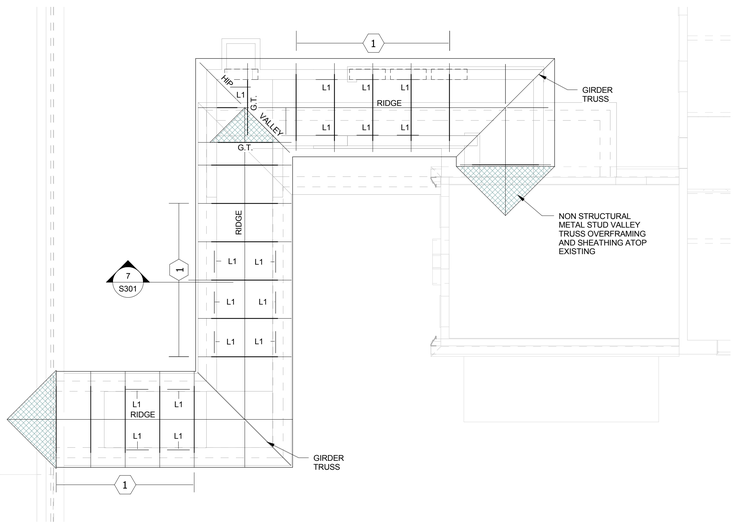
RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

Date	Scale
12.20.13	AS NOTED
Project No.	
656-12-339	
Drawn by	Checked by	Approved by	CAD File
28/9	JN	KK	
Sheet	
S100	

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231



2 S120 FRAMING PLAN
1/8" = 1'-0"

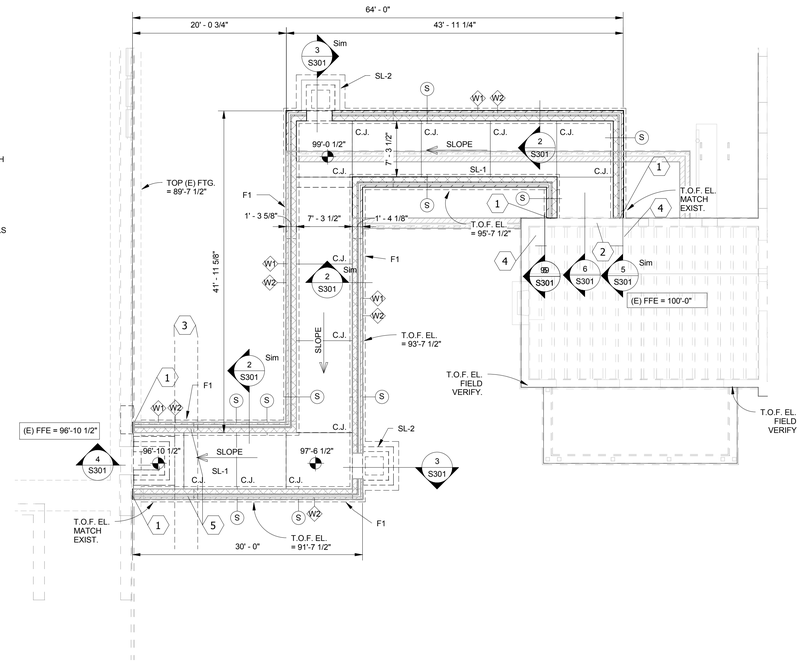
KEYNOTES:

1. METAL STUD ROOF TRUSSES @ 5'-0" O.C.

FRAMING PLAN NOTES:

1. VERIFY SIZE AND LOCATION OF ROOF OPENINGS WITH MECH AND ARCH.
2. ROOF DECK TO 1.5 TYPE B 22 ga.

   TYPICAL: #12 TEK SCREWS @ 36/4 PATTERN
   SIDELAP FASTENERS: #10 TEK - 1 PER SPAN
3. TYPICAL CMU WALLS TO BE REINFORCED WITH #5 VERTICALS @ 2'-0" O.C. UNO. SEE S001 NOTES AND 8/S301 FOR ADDITIONAL CMU REINFORCEMENT REQUIREMENTS.
4. VERIFY ALL WALL OPENING DIMENSIONS W/ ARCH. DRAWINGS.



1 S120 FOUNDATION PLAN
1/8" = 1'-0"

KEYNOTES:

1. DRILL LONG. FOOTING INTO (E.) FOOTING W/ (2) #5 X 2'-0" DOWELS. EMBED. INTO EXISTING TO BE 8". SEE S001 NOTES FOR ADHESIVE. STEP FOOTING AS REQ'D TO MATCH EXISTING.
2. REMOVE (E) CMU TO UNDERSIDE OF ROOF BEAM PER ARCH, DRAWINGS
3. EXISTING CONCRETE DUCT BANK TO REMAIN. B/ DUCT ELEV. =90'-0" FIELD VERIFY
4. NEW MECHANICAL OPENING THRU (E.) PAN JOIST FLOOR. SEE DETAIL 5/S301 FOR FRAMING AROUND OPENING
5. DISCONTINUE F1 FOUNDATION THRU DUCT BANK. PROVIDE 16"X16" MASONRY LINTEL ABOVE DUCT BANK REINFORCED w/ (2) #5 REBAR.

FOUNDATION PLAN NOTES:

1. ALL WALL FOOTINGS ARE TO BE CENTERED ON WALLS U.N.O. ALL PAD FOOTING ARE TO BE CENTERED ON COLUMNS, U.N.O.

2. CONTRACTOR TO VERIFY ALL EXISTING/NEW DIMENSIONS AND NOTIFY ARCHITECT/ENGINEER IF ANY DISCREPANCIES EXIST.

3. ALL EXISTING FOUNDATION INFORMATION SHOWN ON PLAN/DETAILS ARE FOR GRAPHICAL REPRESENTATION ONLY, U.N.O. ALL EXISTING FOUNDATION INFORMATION WAS OBTAINED FROM EXISTING DRAWINGS ISSUED BY U.S. VETERANS BUREAU, DATED 03/15/2012 AND 4/28/1923. G.C. TO CONTACT STRUCTURAL ENGINEER WITH ADDITIONAL INFORMATION AS IT IS VERIFIED IN THE FIELD SO DESIGN ASSUMPTIONS CAN BE VERIFIED.

4. SEE 1/S300 FOR GENERAL FOUNDATION DETAILS. SEE SHEET S001 FOR GENERAL CONCRETE AND FOUNDATION NOTES.

5. SEE ARCH FOR DIMENSIONING OF FOUNDATION WALL HOLD-OUTS, LOCATION OF STOOPS, SLAB RECESS LOCATIONS, AND SLOPED SLAB AREAS.

6. CONTRACTOR TO REFER TO GEOTECHNICAL REPORT FOR OVER-EXCAVATION & STRUCTURAL BACKFILL PROCEDURES.

7. TYPICAL SLAB ON GRADE CONTROL JOINTS TO BE CUT INTO SQUARES SPACED BE SPACED @ 10'-0" MAX U.N.O. ON PLAN.

8. (S) DENOTES FOOTING STEP - S.D. 1/S300

LINTEL SCHEDULE

TYPE MARK	SIZE	TYPE COMMENTS
L1	16X8 MASONRY	SEE 1/S301

STRUCTURAL FOOTING & SLAB SCHEDULE

TYPE	DESCRIPTION
F1	24" x 12" STRIP FTG. W/ (2) #5 BARS CONT. LONG. & #4 BARS @ 48" O.C., TRANSVERSE
SL-1	4" CONC. SLAB W/ #4 BARS @ 18" O.C. A TOP 4" FINE GRANULAR FILL, A TOP 15 MIL VAPOR RETARDER, A TOP COMPACTED GRADE PER GEOTECH REQUIREMENTS.
SL-2	6 1/2", 4000 PSI CONCRETE SLAB W/ #4 BARS @ 12" O.C. ATOP 1.5" DEEP, 20 GA. GALVANIZED DECKING.

WALL SCHEDULE

TYPE MARK	TYPE	TYPE COMMENTS
W1	16" CMU	GROUT SOLID BELOW GRADE. REINF. W/ #5 BARS @ 24" O.C. SEE ARCH. FOR T.O. WALL ELEVATION
W2	8" CMU	REINF. W/ #5 BARS @ 24" O.C.IN RMG

No	REVISION	DATE

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802 21st Street South
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MN 44104
JOE NICHOLSON, PE REG. NO.
DATE: 12-18-2012

DRAWING TITLE: BUILDING 9 CONNECTOR PLANS

APPROVED PROJECT DIRECTOR DATE

RECONFIGURE/EXPAND BUILDINGS 28 &9 FOR RESIDENTIAL REHABILITATION TREAMENT PROGRAM

DRAWING No. | DRAWN BY: JN | CHECKED BY: KK

PROJECT NAME: ST. CLOUD VA MEDICAL CENTER
Enter address here

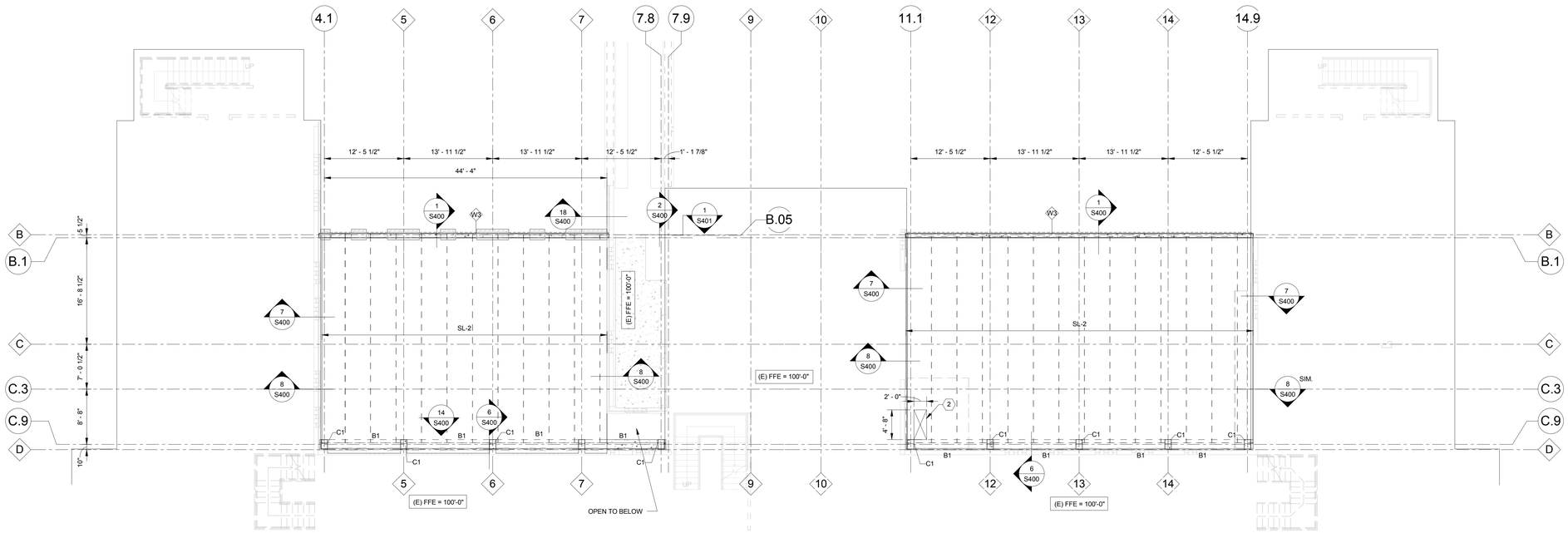
10.11.13	
SCALE: AS NOTED	
656-12-339	
CAD FILE	
DRAWING No. S120	

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231

KEYNOTES:

1. COORDINATE SIZE & LOCATION OF FLOOR OPENINGS W/ ARCH & MECH. NOTIFY ENGINEER IF CHANGES ARE MADE. SEE 12/S400 FOR TYP. OPENING DETAILING (EXISTING FLOOR ASSEMBLY LOCATONS).
2. MECH. OPENINGS IN NEW STRUCTURE. SEE ARCH./MECH. FOR LOCATION & SIZE. SUPPLIER TO DESIGN & PROVIDE FRAMING FOR OPENINGS.
3. DEMO EXISTING WALL BELOW CONCRETE BEAM FOR DUCT OPENING SEE 7/S401 FOR STEEL LINTEL REQUIRED.



STRUCTURAL FOOTING & SLAB SCHEDULE	
Type	**Description**
F1	24" x 12" STRIP FTG. w/(2) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
F2	30" x 12" STRIP FTG. w/(3) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
F3	45" x 15" STRIP FTG. w/ (4) #5 BARS CONT. LONG & #6 BARS @ 18" O.C.
F4	60" x 12" STRIP FTG. w/(3) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
SL-1	4" CONC SLAB w/ #4 BARS @ 18" O.C. A TOP 4" FINE GRANULAR FILL, A TOP 15 MIL VAPOR RETARDER A TOP COMPACTED GRADE PER GEOTECH REQUIREMENTS.
SL-2	2" CONC. TOPPING OVER 12" HOLLOW CORE PLANK
SL-3	8" HOLLOW CORE PLANK
SL-4	6 1/2", 4000PSI CONC SLAB w/ #4 BARS @ 12" O.C. ATOP 1.5" DEEP, 20 GA GALVANIZED DECKING
SL-5	4 1/2" N.W. CONCRETE ATOP 3" VLI-18 GALVANIZED DECK

WALL SCHEDULE		
Type Mark	**Type**	**Type Comments**
W2	24" CONC.	
W3	Exterior - 8" Concrete	PRECAST WALL PANEL PER MANUF.
W4	8" CMU	REINFORCE w/ #4 BARS @ 48" O.C. IN RMG
W6	8" CONC.	#4 BARS @ 18" O.C. VERT & #4 BARS @ 12" O.C. HORIZ.

COLUMN SCHEDULE	
Type	**Description**
4x4	
C1	18" DEEP x 12" WIDE PRECAST (BY OTHERS)
C2	HSS 6x6x1/2"

BEAM SCHEDULE	
Type	**Description**
4HC8	
B1	1'-6" x 1'-8" PRECAST 'L' BEAM.
B2	W6x9.
B3	W6x20.
B4	W12x26. T.O.S. = 110'-2" W/ 1/4"x16" BOTTOM PLATE

PRECAST FRAMING PLAN NOTES:

1. SEE ARCH. FOR ALL WALL DIMENSIONS, & OPENING LOCATIONS.
2. SUPERIMPOSED PRECAST LOADS ARE NOT SHOWN ON PLAN THAT OCCUR AT LOCATIONS OF PRECAST BEARING, SUCH AS DIRECTLY ABOVE PERIMETER AND INTERIOR WALLS. PRECAST SUPPLIER TO PROVIDE SOLID GROUTED CORES FOR A MINIMUM WIDTH OF 12" AT THESE LOCATIONS (U.N.O.).

1 / S200 FIRST LEVEL FRAMING PLAN
1/8" = 1'-0"

FINISHED FLOOR ELEV. = +/- 100'-0" MATCH EXISTING

No	REVISION	DATE

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JOE NICHOLSON, PE REG. NO. MN 44104
DATE: 12-18-2013

Drawing Title: BUILDING 28 FIRST FLOOR PLAN

APPROVED: PROJECT DIRECTOR DATE:

RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

DESIGNED BY	DRAWN BY	CHECKED BY
28/9	JN	KK

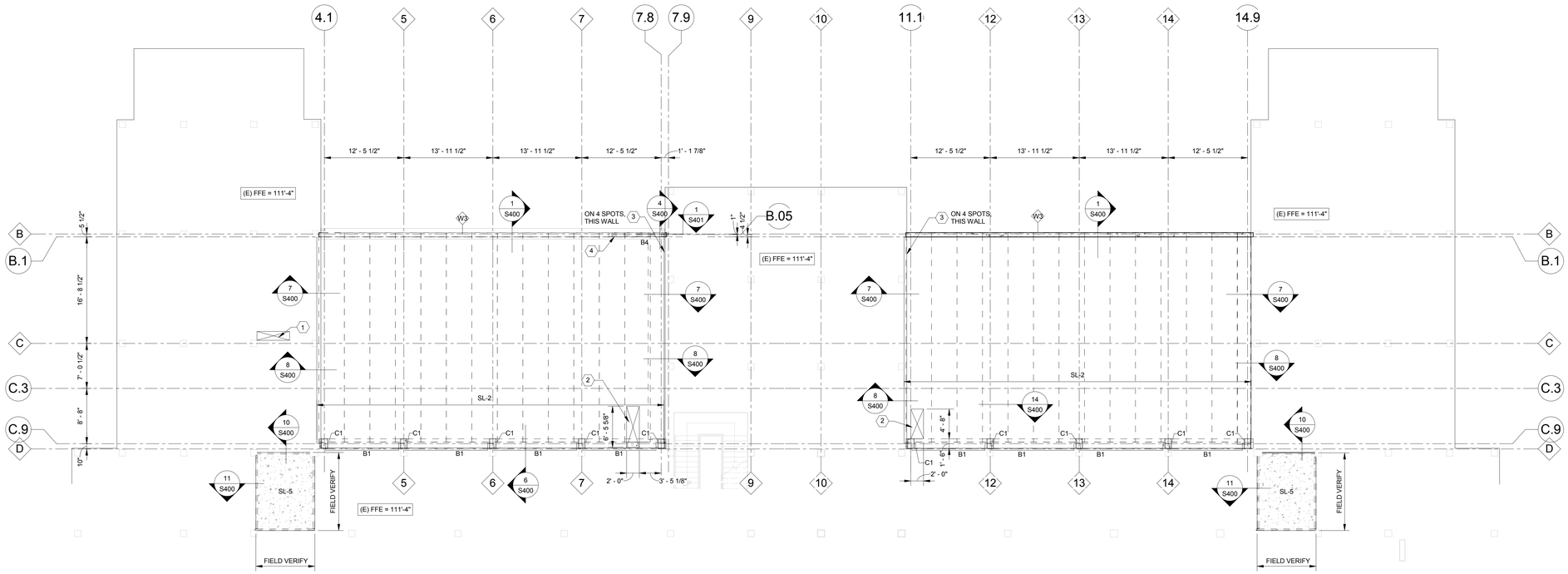
12.20.13	
AS NOTED	
656-12-339	
CAD FILE	
Sheet	S200

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231

KEYNOTES:

1. COORDINATE SIZE & LOCATION OF FLOOR OPENINGS W/ ARCH & MECH. NOTIFY ENGINEER IF CHANGES ARE MADE. SEE 12/S400 FOR TYP. OPENING DETAILING (EXISTING FLOOR ASSEMBLY LOCATONS).
2. MECH. OPENINGS IN NEW STRUCTURE. SEE ARCH./MECH. FOR LOCATION & SIZE. SUPPLIER TO DESIGN & PROVIDE FRAMING FOR OPENINGS.
3. DEMO EXISTING WALL BELOW CONCRETE BEAM FOR DUCT OPENING SEE 7/S401 FOR STEEL LINTEL REQUIRED.
4. PRECAST MFG TO DESIGN STEEL BEAM TO END OF PRECAST CONNECTION FOR
   DL = 23k
   LL = 4.6k
   SL = 8.4k



STRUCTURAL FOOTING & SLAB SCHEDULE	
Type	**Description**
F1	24" x 12" STRIP FTG. w/(2) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
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F3	45" x 15" STRIP FTG. w/ (4) #5 BARS CONT. LONG & #6 BARS @ 18" O.C.
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SL-5	4 1/2" N.W. CONCRETE ATOP 3" VLI-18 GALVANIZED DECK

WALL SCHEDULE		
Type Mark	**Type**	**Type Comments**
W2	24" CONC.	
W3	Exterior - 8" Concrete	PRECAST WALL PANEL PER MANUF.
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COLUMN SCHEDULE	
Type	**Description**
4x4	
C1	18" DEEP x 12" WIDE PRECAST (BY OTHERS)
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BEAM SCHEDULE	
Type	**Description**
4HC8	
B1	1'-6" x 1'-8" PRECAST 'L' BEAM.
B2	W6x9.
B3	W6x20.
B4	W12x26. T.O.S. = 110'-2" W/ 1/4"x16" BOTTOM PLATE

PRECAST FRAMING PLAN NOTES:

1. SEE ARCH. FOR ALL WALL DIMENSIONS, & OPENING LOCATIONS.
2. SUPERIMPOSED PRECAST LOADS ARE NOT SHOWN ON PLAN THAT OCCUR AT LOCATIONS OF PRECAST BEARING, SUCH AS DIRECTLY ABOVE PERIMETER AND INTERIOR WALLS. PRECAST SUPPLIER TO PROVIDE SOLID GROUTED CORES FOR A MINIMUM WIDTH OF 12" AT THESE LOCATIONS (U.N.O.).

1 / S201 SECOND LEVEL FRAMING
1/8" = 1'-0"

FINISHED FLOOR ELEV. = +/- 111'-4" MATCH EXISTING

No	REVISION	DATE

SANDMAN CONSULTING
1301 Highway 29 North
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MN 44104
JOE NICHOLSON, PE REG. NO.
DATE: 12-18-2013

DRAWING TITLE: BUILDING 28 SECOND FLOOR PLAN

PROJECT TITLE: RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

DATE: 12.20.13
SCALE: AS NOTED
PROJ. NO.: 656-12-339

DRAWN BY: JN
CHECKED BY: KK
CAD FILE:

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

DRAWING NO.: S201

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231

KEYNOTES:

1. COORDINATE SIZE & LOCATION OF FLOOR OPENINGS W/ ARCH & MECH. NOTIFY ENGINEER IF CHANGES ARE NOTED. SEE 12/S400 FOR TYP. OPENING DETAILING (EXISTING FLOOR ASSEMBLY LOCATONS).
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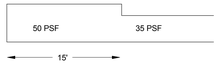
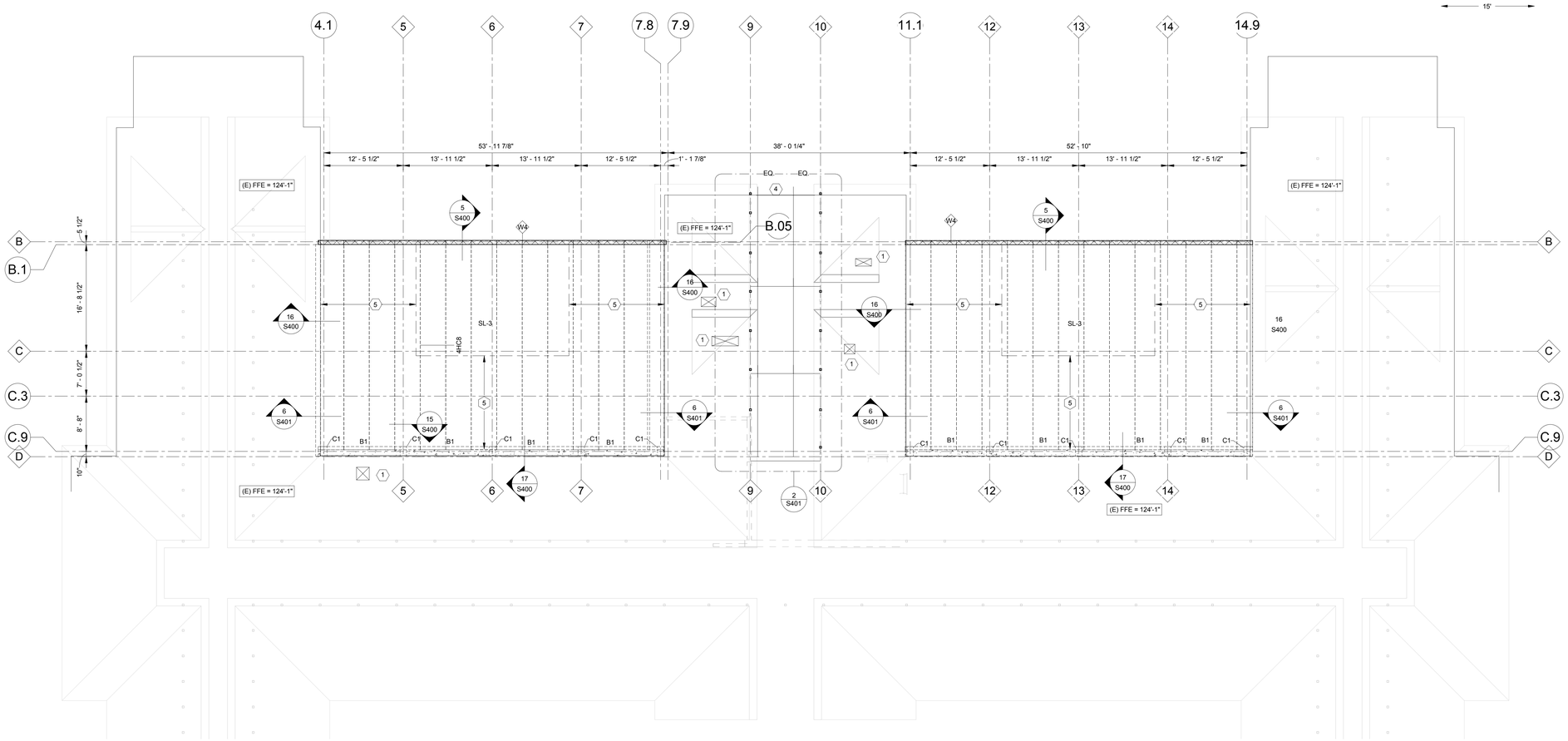
3. DEMO EXISTING WALL BELOW CONCRETE BEAM FOR DUCT OPENING SEE 7/S401 FOR STEEL LINTEL REQUIRED.
4. DEMO EXISTING WALL PER ARCH DRAWINGS. INFILL EXISITNG CLAY MASONRY w/ NEW 6" UNREINFORCED MASONRY PER DETAIL 18/S400.
5. PRECASTER TO DESIGN PLANK FOR THE FOLLOWING ADDITIONAL SNOW LOAD





STRUCTURAL FOOTING & SLAB SCHEDULE	
Type	**Description**
F1	24" x 12" STRIP FTG. w/(2) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
F2	30" x 12" STRIP FTG. w/(3) #5 BARS CONT. LONG & #4 BARS @ 48" O.C. HORIZ.
F3	45" x 15" STRIP FTG. w/ (4) #5 BARS CONT. LONG & #6 BARS @ 18" O.C.
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SL-5	4 1/2" N.W. CONCRETE ATOP 3" VLI-18 GALVANIZED DECK

COLUMN SCHEDULE	
Type	**Description**
4x4	
C1	18" DEEP x 12" WIDE PRECAST (BY OTHERS)
C2	HSS 6x6x1/2"

BEAM SCHEDULE	
Type	**Description**
4HC8	
B1	1'-6" x 1'-8" PRECAST 'L' BEAM.
B2	W6x9.
B3	W6x20.
B4	W12x26. T.O.S. = 110'-2" W/ 1/4"x16" BOTTOM PLATE

WALL SCHEDULE		
Type Mark	**Type**	**Type Comments**
W2	24" CONC.	
W3	Exterior - 8" Concrete	PRECAST WALL PANEL PER MANUF.
W4	8" CMU	REINFORCE w/ #4 BARS @ 48" O.C. IN RMG
W6	8" CONC.	#4 BARS @ 18" O.C. VERT & #4 BARS @ 12" O.C. HORIZ.

PRECAST FRAMING PLAN NOTES:

1. SEE ARCH. FOR ALL WALL DIMENSIONS, & OPENING LOCATIONS.
2. SUPERIMPOSED PRECAST LOADS ARE NOT SHOWN ON PLAN THAT OCCUR AT LOCATIONS OF PRECAST BEARING, SUCH AS DIRECTLY ABOVE PERIMETER AND INTERIOR WALLS. PRECAST SUPPLIER TO PROVIDE SOLID GROUTED CORES FOR A MINIMUM WIDTH OF 12" AT THESE LOCATIONS (U.N.O.).

16 / S202 ROOF FRAMING @ ADDITIONS
1/8" = 1'-0"

T.O. PRECAST ELEV. = +/- 123'-5"

No	REVISION	DATE

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JOE NICHOLSON, PE REG. NO. MN 44104
DATE: 12-18-2013

DRAWING TITLE: BUILDING 28 ROOF PLAN

APPROVED: PROJECT DIRECTOR DATE:

RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

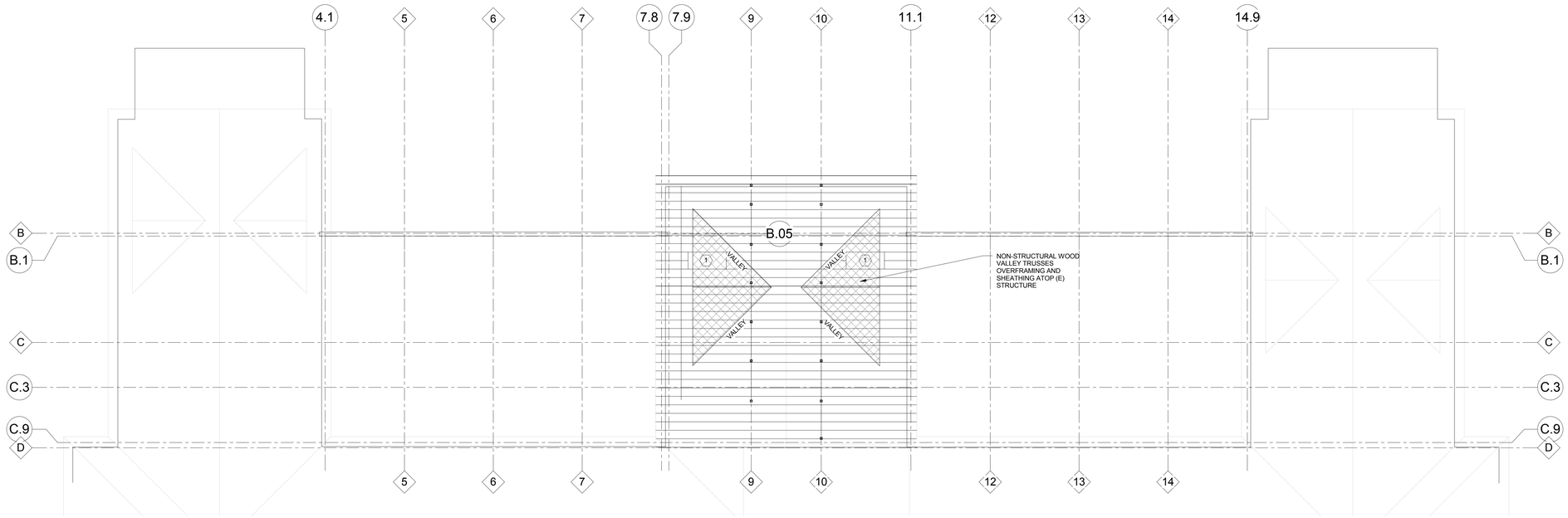
12.20.13	
AS NOTED	
656-12-339	
28/9	JN / KK
S202	

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231

KEYNOTES:

1. REMOVE PORTION OF (1) EXISTING WOOD ROOF JOIST FOR MECHANICAL DUCT. SEE DETAIL 5/S401. REFER TO MECANICAL/ARCH DRAWINGS FOR LOCATION.



FRAMING NOTES:

1. TRUSS SUPPLIER TO COOR. TRUSS LAYOUT WITH ARCH. AND MECHANICAL AND TO PROVIDE CONNECTION TO EXISTING STRUCTURE.

1 / S203 ROOF FRAMING @ PENTHOUSE
1/8" = 1'-0"

No	REVISION	DATE

SANDMAN CONSULTING
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JOE NICHOLSON, PE REG. NO. MN 44104

DATE: 12-18-2013

APPROVED: PROJECT DIRECTOR DATE

Drawing Title
BUILDING 28 ATTIC FRAMING PLAN

RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

Drawn by	Checked by	Approved by
28/9	JN	KK

12.20.13
AS NOTED
656-12-339
CAD File
Sheet S203

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231

REINFORCING STEEL SPLICE LENGTHS FOR STANDARD NON-COATED BARS							
CONCRETE STRENGTH F'c	TYPE #1 SPLICE CLASS A SPLICE		TYPE #2 SPLICE CLASS B SPLICE		TYPE #3 SPLICE CLASS B SPLICE		TYPE #3 SPLICE CLASS B SPLICE
	#6 AND SMALLER	#7 AND LARGER	#6 AND SMALLER	#7 AND LARGER	#6 AND SMALLER	#7 AND LARGER	#4 AND LARGER
3,000 psi	44 Bd	55 Bd	57 Bd	71 Bd	85 Bd	107 Bd	30 Bd
4,000 psi	38 Bd	47 Bd	49 Bd	62 Bd	74 Bd	92 Bd	30 Bd

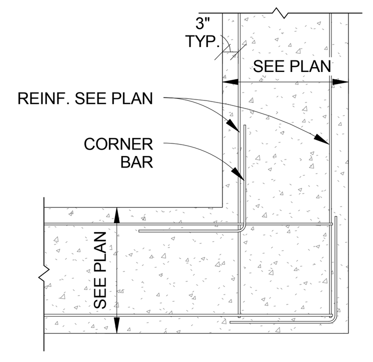
Bd= BAR DIAMETER

NOTES:

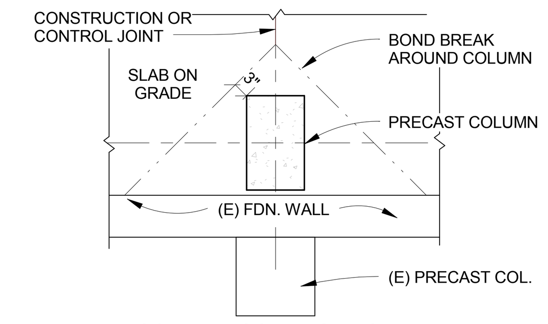
1. MIN. LAP: 18" FOR TYPE #1 THRU TYPE #3 & 12" FOR TYPE #4 SPLICES.
2. REQ'D. SPLICE LENGTH = LISTED SPLICE LENGTH X ADJUSTMENT FACTORS ADJUSTMENT FACTORS = 1.0 IF NONE BELOW APPLY
   A. FOR HORIZ. REINFORCING W/ MORE THAN 12" OF FRESH CONCRETE PLACED BELOW BAR - ADJUSTMENT FACTOR = 1.3
   B. FOR Fy OTHER THAN 60 KSI - ADJUSTMENT FACTOR = Fy (USED)/ 60
   C. FOR LIGHT WEIGHT CONCRETE - ADJUSTMENT FACTOR = 1.3
   D. TYPICAL EPOXY COATED REINFORCING-ADJUSTMENT FACTOR = 1.2
   E. EPOXY COATED REINFORCING W/ COVER LESS THAN Bd OR CLEAR SPACING LESS THAN 6 Bd - ADJUSTMENT FACTOR = 1.5
3. ALL ADJUSTMENT FACTORS THAT APPLY SHALL BE USED TO CALCULATE REQ'D SPLICE LENGTH.

CONCRETE REINFORCEMENT CLEAR COVER, U.N.O. (NON-PRESTRESSED)	
CONDITION & DESIGNATION	CLEAR COVER
CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THRU #18	2"
CONCRETE EXPOSED TO EARTH OR WEATHER: #5 OR SMALLER	1-1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	
SLABS, WALLS, JOISTS: #14 & #18	1-1/2"
SLABS, WALLS, JOISTS: #11 & SMALLER	3/4"
BEAMS, COLUMNS (PRIMARY REINF., TIES, STIRRUPS, SPIRALS)	1-1/2"

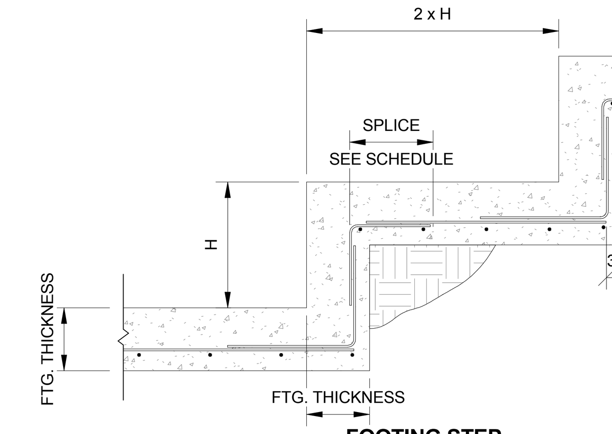
REBAR CLEAR COVER SCHEDULE



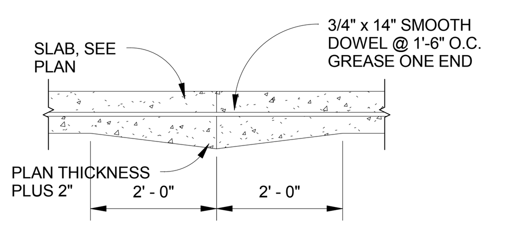
FOOTING CORNER



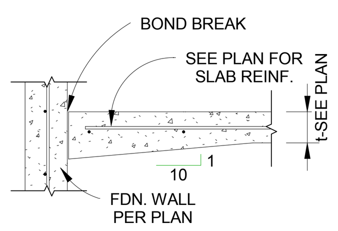
COLUMN ISOLATION JOINT



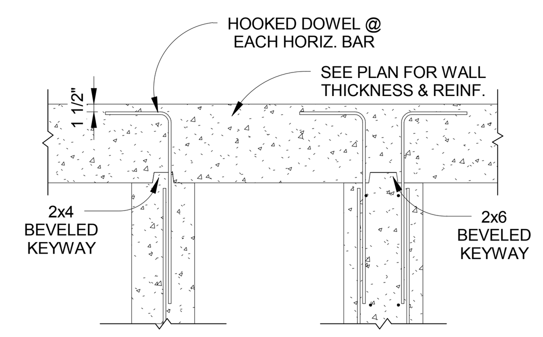
FOOTING STEP



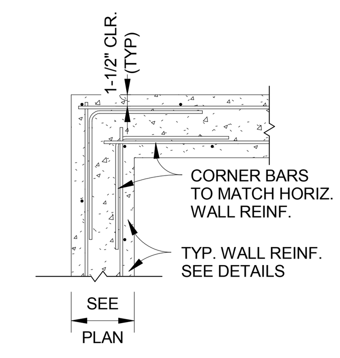
SLAB CONSTR. JOINT



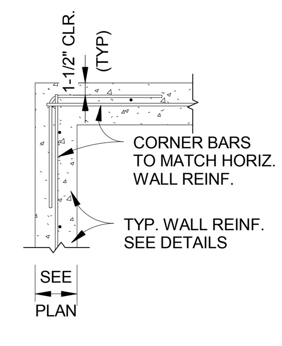
SLAB THICKENED EDGE



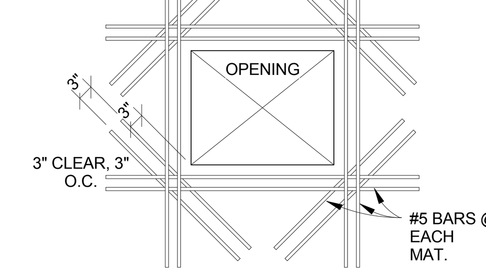
WALL INTERSECTION



WALL CORNER

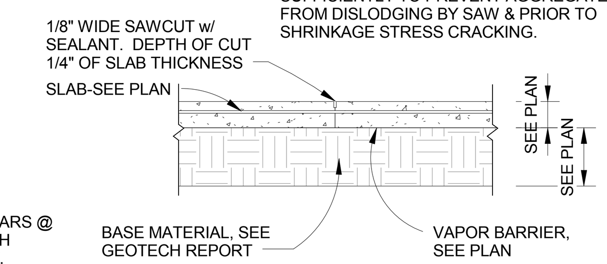


WALL CORNER

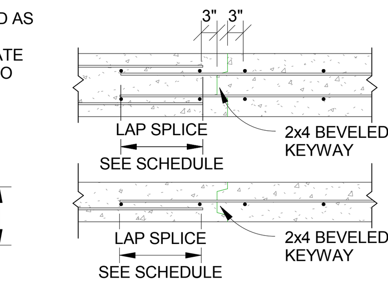


CORNER BARS @ WALL OPENING

NOTES:
1. SAWCUT JOINT SHALL BE EXECUTED AS SOON AS CONCRETE IS HARDENED SUFFICIENTLY TO PREVENT AGGREGATE FROM DISLODGING BY SAW & PRIOR TO SHRINKAGE STRESS CRACKING.

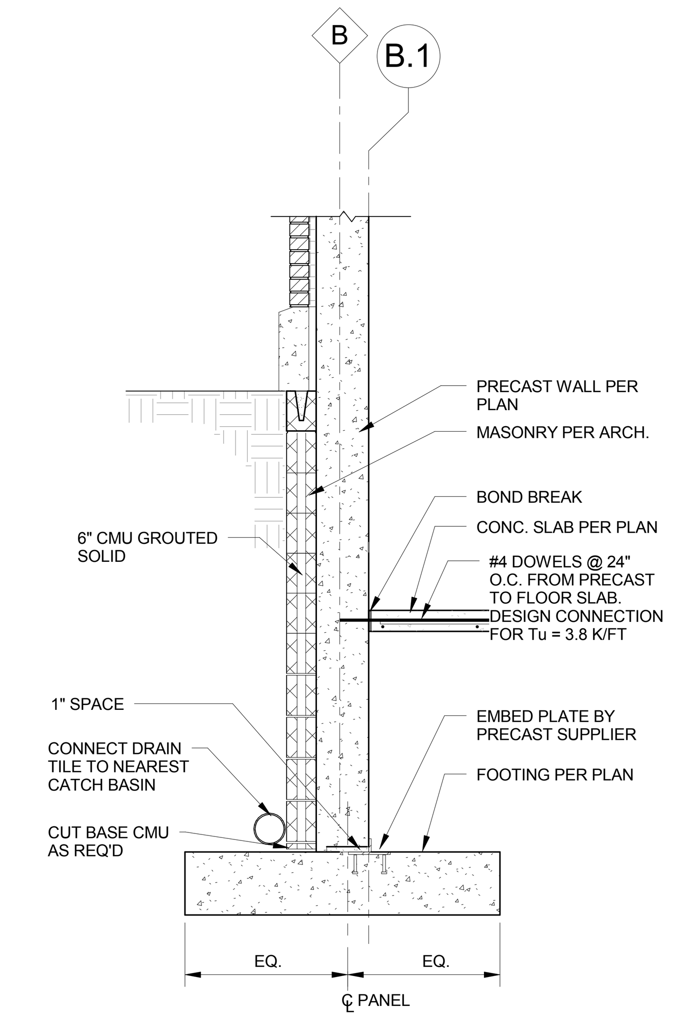


TYPICAL SAW JOINT

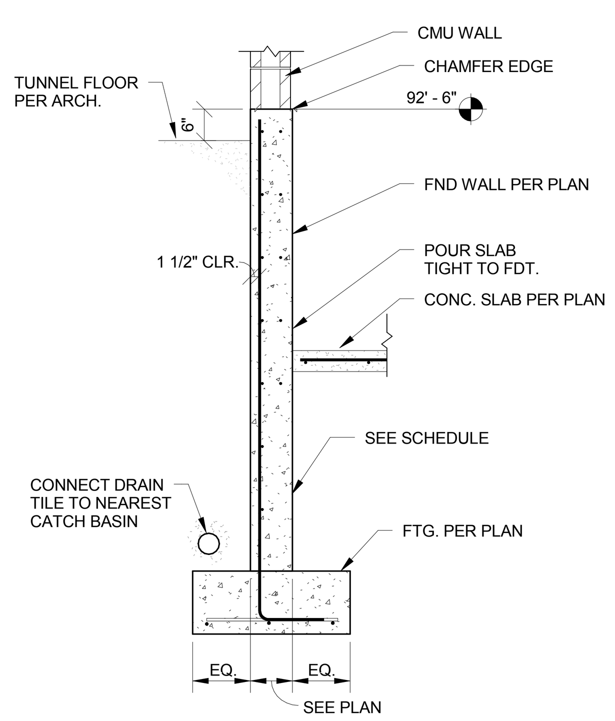


WALL CONSTRUCTION JOINT

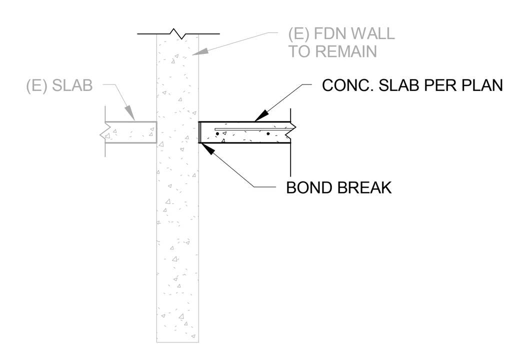
1 TYP. CONCRETE FOUNDATION DETAILS
S300 1/2" = 1'-0"



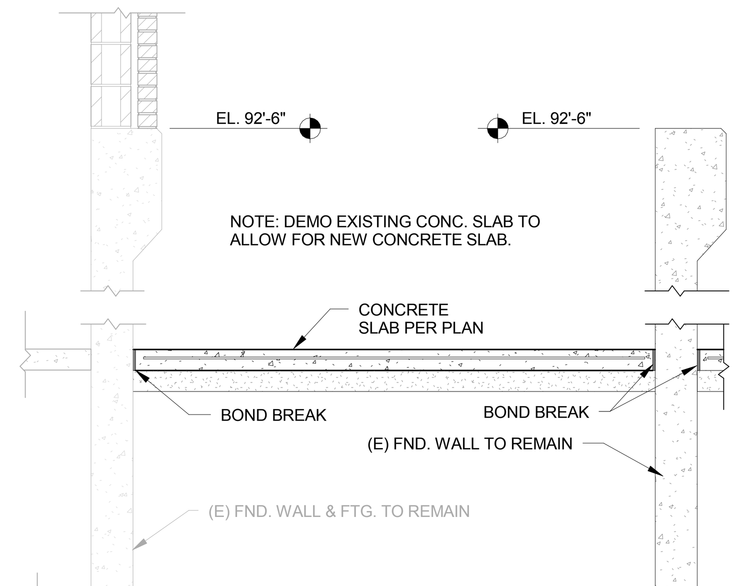
2 TYP. FOUNDATION
S300 1/2" = 1'-0"



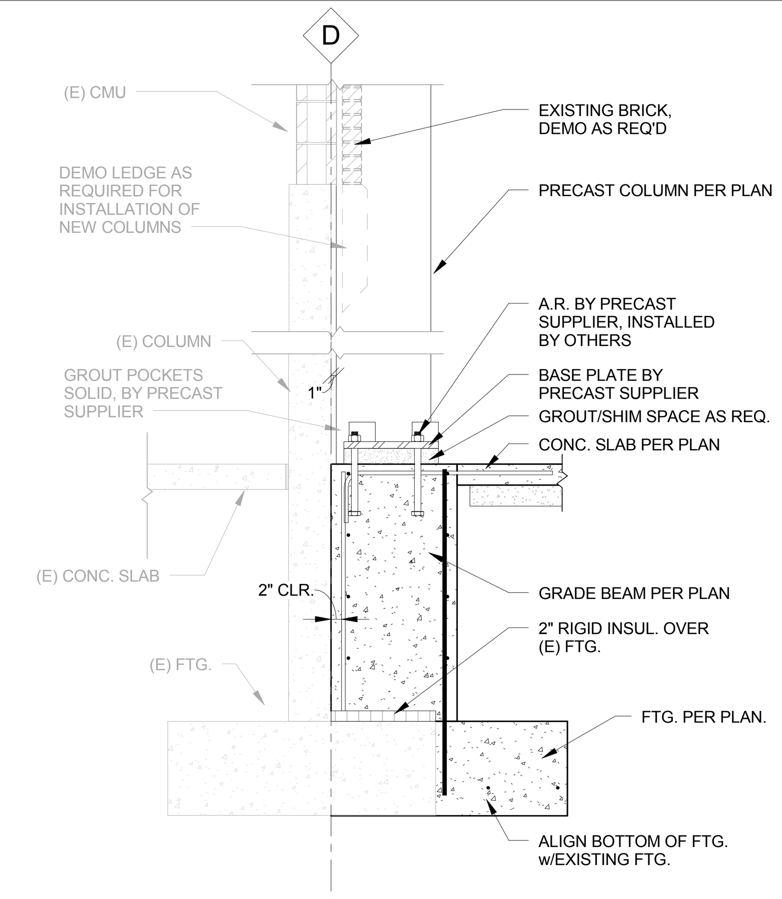
3 SECTION @ (E) TUNNEL
S300 1/2" = 1'-0"



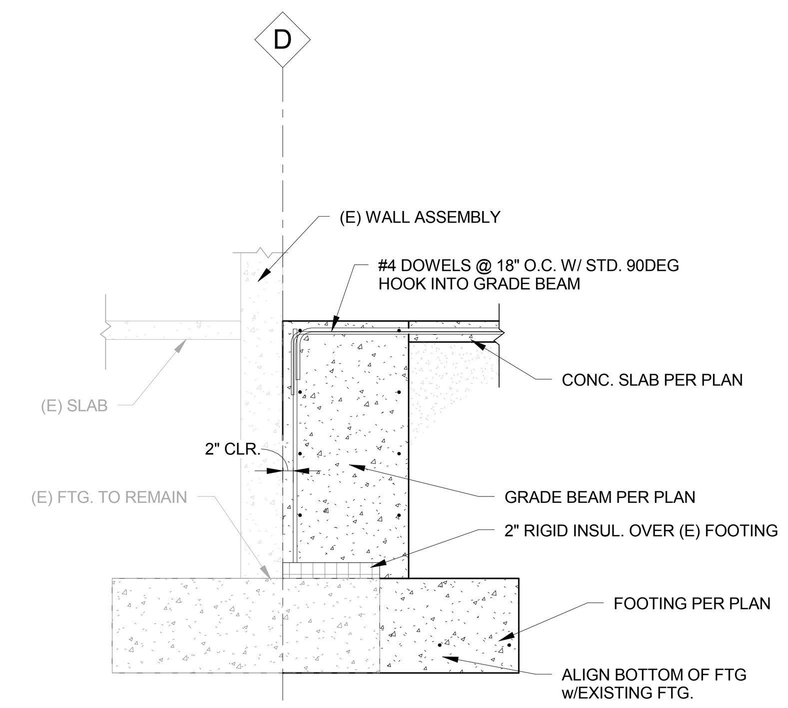
4 NEW SLAB @ EXIST.
S300 1/2" = 1'-0"



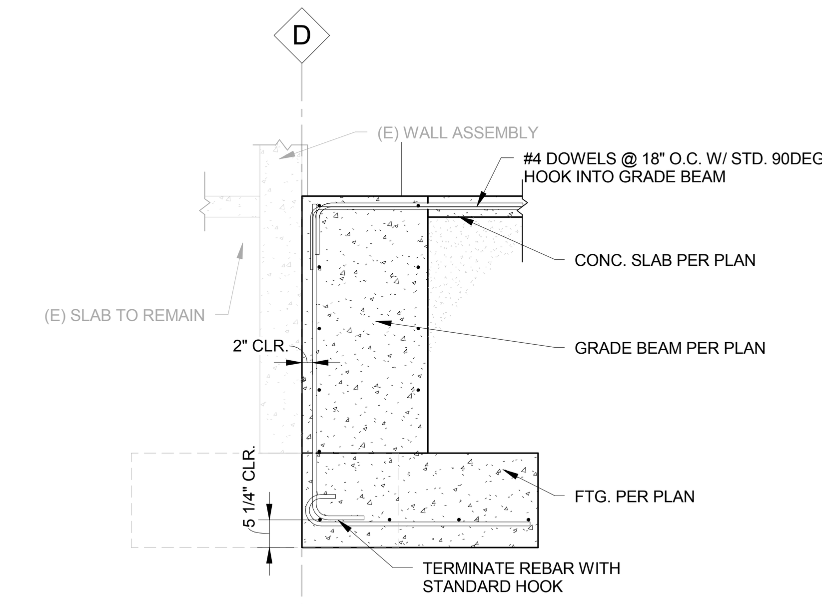
5 NEW TO EXIST. @ INTERIOR
S300 1/2" = 1'-0"



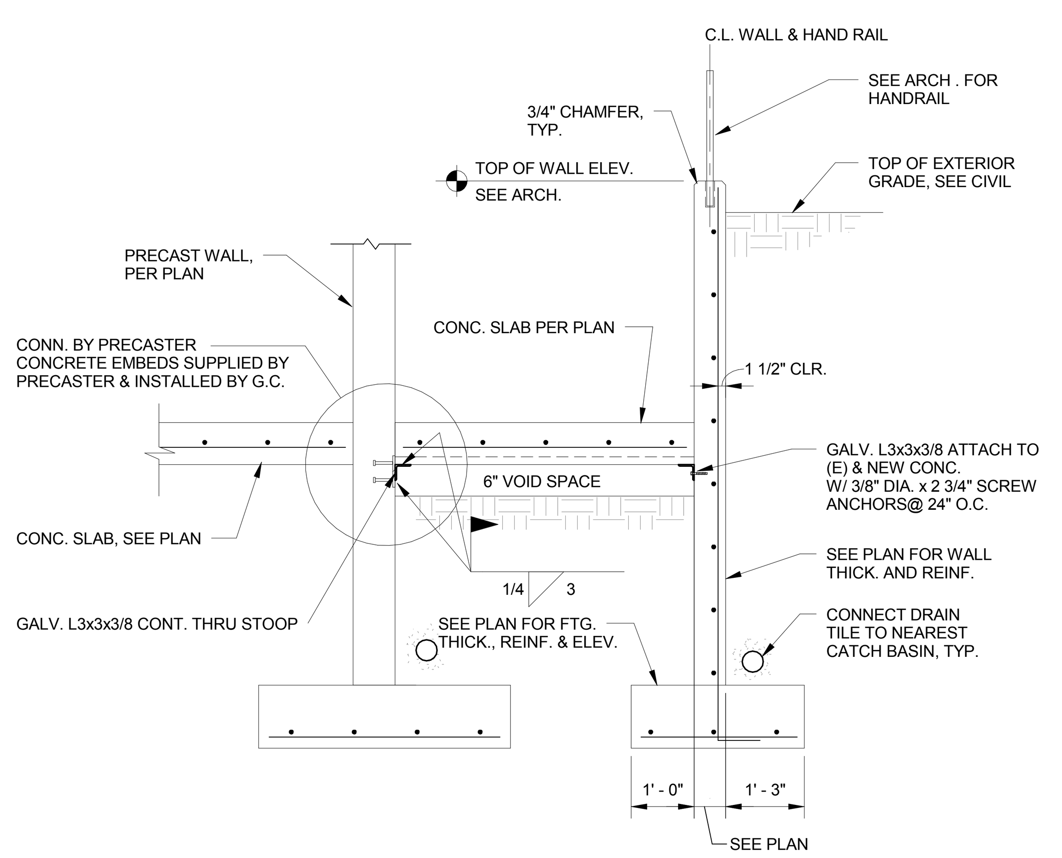
6 NEW COLUMN @ EXIST.
S300 1/2" = 1'-0"



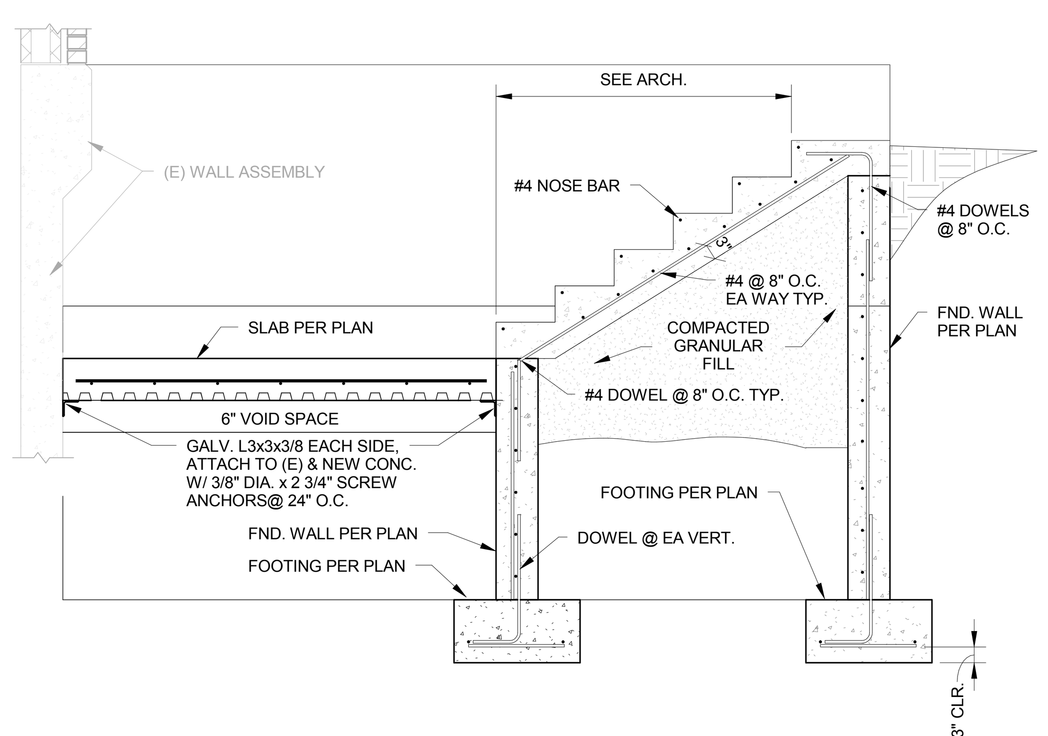
7 GRADE BEAM @ EXIST. FTG.
S300 1/2" = 1'-0"



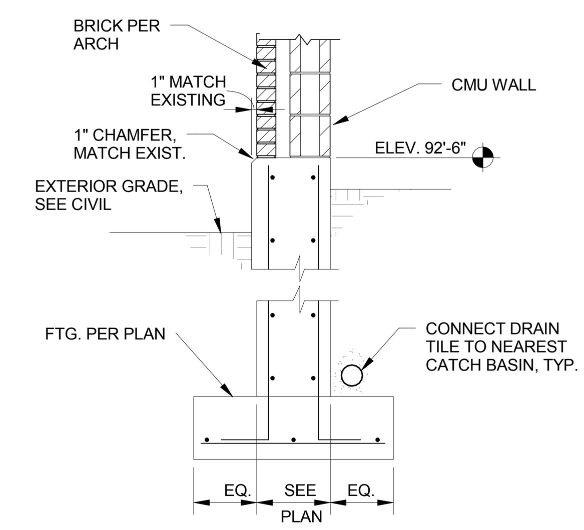
8 GRADE BEAM-TYP.
S300 1/2" = 1'-0"



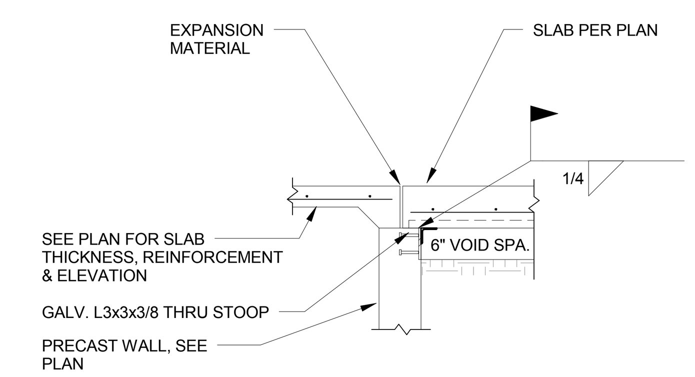
9 STOOP SECTION
S300 1/2" = 1'-0"



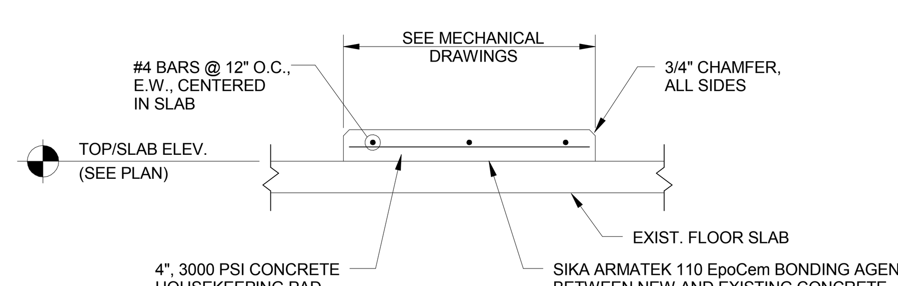
10 STOOP/ STAIR SECTION
S300 1/2" = 1'-0"



11 SECTION AT (E) TUNNEL
S300 1/2" = 1'-0"

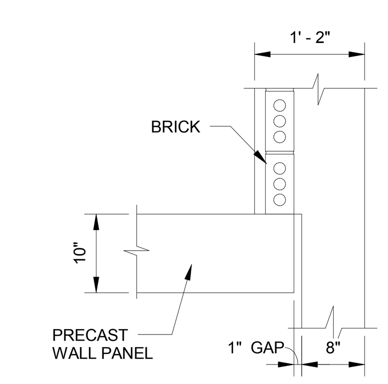


12 LEV_0-SLAB AT WALL
S300 1/2" = 1'-0"



REFER TO MECHANICAL DRAWINGS FOR LOCATIONS OF NEW HOUSEKEEPING PADS. CONSTRUCT PADS 6" LARGER THAN EQUIPMENT EACH DIRECTION

13 HOUSEKEEPING PAD
S300 3/4" = 1'-0"



14 INTERSECTION DETAIL
S300 3/4" = 1'-0"

No	REVISION	DATE

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1301 Highway 29 North
Alexandria, MN 56308
p 320.762.3902
402 21st Street South
Moorhead, MN 56560
p 218.227.002
email: contact@sc-pc.com

JLG architects
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525 Broadway Street
Alexandria, MN 56308
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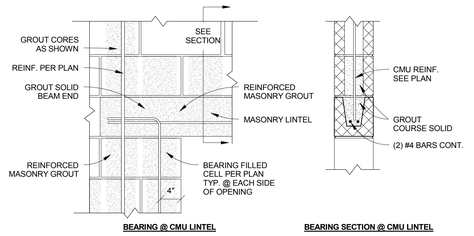
MN 44104
JOE NICHOLSON, PE REG. NO.
DATE: 12-18-2013

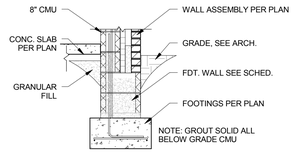
DRAWING TITLE: BUILDING 28 FOUNDATION DETAILS
APPROVED: PROJECT DIRECTOR DATE:
PROJECT TITLE: RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM
BUILDING No 28/9 | CHECKED BY JN | DRAWN KK
LOCATION: ST. CLOUD VA MEDICAL CENTER ST. CLOUD, MN 56303
DATE 12.20.13
PLOT SCALE AS NOTED
PROJECT NO. 656-12-339
CAD FILE
DRAWING NO. S300

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

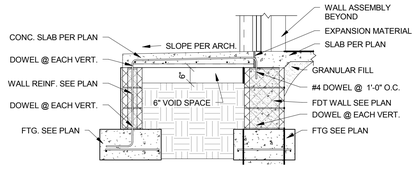
VA FORM 08-6231



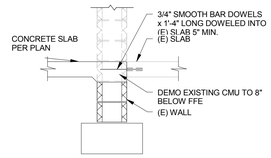
1 S301 TYP. CMU LINTEL BRG.
1" = 1'-0"



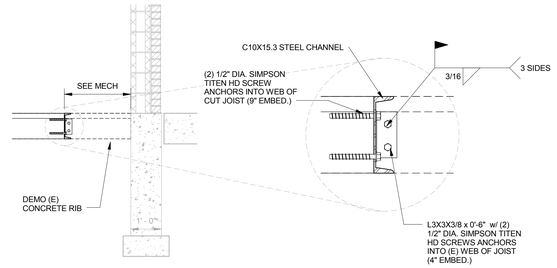
2 S301 TYP. FOUNDATION
1/2" = 1'-0"



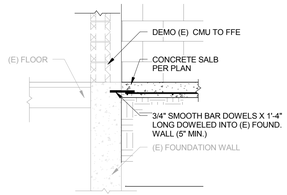
3 S301 STOOP/DOORWAY
1/2" = 1'-0"



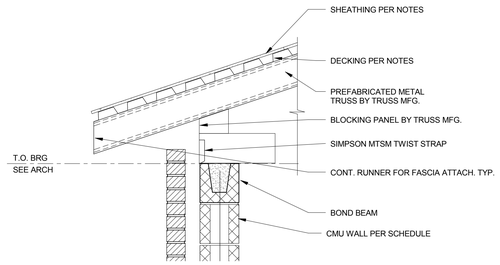
4 S301 NEW SLAB AT EXISITNG
1/2" = 1'-0"



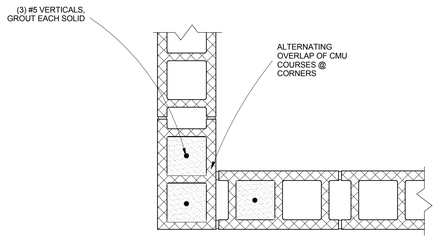
5 S301 Floor Opening-Mech
1/2" = 1'-0"



6 S301 NEW SLAB @ EXISTING FOUNDATION
1/2" = 1'-0"

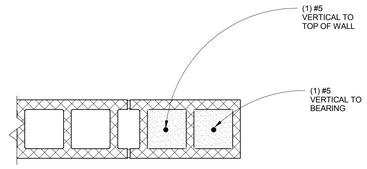


7 S301 02 TYP BRG
1" = 1'-0"



REINF. SECTION

"B"



FILLED CELL @ MASONRY OPENING

"C"

8 S301 22 TYPICAL FILLED CELL
1 1/2" = 1'-0"

No	REVISION	DATE

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1301 Highway 23 North
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Moorhead, MN 56560
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JOE NICHOLSON, PE REG. NO. MN 44104
DATE: 12-18-2013

DRAWING TITLE
BUILDING 9 FOUNDATION & FRAMING DETAILS

APPROVED: PROJECT DIRECTOR DATE

RECONFIGURE/EXPAND BUILDINGS 28 &9 FOR RESIDENTIAL REHABILITATION TREAMENT PROGRAM

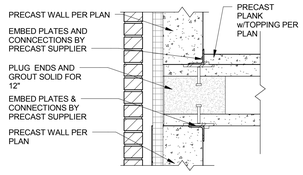
BUILDING No. | DRAWN BY: JN | CHECKED BY: KK

ST. CLOUD VA MEDICAL CENTER
Enter address here

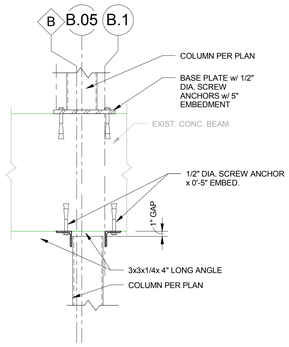
10.11.13
AS NOTED
656-12-339
CAD FILE
SHEET S301

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

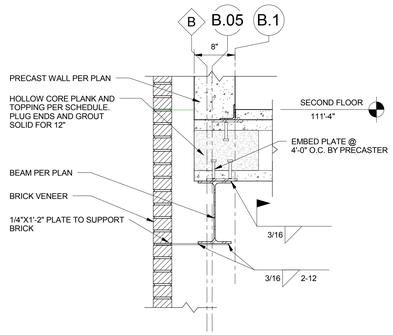
VA FORM 08-6231



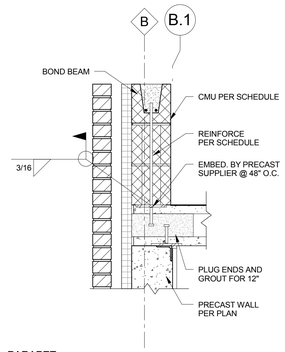
1 S400 PRECAST BRG. @ WALL
1" = 1'-0"



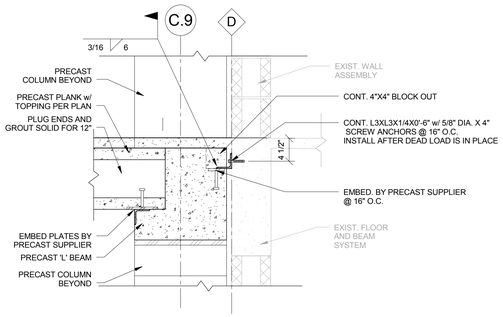
2 S400 COL. BRG @ EXIST. BEAM
1" = 1'-0"



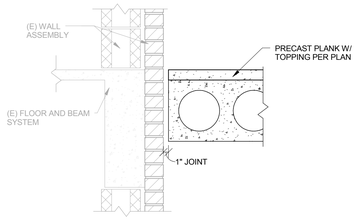
4 S400 BEAM BRG.
1" = 1'-0"



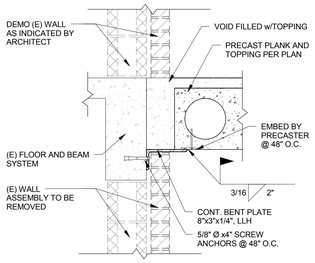
5 S400 PARAPET
1" = 1'-0"



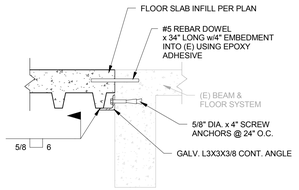
6 S400 PRECAST @ EXISTING
1" = 1'-0"



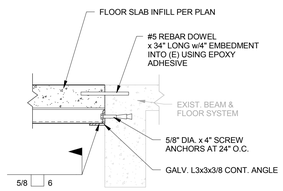
7 S400 NEW FLR. PARALLEL TO EXIST.
1" = 1'-0"



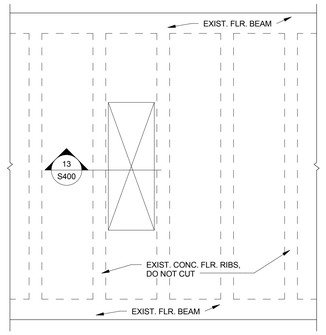
8 S400 NEW PRECAST @ EXIST.
1" = 1'-0"



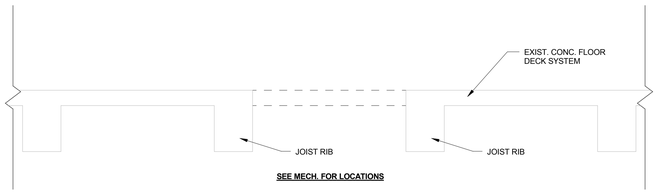
10 S400 FLOOR INFILL SIDE
1" = 1'-0"



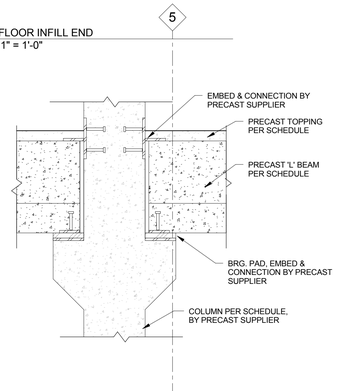
11 S400 FLOOR INFILL END
1" = 1'-0"



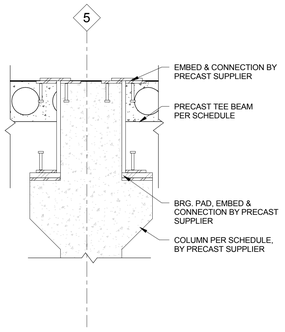
12 S400 TYP. FLOOR OPENING @ EXSIT.
1/2" = 1'-0"



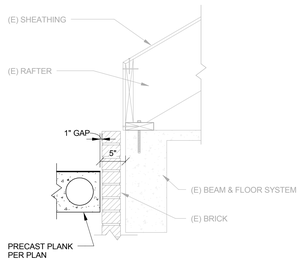
13 S400 SECTION FLOOR OPENING @ EXSIT.
1 1/2" = 1'-0"



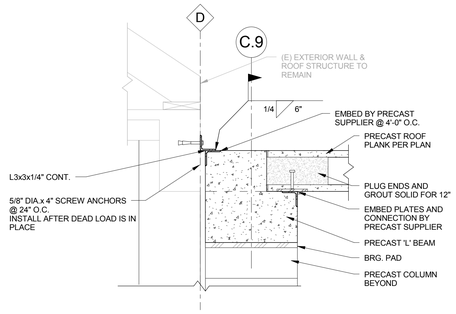
14 S400 INTERIOR PRECAST COL. BRG
1" = 1'-0"



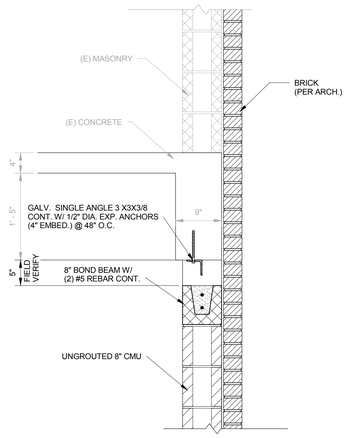
15 S400 PRECAST COL. BRG @ ROOF
1" = 1'-0"



16 S400 LEV_4 TYP EAVE @ ATTIC MECH.
1" = 1'-0"



17 S400 NEW ROOF AT EXIST.
1" = 1'-0"



18 S400 NEW WALL AT EXIST.
1" = 1'-0"

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DATE: 12-18-2013

APPROVED: PROJECT DIRECTOR DATE

DRAWING TITLE: BUILDING 28 FRAMING DETAILS

RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

12.20.13
AS NOTED
656-12-339

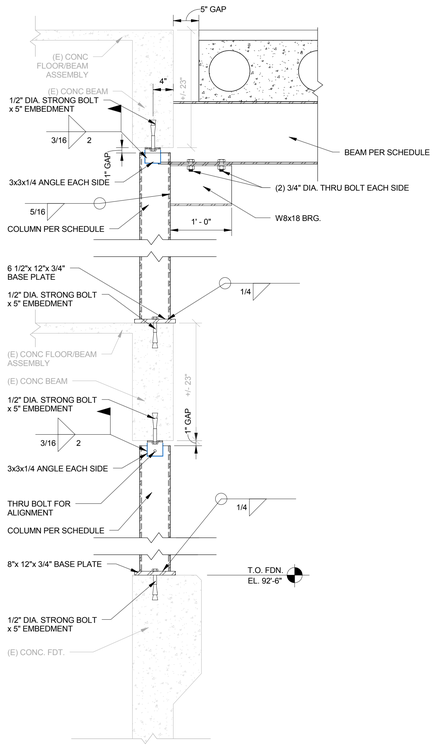
BUILDING NO. 28/9 | DRAWN BY JN | CHECKED BY KK | CAD FILE

PROJECT: ST. CLOUD VA MEDICAL CENTER ST. CLOUD, MN 56303

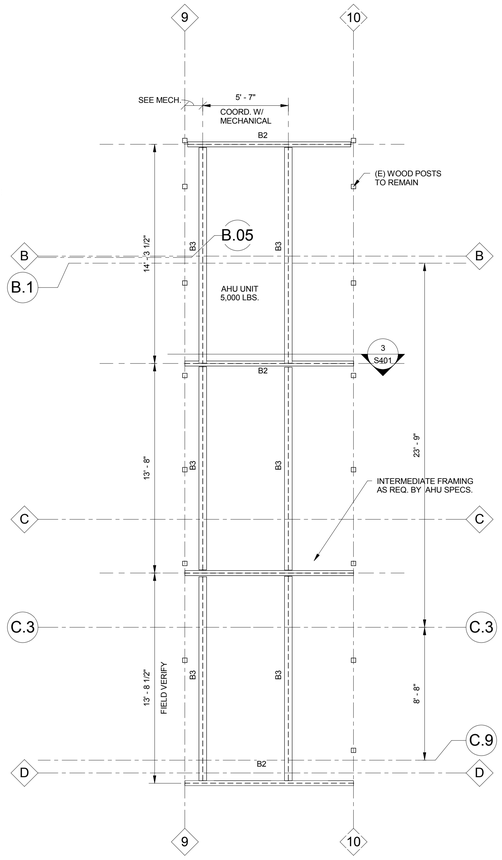
SHEET: S400

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231



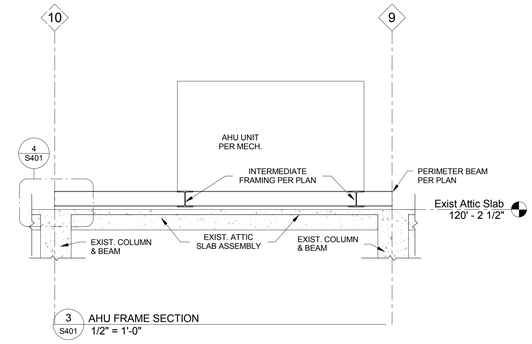
1 S401 INT. COLUMN @ EXIST.
1" = 1'-0"



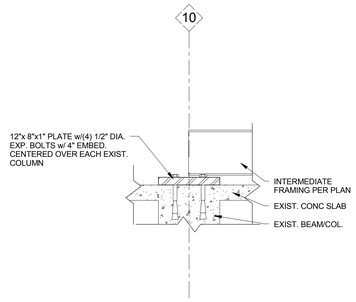
2 S401 AHU SUPPORT
1/4" = 1'-0"

COLUMN SCHEDULE	
Type	Description
4x4	
C1	18" DEEP x 12" WIDE PRECAST (BY OTHERS)
C2	HSS 6x6x1/2"

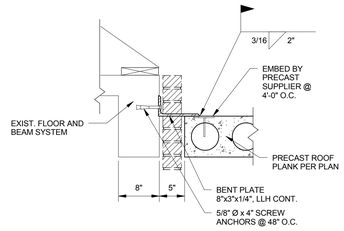
BEAM SCHEDULE	
Type	Description
4HC8	
B1	1'-6" x 1'-8" PRECAST 'L' BEAM
B2	W6x9
B3	W6x15
B4	W12x26, T.O.S. = 110'-2" W/ 1/4"x16" BOTTOM PLATE



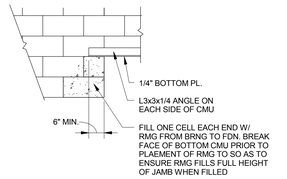
3 S401 AHU FRAME SECTION
1/2" = 1'-0"

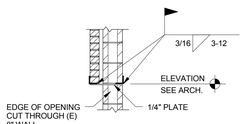


4 S401 AHU FRAME CONNECTION
1 1/2" = 1'-0"



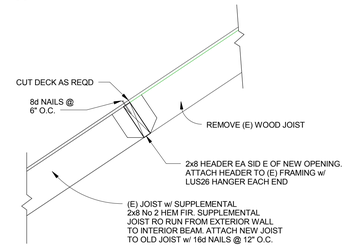
6 S401 NEW PRECAST @ EXIST. ROOF
1" = 1'-0"





NOTE:
FINISH OF OPENING MUST MEET ARCHITECT'S REQUIREMENTS

7 S401 PINCH ANGLE DETAIL
1/2" = 1'-0"



5 S401 LOUVER HEADER DETAIL
1" = 1'-0"

No	REVISION	DATE

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Project Number

STAMP/SEAL:
I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

JOE NICHOLSON, PE MN 44104 REG. NO.
DATE: 12-18-2013

DRAWING TITLE: BUILDING 28 FRAMING DETAILS

APPROVED PROJECT DIRECTOR: DATE:

PROJECT TITLE: RECONFIGURE/EXPAND BUILDINGS 28 & 9 FOR RESIDENTIAL REHABILITATION TREATMENT PROGRAM

ST. CLOUD VA MEDICAL CENTER
ST. CLOUD, MN 56303

DATE	SCALE
12.20.13	AS NOTED
PROJECT NO. 656-12-339	
DRAWN BY JN	CHECKED BY KK
CAD FILE	DRAWING S401

St. Cloud VA Health Care System
Brainerd | Montevideo | Alexandria

VA FORM 08-6231