<u>GENERAL NOTES</u> **DESIGN PARAMETERS:** 1. Design Codes - (All latest editions unless noted): A. International Building Code (IBC 2006) B. American Society of Civil Engineers (ASCE 7-05) Minimum Design Loads for Buildings and Other Structures 2. Foundation A. Drilled Piers on competent shale are designed for a maximum soil bearing pressure of 10,000 psf for drilled piers, based on a report by Terracon Consulatants dated November, 2011. Four (4) feet of soil below buildings shall be undercut and backfilled with compacted structural fill per soils B. If the soil is of questionable bearing value, the Engineer or Architect shall be notified immediately. After footing excavations are completed and before placing concrete, the excavated areas shall be inspected and approved by the Owner selected independent testing laboratory D. The Soils Engineer is the sole judge of suitability of underlying material to support foundations and shall approve bearing material before foundation installation. See specifications. E. Coefficient of horizontal friction between concrete and soil = 0.35 F. Minimum depth from exterior ground surface to bottom of foundations = 24 inches. G. Prepare site and place fill in accordance with the recommendations in the soils report noted above. Observe construction recommendations noted in the soils report. All fill material shall be in accordance with soils report recommendations. H. Construct non-basement floor slabs on the granular fill layer required by the plan notes. The granular fill shall be placed on non-expansive soil which has been place in accordance with the soils report. I. Backfill basement and retaining walls with ASTM C-33 No. 57 stone or equivalent approved by the soils Engineer. Extend stone from the base of walls outward at a 45 degree angle to the vertical J. Backfilling: - Do not backfill basement walls and grade beams until bracing floors are in place or temporary bracing is installed. Backfill in even lifts alternating from side to side. Backfill under foundations with concrete or as approved by soils Engineer. Future Roof Loads: A. Future Roof Dead Loads.. ..20 psf B. Future Roof Live Load.. ..20 psf 4. Wind Load: A. Wind Loads (IBC 2006) B. Wind Speed.. ..90 mph Wind Exposure Category. Wind Importance Factor.. ..1.0 Snow Load: A. Snow Load (IBC 2006) .10 psf B. Ground Snow Load... C. Exposure Coefficient Ce. ..0.90 D. Thermal Factor Ct..1.1 Importance Factor for Snow I. ...1.0 Roof Slope Factor Cs. 10 G. Roof Snow Load.. . Pf= 7 psf (use 10 psf Min) 6. Floor Loads: A. Floor Dead Loads. .51 psf ..100 psf B. Floor Live Loads.. C. Floor MEP Allowance. ..15 psf D. Floor Partition Load... ..20 psf Seismic Loads (IBC 2006): 0.2 Sec Spectral Acceleration ..Ss = 0.514 0.2 Sec Site CoefficientFa = 1.190 0.2 Sec Design Acceleration ..Sds = 0.410 1.0 Sec Spectral Acceleration ..S1 = 0.1651.0 Sec Site CoefficientFv = 1.63 1.0 Sec Design Acceleration . ..Sd1 = 0.180 Site Class.... Occupancy Category... Seismic Importance Factor.. AISC Seismic Provisions. ..Not Applicable Seismic Design Category. Basic Structural System... .Steel Systems Not Specifically Detailed for Seismic Resistance Seismic Force Resisting System ..R=3 Response Modification Factor. Deflection Amplification Factor.. ..Cd=3 Analysis Procedure... ...Equivalent Lateral Force Procedure Non-Structural Component Seismic Exemption: A. Architectural Components: Architectural components must comply with seismic requirements of Chapter 13 ASCE 7. (SDC=C) B. Mechanical and Electrical Components Mechanical and electrical components with an Ip= 1.0 are exempt from seismic requirements. Components with Ip>1.0 must comply with seismic requirements. (SDC=C) Mechanical and electrical components with an Ip=1.0 and either components are mounted 4 ft or less above a floor level and weight 400 lbs or less or flexible connections between components and the associated duct work, piping and conduit. Mechanical and Electrical componets with an Ip= 1.0 and the components weight 200lbs or less or distribution systems weighing 5 lb/ft or less. Component Seismic Importance Factor: The component importance factor lp shall be =1.5 if any of the following conditions apply: The component is required to function for the life/safety purposes after an earthquake, including fire protection sprinkler systems. The component contains hazardous materials. The structure is in or attached to an occupancy category IV structure and it is needed for the continued operation of the facility. I hereby certify that the structural plans submitted herewith are designed with the structural load carrying elements to resist the anticipated forces of the designated seismic zone in which the structure is located in accordance with Arkansas Code Annotated 12-80-101 et. seg. Date: JANUARY 31, 2014 Casy nomen Casey Daniel, P.E. Arkansas Registration No. 12371 **GENERAL INFORMATION:** All columns shall be centered on grid lines unless noted otherwise. All column footings shall be centered on columns unless noted otherwise. All wall footings shall be centered on walls unless noted otherwise. 3.All wall footings shall be centered on walls unless noted otherwise. 3.All wall footings shall be centered on walls unless noted otherwise. Unless otherwise noted or detailed, concrete pads for mechanical equipment shall be 4" thick (minimum) and reinforced with #3 @ 12" OC each way centered. Substitution of expansion anchors for embedded anchors shall not be permitted, Unless Approved by Engineer. Contractor is responsible for coordinating weights, size, and location of actual mechanical units ordered. Unless Directed Otherwise By Geotechnical Engineer all fill material under structure shall be sandy clay or clayey sand exhibiting a liquid limit less than 35. Fill material shall be placed in loose lifts not to exceed 8" and compacted to a density of not less than 95% of Modified Proctor Maximum Dry Density (ASTM D-1557) at or slightly wet of optimum moisture content. In place moisture and density of each lift shall be determined by in-situ field tests prior to placing additional fill. Permanent stability of the building and components is not provided until the erection is completed as shown on the contract drawings. Erection stability and temporary supports required for construction including guys, braces, and shoring are the responsibility of the contractor. Testing: A. Refer to specifications for specific requirements regarding sampling and testing. B. Where sampling and testing requirements are omitted from the specifications sample and test concrete as follows: B1. Contractor shall engage a testing laboratory acceptable to the owner and Architect. Test conducted shall be paid for by the contractor. B2. Prepare field samples of 4 compressive test cylinders in accordance with ASTM C31 and one slump test for each class of concrete placed each day. Samples shall be taken not less than once per day for each 50 cubic yards of concrete. Test for cylinders shall be conducted one at 7 days and 2 at 28 days, with remaining cylinders retained for future testing in case of low test results. **CAST-IN-PLACE CONCRETE:** Minimum Concrete Compressive Strengths: A Footings f'c = 3,000 psi at 28 days. Max w/c=0.58 B Interior Slabs-On-Grade f'c = 4,000 psi at 28 days. Max w/c=0.45 C Ext Exposed Conc (Air Entrained) fc = 3,500 psi at 28 days. Max w/c=0.48 Before concrete is placed reinforcement shall be secured against displacement within tolerances permitted in 2 section 7.5.2 of ACI code. Where lap splices are required of deformed bars and not specifically indicated on drawings, splices shall be 3 class B splice. When bars of different size are lap spliced, the splice length shall be based upon larger bar. Concrete protective covering for reinforcement at surfaces not exposed directly to the ground shall be 3/4" for slabs, joists, and walls and 1 1/2" for beam stirrups and column ties or spirals. Do not "wet stick" dowels. The following minimum concrete cover shall be provided: A Concrete cast against and permanently exposed to earth 3" B Concrete exposed to earth or weather - No. 6 thru No. 18: 2" Concrete exposed to earth or weather - No. 5 and smaller: 1 1/2" D Concrete not exposed to earth or weather: slabs, walls, joists No.14 and No. 18: 1 1/2 slabs, walls, joists No.11 and smaller: 3/4" Location and sizes of openings, sleeves, etc. required for other trades must be verified by these trades before placing concrete. Contractor is responsible for "means and methods" of construction and shall provide adequate shoring to prevent collapse or damage to structural elements during construction. DESCRIPTION DATE NO APPROVED: MEDICAL CENTER DIRECTOR APPROV APPROV PROVED: ASSOCIATE DIRECTOR

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APPRO

CONCRETE SLAB ON GRADE:

- Provide a 4-inch clean medium-to-coarse gravel compacted drainage fill below all interior slabs-on-grade unless noted or detailed
- otherwise. A 10-mil minimum polyethylene film vapor retarder shall be placed below all interior slabs-on-grade.
- Welded wire fabric or deformed rebar shall be cut 3 inches on either side of a sawcut or construction control joint. Provide bolsters or supports as needed to maintain reinforcement at proper location in slab.
- Maximum water cement ratio shall not exceed the amount specified. Saw cutting control joints shall proceed as soon as possible without chipping or spalling concrete. Lapsed time between casting and sawcutting shall not exceed 8 hours.

EXPANSION JOINT FILLER:

requirements of ASTM D1751.

- Non-extruding premolded material composed of fiberboard impregnated with asphalt conforming to the
- **EPOXY ANCHORS:**
- Where epoxy anchorage of threaded rods and rebar is approved by Engineer of record in concrete filled cells of cmu, use HILTI HY150 1 epoxy. Where epoxy anchorage of threaded rods or rebar is approved by Engineer of record in cast-in-place concrete, use HILTI RE500. Follow Manufacturers instructions for installation of holes. Unless depth of embedment is not shown on contract drawings contact engineer of record for depth of embedment. As a minimum depth of embedment shall be indicated by manufacturer to develope full tensile strength of anchorage. Where anchorage is required into hollow masonry contractor shall use HILTI HIT HY20 for masonry construction.

WATERSTOPS:

Provide flexible strip construction joint waterstop RX-101 as manufactured by volclay, or approved equal. Follow manufacturers directions with respect to storage and application of WB-Adhesive.

LIGHT GAUGE STUDS & MISC FRAMING:

- Light gauge structural studs shall be as follows:
- A. Exterior non-load bearing wall framing = See Schedule (600S162-43 @ 16" OC Min.) B. Interior non-load bearing partition studs = See Architectural drawings for size.
- C. For stud and track gages greater than and equal to 16GA, Fy=50ksi All stud tracks shall match stud gage but not less than 18 ga. Tracks above or below glass curtain walls greater than 10 feet in height shall be a minimum of 16 ga. All window and door openings in load bearing walls are to have double joist headers with double studs each end for bearing.
- All walls shall have horizontal bridging spaced at intervals not exceeding 4'-0" OC maximum.
- Non-load bearing walls shall be provided with slip-type support clips (deflection clips) to allow adjacent support deflection without damaging For multi-story projects or projects with more than a single gage of stud, studs shall have ends color coded or size and gage permanently
- labeled on studs Load bearing studs shall be squarely seated against top and bottom tracks with gap not exceeding 1/8". Fasten both flanges of stud to top and
- bottom tracks. Install supplementary framing, blocking, in stud framing as indicated in the Architectural drawings to support fixtures, equipment, services,

casework, heavy trim, furnishings, and similar work requiring attachment to framing. If supplementary support is not indicated comply with stud

- manufactures recommendations and industry standards in each case. Tracks bearing on concrete shall be attached with a minimum 0.145" pdf @ 8" OC or as noted on drawings.
- At jambs, heads and sill of curtainwall window systems provideminimum 16GA support members unless detailed otherwise

STEEL JOIST AND JOIST GIRDERS

- Erect in accordance with AISC, SJI, OSHA and project specifications. Do not erect damaged joists. obtain corrective procedures from manufacturer. Do not field cut, drill or modify joists.
- Provide and anchor bridging per SJI, OSHA, where shown, note and between pair of joist in any series. Connections to supports:
- -Weld to steel supports per SJI, unless noted. Anchor to other supports per SJI unless noted otherwise. Bolt members on column centerlines to steel supports using A325 bolts. Bolt diameter per SJI specifications. provide threaded studs where type of support would prevent through bolting.
- Locate pipe and equipment hangers and other concentrated loads only where loads are shown on joist shop drawings. Attachment method as approved by joist supplier where pipes and equipment are suspended from joist bottom chord. Slope bearings to provide full contact with supports.
- Provide clip angles, plates, etc., shown shop welded to members. Coordinate with structural steel fabricator.

NON-COMPOSITE STEEL FLOOR DECK

- See plan for size and required gage Steel deck shall be galvanized unless noted otherwise.
- Minimum end bearing shall be 1 1/2".
- Attach deck to supporting members with 4-welding washers minimum per panel and 4-#10 tek screw sidelap fasteners minimum per panel unless noted otherwise. At contractor's option power driven mechanical fasteners may be substituted for welds. Contractor shall submit manufacturer's data for size of
- fastener and spacing for approval. Refer to the steel deck institute (SDI) standards for installation, storage, and all other construction practices.
- Contractor shall place bolsters to support WWF reinforcement over each support (joist or beams). Bolsters shall provide 3/4" clearance from top of concrete. Testing and Inspection: Unless specifically stated otherwise in specifications, contractor shall engage a testing laboratory acceptable to owner and Architect to provide inspections for proper attachment and installation.

STEEL ROOF DECK:

- Deck type and minimum steel thickness as noted on plans.
- The deck shall be fastened to supporting steel in accordance with the manufacturer's instructions. Minimum deck fastening shall be as follows: Five- 5/8" round puddle welds at ends of panel and at each intermediate support. Side lap seams to be fastened with #12 TEKS at 18" OC. Long panel sides at support angles to be fastened with 5/8" round puddle welds at 12" OC along full length of panel.

STRUCTURAL STEEL

connections.

1		Steel shape and plate materials:
		W Shapes - ASTM A992 or A572 Grade 50
		Pipe - A53 - Grade B 35 ksi
		Round HSS - A500 Grade C 42 ksi
		Rectangular HSS - A500 Grade B 46 ksi
		Built-Up shapes - A572 Grade 50
		All Others - A36 or A572 Grade 50
2		The fabrication and erection of structural steel shall comply with "The Code Standard Practice for Steel Buildings and Bridges" as published by AISC
3		Unless detailed otherwise, connections shall comply with the typical connection details indicated on drawings. Where beam and reactions are shown and connection details are not indicated on the structural drawings provide a design for the connection and submit to the structural engineer of record for approval. Where typical connection details and beam end reactions are omitted beam connections shall be selected to support one half the total uniform load capacity indicated in "Allowable Uniform Load Tables" in part 2 of the AISC manual of steel construction, 9th edition.
4		Bolted Connections-
	А	Unless detailed otherwise, all field connections shall be made using 3/4" diameter ASTM A325N high strength bolts. Washers shall be installed under nuts as snug tight connections.
	В	Use slip critical (A325SC) bolts for bracing, moment connections, cantilevers, tension members and at oversized or slotted holes where the force on joint is parallel to long axis of slot. Use A325N elsewhere.
	С	Where specifically identified on the drawings as slip critical all high strength bolts shall be tightened to comply with "slip critical" joints. Specifications are as follows:
		1. Installation of Alternate Design Bolts (Twist off Type)
		2. Direct tension Indicators
	D	A490 and A325 high strength bolts shall not be reused that have been previously tightened.
	F	Unless specifically noted as slip critical connections, all bolted connections shall be visually inspected to comply with snug tight conditions

- E Unless specifically noted as slip critical connections, all bolted connections shall be visually inspected to comply with snug tight conditions. Welded Connections-A Welding of structural steel shall comply with the latest edition of AWS1.1 and all welds including field welds shall be made by certified
- welders using E70 electrodes and must meet CHARPY V-NOTCH requirements. B All fillet welds to be visually inspected. All full penetration welds shall be inspected by ultrasonic means or by other approved methods. C Contractor shall remove back-up bars and run-off tabs projecting into or obstructing installation of building materials. D Fabricator shall cope beams or otherwise provide weld access holes to allow proper installation and use of back-up bars at welded

Steel erector is responsible for providing all necessary temporary bracing during erection.

ED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
'ED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
'ED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

STRUCTURAL ALLOWANCES

- <u> / E REVISIONS</u> anchors, bolts, decking, grating, etc. as directed by the EOR for use in the field.
- At the project closeout, credit all unused amounts of the structural allowance to the Owner by Change Order Credit.
- SPECIAL INSPECTION AND TESTIN

hourly rates.

- walls, and other misc. structural items.
- be used for correcting work
- 4. Claims for extra costs associated with testing and inspection will not be entertained until the entire allowance is first used.

COMPOSITE STEEL FLOOR DECK

See plan for size and required gage.
Steel deck shall be galvanized unless not
Provide pour stops, column closures, cove
Minimum end bearing shall be 1.5".
Attach deck to supporting members with 4 otherwise. At contractors option power dri fastener and spacing for approval.
Where required, use 3/4" ${\ensuremath{\varnothing}}$ stud shear cor (including studs) to composite beams sha
Refer to the steel deck institute (SDI) stan Steel Studs shall have a finished length of
Testing: Unless noted otherwise in specifi inspection of shear studs and to test field and installation.

bolsters over each support. Reinforcing shall be 1" below top of concrete.

FUTURE CONSTRUCTION

This	s building has been designed for additional future construct
fran	ning systems have taken into account future levels. The fo
leve	els. The current design accounts for the following levels:
1.	GROUND FLOOR
2.	1ST FLOOR
3.	2ND FLOOR
4.	3RD FLOOR

OSHA/EPA/State requirements related to regulations applicable to the work.

5. ROOF

HAZARDOUS MATERIAL WARNING

Unexpected Discovery of Asbestos For any previously untested building components suspected to contain asbestos and located in areas to be impacted by the work, samples will need to be collected of any materials found to determine the presence or absence of asbestos. The sampling must be performed by an AHERA accredited Asbestos Building Inspector. The Contractor, and the Contractor's Competent Person, Project Supervisor, and other required personnel shall meet with designated building personnel prior to beginning work at a safety pre-construction meeting to discuss details of site safety, including the preparation of any Accident Prevention Plans (APPs), emergency plans, Asbestos Hazard Abatement Plans (AHAPs) or any other applicable safety documents. Deficiencies of any supplied documents will be discussed and all documents must meet final approval prior to the commencement of work. The Contractor shall incur all costs, including all sampling/analytical costs to assure compliance with

The contractor shall allow for \$25,000 (Twenty Five Thousand Dollars) for any revisions requested by the structural engineer during construction or review of shop drawings. Provide additional misc. steel angles, channels, wide flange shapes, HSS shapes, plates, rods, shear studs, rebar, concrete, bar joists, Allowance to include total cost of work including material costs, labor, installation, taxes, insurance, overhead & profit, and delivery. Claims for extra costs associated with additional structural items will not be entertained until the entire allowance is first used.

The contractor shall allow for \$50,000 (Fifty Thousand Dollars) for Special Inspection Items and Testing Services. Refer to sheet S002 for an outline of required services. This will consist of on-site inspection and testing of slabs, foundations, concrete reinforcing, steel framing and connections, CMU Items found to be deficient or not in compliance with the contract drawings shall be corrected at no cost to the Owner. Structural allowances shall not . The contractor shall pay for Special inspections and testing based on payment requests from the inspection and testing firm incorporating their standard 5. At the project closeout, credit all unused amounts of the allowance to the Owner by Change Order Credit.

oted otherwise over plates, and girder fillers per SDI standards as needed

h 4 - 5/8" puddle welds minimum per panel and 6-welded sidelap connections minimum per panel unless noted

lriven mechanical fasteners may be substituted for welds. Contractor shall submit manufacturers data for size of

onnectors as shown on the plans. Max spacing of studs shall be 36", and max spacing of deck attachments hall not exceed 16". andards for installation, storage, and all other construction practices

of (Deck rib height + 1.5")

cifications, contractor shall engage a testing laboratory acceptable to owner and Architect to provide a visual d welded shear connectors in accordance with AWS D1.1 requirements, and inspect the deck for proper attachment Unless noted otherwise on drawings, provide minimum temperative and shrinkage reinforcement of 6x6-W2.9xW2.9 WWF. WWF shall be supported on

ction. The design of the current lateral and gravity foundations are also designed to support these future

ADD	ADDENDUM
ADDL	ADDITIONAL
ALT	ALTERNATE
AB	
& ANG	AND ANGLE
ARCH	ARCHITECT
@	AT
BP	BASE PLATE
BM	BEAM
BMS BRG	BEAMS BEARING
BOT	BOTTOM
B/	BOTTOM OF/BACK OF
BRDG	BRIDGING
BLDG	BUILDING
CIP	
CLG C OR CL	CEILING CENTER OR CENTERLINE
C/C	CENTER TO CENTER
CLR	CLEAR
COL	COLUMN
CP	COMPLETE PENETRATION
CONC CMU	CONCRETE CONCRETE MASONRY UNIT
CMU	CONCRETE MASONRY UNIT
CONST	CONSTRUCTION
CJ	CONTROL JOINT
CONT	CONTINUOUS
CONTR	
CWA DBA	COORDINATE WITH ARCHITECT DEFORMED BAR ANCHOR
DBE	BECK BEARING ELEVATION
DL	DEAD LOAD
DK	DECK
DEP	DEPRESSED
DET DIAG	DETAIL DIAGONAL
DIAG DIA OR Ø	DIAGONAL
DIM	DIMENSION
DWLS	DOWELS
DN	DOWN
DWG DP	DRAWINGS DRILLED PIER
EA	EACH
EE	EACH END
EF	EACH FACE
ES	EACH SIDE
EW	
ELECT EL	ELECTRICAL ELEVATION
EQ	EQUAL
EJ	EXPANSION JOINT
EXT	EXTERIOR
FF	FAR FACE
FIN FS	FINISH FAR SIDE
FLR	FLOOR
FTG	FOOTING
FDN	FOUNDATION
FRMG	FRAMING
GALV GA	GALVANIZED GAUGE
HT	HEIGHT
HP	HIGH POINT
HORIZ	HORIZONTAL
HEF	HORIZONTAL EA FACE
IF INT	
JBE	INTERIOR JOIST BEARING ELEVATION
JT	JOINT
JST	JOIST
K OR k	KIP= 1,000 lbs
LT WT LTL	LIGHT WEIGHT LINTEL

STRUCTURAL ABBREVIATIONS:

LONG

LLH

LLV

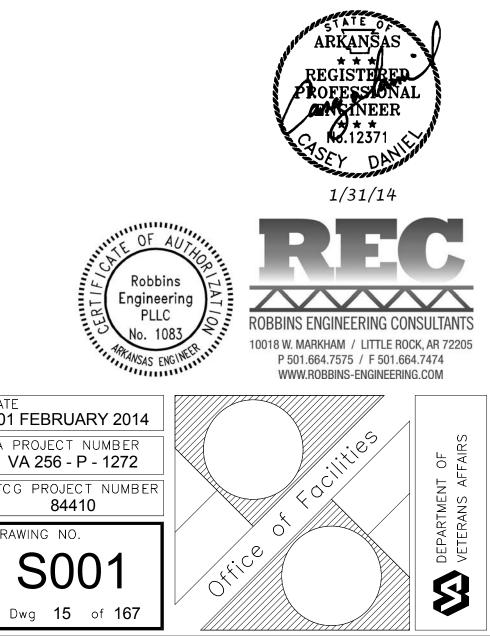
LIVE LOAD

LONGITUDINAL

LONG LEG HORIZONTAL

LONG LEG VERTICAL

LW LP MFR MK MSRY MBA MBM MBS MO MTI MIN MISC NF NS NML WT NIC NTS OC OPNG OPP OPP H OF PL PLBG PJF PP RAD RECT REF RF REINF REQ'D REV SCHED SECT SW SIM SOG SPA SPECS SQ STD STL SDI SJI STRUCT SUPPT SYMM SYP THK T/C T/F T/J T/P T/SL T/SOG T/S TYP UNO VB VERT VEF WB W/WF W/O



DRAWING TITLE PROJECT TITLE 01 FEBRUARY 2014 EUGENE J. TOWBIN HEALTHCARE CENTER GENERAL NOTES NEW SUBSTANCE ABUSE VA PROJECT NUMBER VA 256 - P - 1272 BUILDING TFCG PROJECT NUMBER 84410 BUILDING NUMBER DRAWN CHECKED RAWING NO. 172 CDD MWO S00⁻ LOCATION

C.A.V.H.S., LR DIVISION

LONG WAY LOW POINT MANUFACTURER MARK MASONRY MECHANICAL BAR ANCHOR METAL BUILDING MANUFACTURER MECHANICAL BAR SPLICE MASONRY OPENINGS MATERIAL MINIMUM MISCELLANEOUS NEAR FACE NEAR SIDE NORMAL WEIGHT NOT IN CONTRACT NOT TO SCALE ON CENTER OPENING OPPOSITE **OPPOSITE HAND** OUTSIDE FACE PLATE PLUMBING PRE-MOLDED JOINT FILLER PARTIAL PENETRATION RADIUS RECTANGULAR REFERENCE REFER TO REINFORCING REQUIRED REVISION SCHEDULE SECTION SHORT WAY SIMILAR SLAB SLAB ON GRADE SPACE, SPACING OR SPACES **SPECIFICATIONS** SQUARE STANDARD STEEL STEEL DECK INSTITUTE STEEL JOIST INSTITUTE STRUCTURE OT STRUCTURAL SUPPORT SYMMETRICAL SOUTHERN YELLOW PINE THICKNESS TOP TOP OF TOP OF CONCRETE TOP OF FOOTING TOP OF JOIST TOP OF LEDGE TOP OF PILASTER TOP OF SLAB TOP OF SLAB ON GRADE TOP OF STRUCTURAL STEEL TYPICAL UNLESS NOTED OTHERWISE VAPOR BARRIER VERTICAL VERTICAL EACH FACE WIND BRACE WELDED WIRE FABRIC WIDE FLANGE WITH WITHOUT WORK POINT WATER STOP WEIGHT

	2006 IBC SPECIAL INSPECTIONS		FREQUENCY *
CONST. TYPE	INSPECTION TASK 1. Verify materials below footings are adequate to achieve the design bearing capacity.	CONTINUOUS	PERIODIC
	 Verify excavations are extended to proper depth and have reached proper material. 		
SOILS	3. Perform classification and testing of controlled fill materials.		
RE: 1704.7	Verify use of proper materials, densities and lift thickness during	X	
	Prior to placement of controlled fill, observe subgrade and verify		X
	 that site has been prepared properly. Inspection of reinforcing steel and placement. 		
	 Inspection of reinforcing steel welding in accordance with table 1704.3, item 5B. 		
	Inspect bolts and embeds to be installed in concrete prior to and	X	
	 during placement of concrete. Verifying use of required design mix. 		X
	Sampling fresh concrete and performing slump, air content and determining the temperature of fresh concrete at the time of making	X	
CONCRETE RE: 1704.4	S. Contracting the temperature of mean concrete at the time of making specimens for strength tests.		
	o. application techniques	× – – – – – – – – – – – – – – – – – – –	
·	7. Inspection for maintenance of specified curing temperature and techniques Inspect formwork for shape, location and dimensions of the concrete		X
	8. member being formed.		X
	9. Inspection of anchors installed in hardened concrete. Material verification of high-strength bolts, nuts and washers.		×
	1. A. Identification of markings to conform to ASTM standards specified in the approved construction documents.	—	X
	B. Manufacturer's certificate of compliance is required.		X
	 Inspection of high strength bolting. A. Bearing type connections, snug tight & slip critical. 	_	X
	A. Bearing type connections, shug tight a slip childal. A. Bearing type connections, shug tight a slip childal.		
	A. Identification markings to conform to ASTM standards specified in the approved construction documents.		×
	B. Manufacturer's certificate of compliance required.		×
	4. Material verification of weld filler material.		
	 A. Identification markings to conform to ASTM standards specified in the approved construction documents. 	—	×
STRUCTURAL	B. Manufacturer's certificate of compliance required.		×
STEEL RE: 1704.3	4. Inspection of welding.		
	A. Structural steel: 1. Complete & partial penetration groove welds.	×	
	2. Multi-pass fillet welds.	× ×	
	3. Single-pass fillet welds > 5/16".	X	
	4. Single-pass fillet welds < 5/16".		X
	5. Single-pass fillet welds < 5/16".B. Reinforcing steel:		X
	1. Verification of weldability of reinforcing steel other than ASTM A706		
	2. Shear Reinforcement.	X	X
	3. Other Reinforcing steel.		X
	 Inspection of steel frame joint details in compliance with approved construction documents: 		
	 A. Details such as braces and stiffeners. B. Member Locations. 	_	×
	C. Application of joint details at each connection.		
	 As masonry construction begins, the following shall be verified to ensure compliance: A. Proportions of site prepared mortar. 		X
	A. Construction of mortar joints.		
	C. Location of reinforcement and connectors.		X
	 During construction the inspection program shall verify: A. Size and location of structural elements: 		X
	B. Type, size and location of anchors including other details		X
	of anchorage of masonry to structural members, frames or other construction.		^
MASONRY	C. Specified size, grade and type of reinforcement.		X
RE: 1704.3	 D. Welding of reinforcing bars. E. Protection of masonry during cold (Temperature below 40° F) 	×	
	or hot wather (Temperature above 90°).		×
	 Prior to grouting, the following shall be verified to ensure compliance: A. Grout space is clean 	_	×
	A. Grout space is clean. B. Placement of reinforcement and connectors.		X
	C. Proportions of site-prepared grout.		X
	D. Construction of mortar joints.		X
	4. Grout placement shall be verified to ensure compliance with code and construction document provisions	×	_
	5. Preparation of any grout specimens, mortar specimens and/or prisms shall be observed.	×	
·	 Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified 		X
	documents and the approved submittals shall be verified. 1. Observe drilling operations and maintain complete accurate records for	X	
סורף	2. Verify placement locations and plumbness, confirm pier diameters,		
PIER FOUNDATIONS RE: 1704.9	lengths, embedment into bedrock and adequate end bearing strata capacity.	×	_
	 For concrete piers, perform additional inspections in accordance with section 1704.5. 		1
	section 1704.5. 1. Deck attachment per general and plan notes on construction		X
METAL DECK	documents.		
COMPONENT ANCHORAGE RE: 1704	1. Installation of shallow expansion, chemical and cast in place anchors in masonry and concrete.		X
Special inspection,	<u>continuous:</u> The full time observation of work requires special oproved special inspector who is present in the area where the work is		
inencotion but	WORKED AVEGATIVATED WITCHS DIESENTID THE ALEA WHERE THE WORK IS		

NO.	DESCRIPTION DATE	
		APPROVED: MEDICAL CENTER DIRECTOF
		APPROVED: ASSOCIATE DIRECTOR
		APPROVED: CHIEF OF STAFF

SPECIAL INSPECTIONS:

specifications. The approved independant testing agency's individual special inspector shall demonstrate competence for inspection of particular type of construction of operation requiring special inspection and shall meet the minimum special inspector qualifications in table 1704.1 of the 2006 IBC. The special inspector shall bring non-conforming items to the immediate attention of the contractor in writing and note all such items in the reports. Any unresolved item about to be covered by the work shall be brought to the attention of the contractor's and owner's construction manager's attention immediately. The special inspector shall furnish reports, tests and inspections directly to the engineer of record, the owner's construction manager and the contractor. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in comformance with the approved plans and specifications. The contractor is responsible for notifying the special inspection agency regarding individual inspection for items listed on the schedule and as noted on the building department approved plans. Adequate notice and access to the approved plans shall be provided so that the special inspector has time to become familiar with the project.

Provide special inspections for the following items per section 1704 of the IBC and section 014000 of the project

STRUCTURAL OBSERVATION REQUIREMENTS:

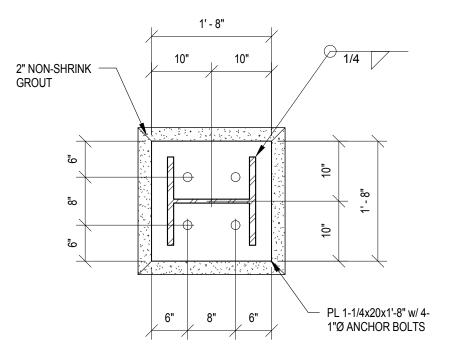
- 1. A representative of the engineer of record will perform structural observation of the elements and connections of the structural system at critical stages of construction and the completed structure for general conformance with the approved plans and specifications. Structural observation does not waive the responsibility for the inspections required of the building official or the special inspectors.
- 2. A pre-construction meeting shall be held and attended by the design architect or engineer, the engineer who will perform the structural observation, the contractor and affected sub-contractors and the special inspectors. 3. The general contractor shall notify the structural engineer of record at least 48 hrs. prior to completing construction
- operations that require structural observation by calling (501) 664-7575 to schedule a site visit. 4. At a minimum, the following significant construction stages require a site visit and an observation report from the structural observer.

Construction Stages

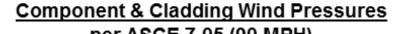
- 1. At the first day of pier drilling and installation. 2. After installation of concrete grade beam reinforcing and before concrete placement. 3. After erection of structural steel and before metal deck replacement.
- 4. After installation of metal roof deck and before roof installation.

SUBMITTAL PROCEDURES:

- 1. Transmit submittals in advance of related construction activities to avoid unnecessary delay. The structural engineer of record for this project may withhold action on a submittal requiring coordination with other submittal until all related submittals are received.
- 2. Submit one electronic portable document format (.pdf) copy through the architect for the "Shop Drawing" review. The electronic copy will be marked up by the structural engineer of record. One copy will be kept by the structural engineer of record and an additional copy will be returned to the architect. The architect will keep one copy and return a copy to the contractor. The contractor will make additional copies as required for his needs.
- 3. Action stamp: The structural engineer of record will stamp each submittal with a uniform action stamp to indicate the action taken in one of five options. -Permitted: Work covered by the submittal generally complies with the requirements of the contract documents.
 - -Rejected: Work covered by the submittal is unacceptable and may not proceed.
 - -Submit Specified Item: Comply with the content of the specifications for the indicated item.
 - -Permitted & Corrections Noted: Work covered by the submittal may proceed provided it complies with the notations or corrections on the submittal and requirements of the contract documents.
- -Revise and Resubmit. Work covered by the submittal does not comply with the requirements of the contract documents and must be changed to comply and resubmit the entire submittal.
- 4. Contractor shall comply with Division One Section "Submittals." 5. No reproduction of construction documents are acceptable for use as shop drawings.





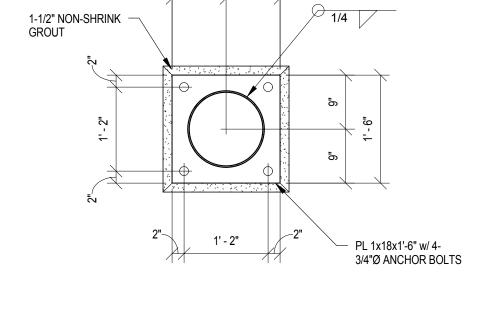


per ASCE 7-05 (90 MPH)						
	Roof Pressures (psf)			Wall Pressures (psf)		
Area (sq. ft.)	Zone 1	Zone 2	Zone 3	Zone 4	Zone 5	
≤10	+10, -38	+10, -38	+10, -60	+22, -24	+22, -29	
25	+9, -37	+9, -37	+9, -44	+21, -23	+21, -27	
50	+9, -37	+9, -37	+9, -32	+20, -22	+20, -25	
75	+8, -36	+8, -36	+8, -25	+19, -21	+19, -24	
≥100	+8, -36	+8, -36	+8, -20	+19, -21	+19, -23	
	Dim "a" = 9'-0" Dim "a" = 9'-0"					

Notes:

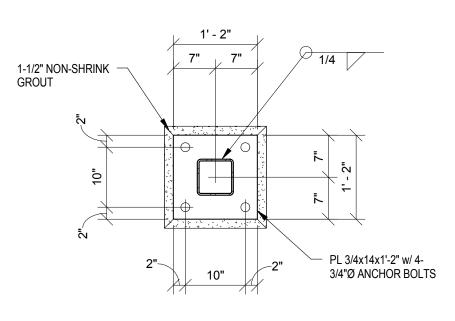
1. Wind loads are generated using ASCE 7 Method 2 - Analytical Procedure. 2. Refer to ASCE 7 Figures 6-11 through 6-17 for additional information and wall/roof diagrams.

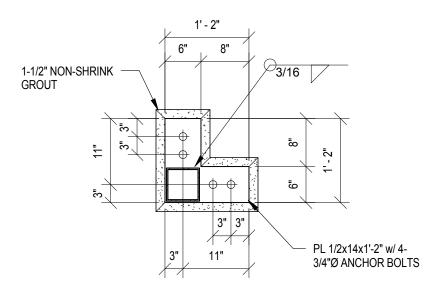
3. Pressures listed are gross pressures. Dead weights of roofing materials have not been subtracted.



9"







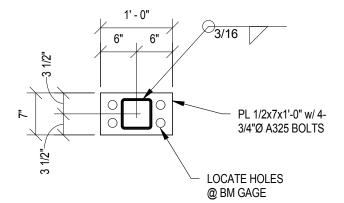


BASEPLATE B6

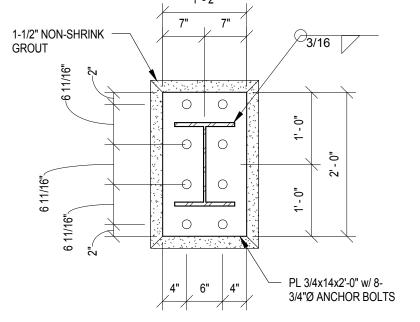


DRAWING TITLE INSPECTION NOTES & BASEPLATE DETAILS	PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE BUILDING			DATE 01 FEBRUARY 2 VA PROJECT NUME VA 256 - P - 12 TFCG PROJECT NU	
	BUILDING NUMBER	CHECKED CDD	DRAWN MWO	DRAWING NO.	
	LOCATION C.A.V.H		ISION	Dwg 16 of	

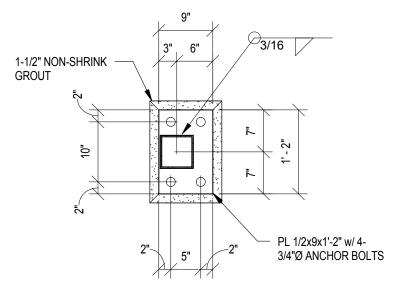
BASEPLATE B7

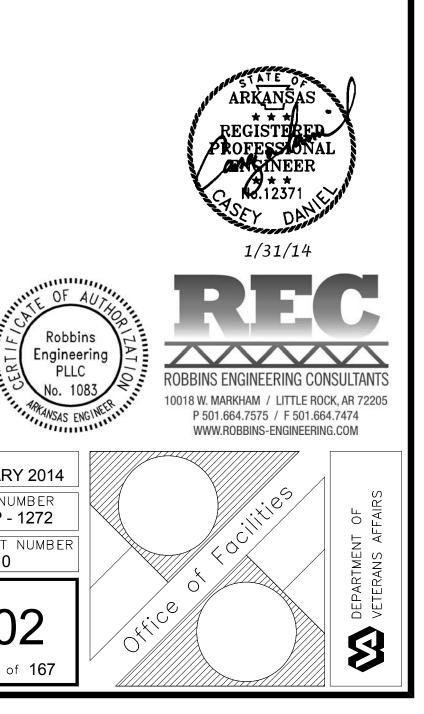


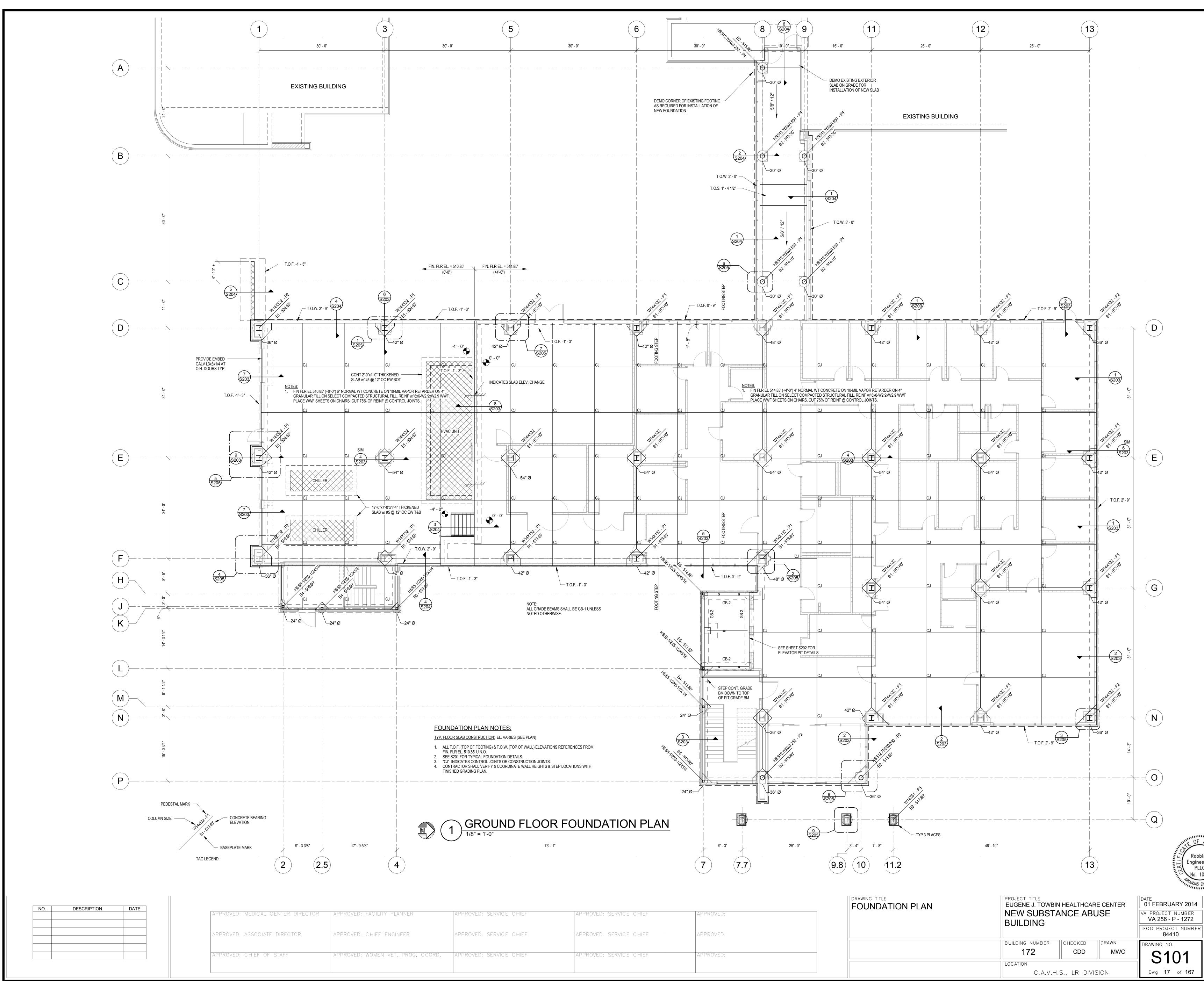






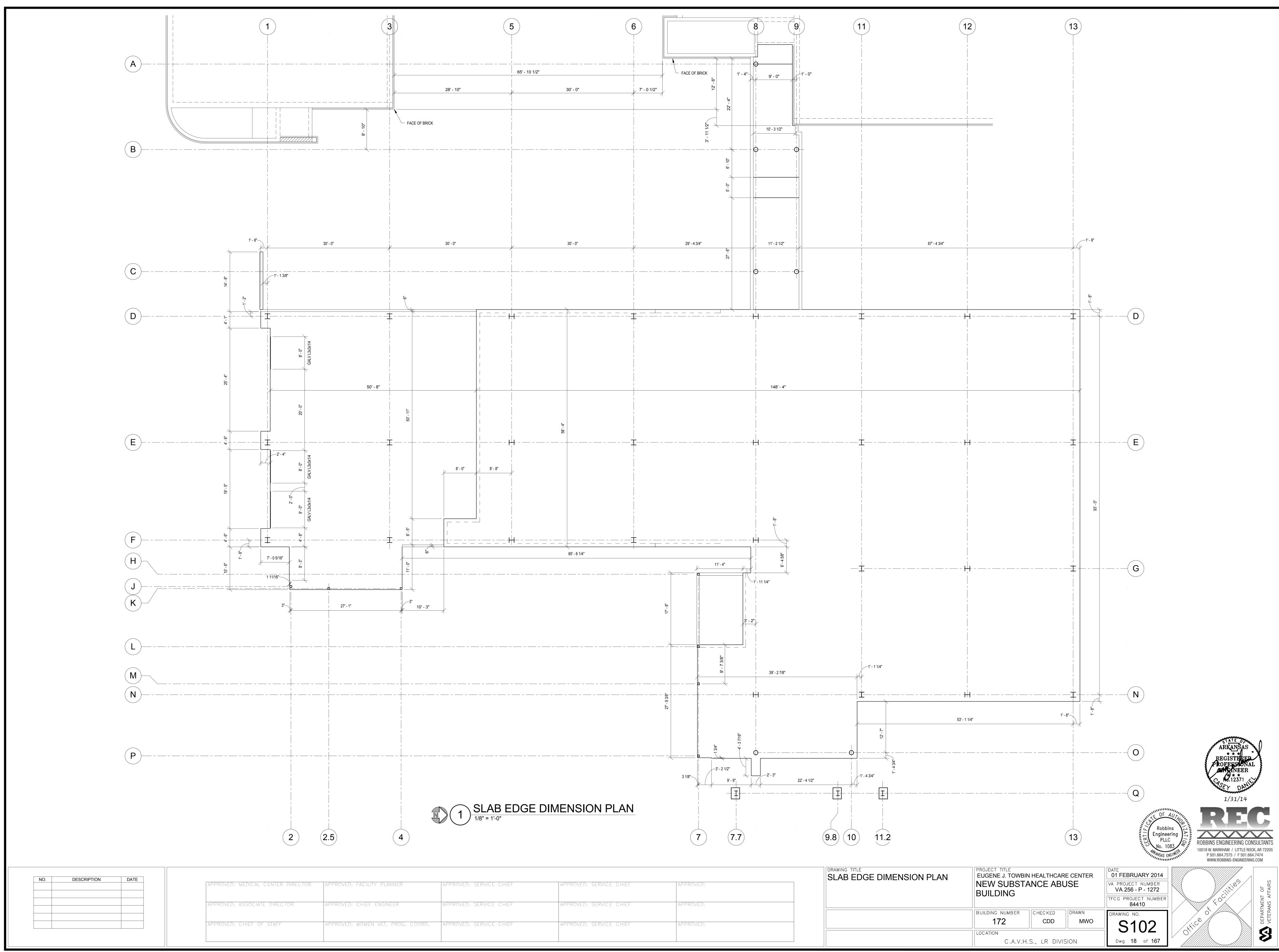




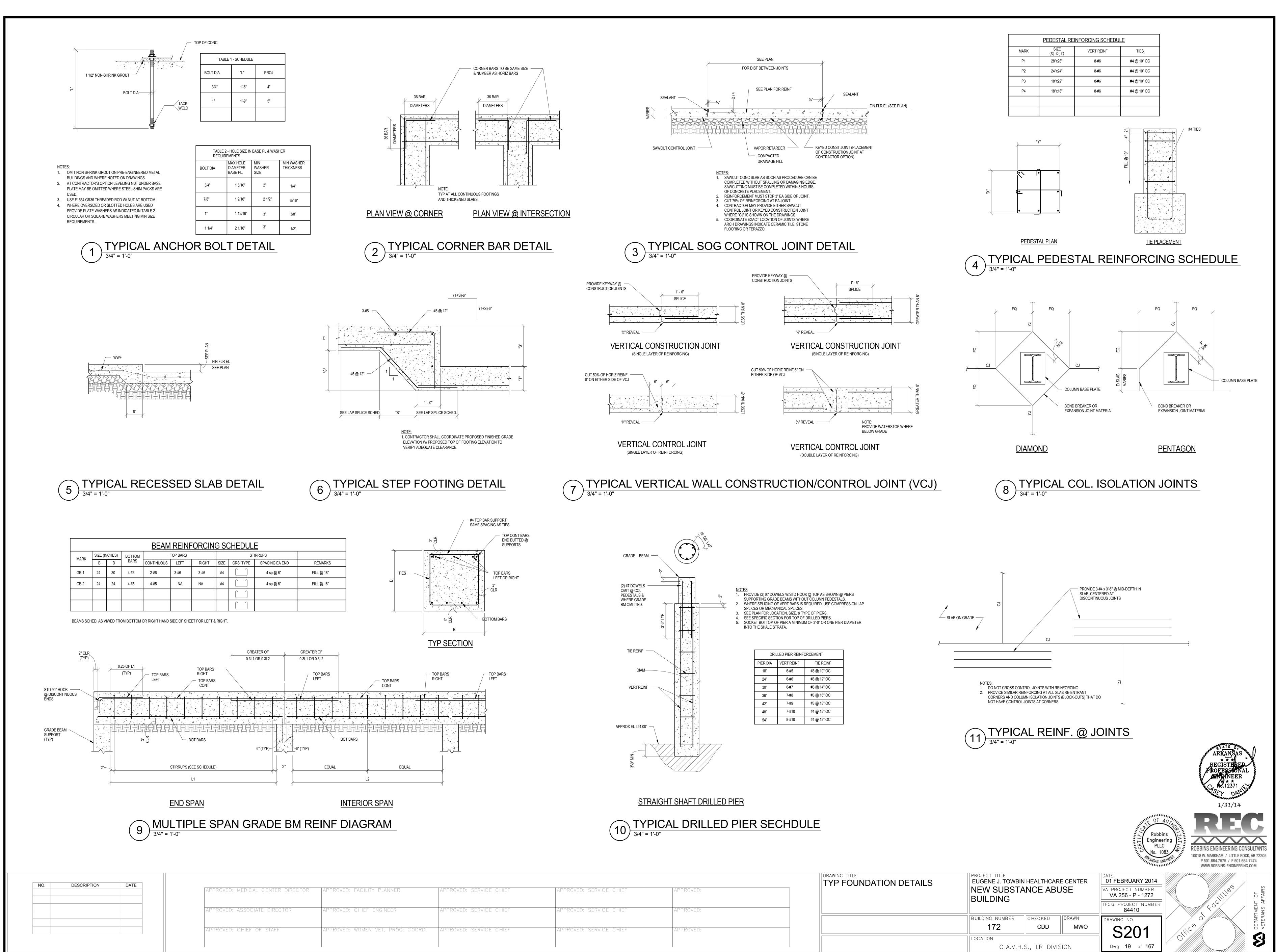


/ED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
/ED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
/ED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

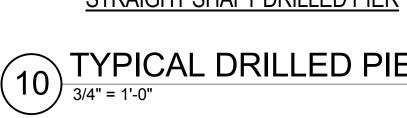


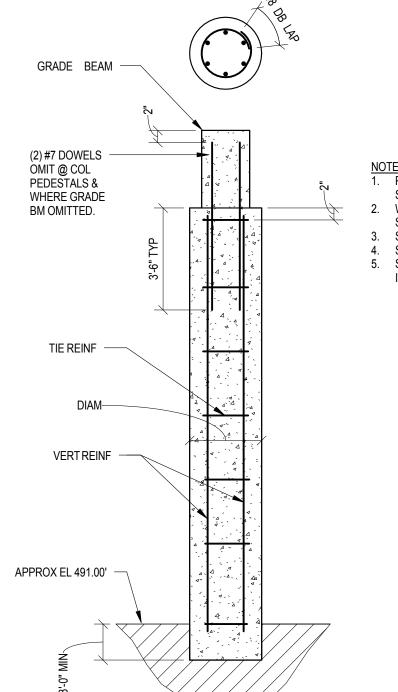


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/ED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
/ED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:



OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
OVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
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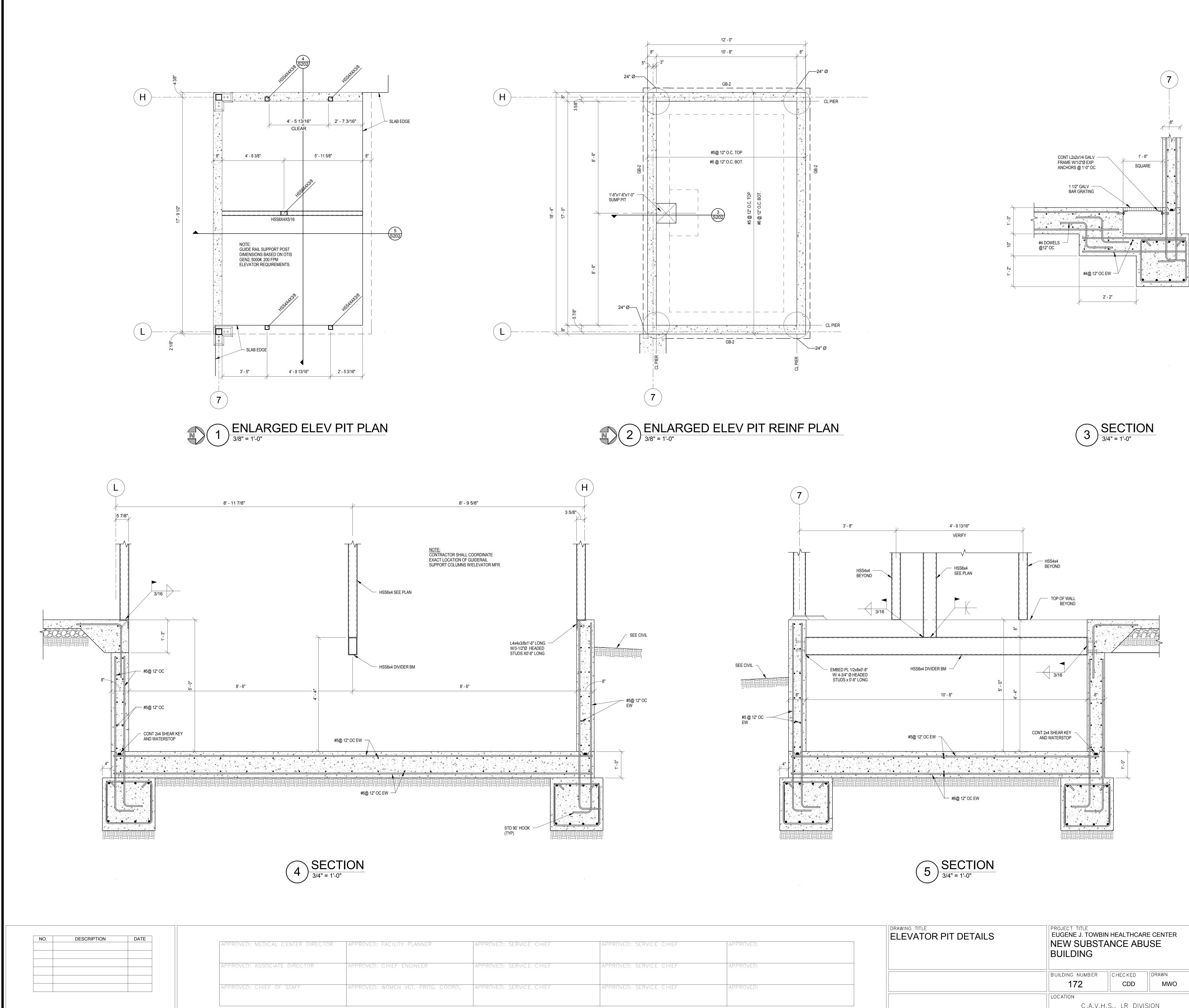


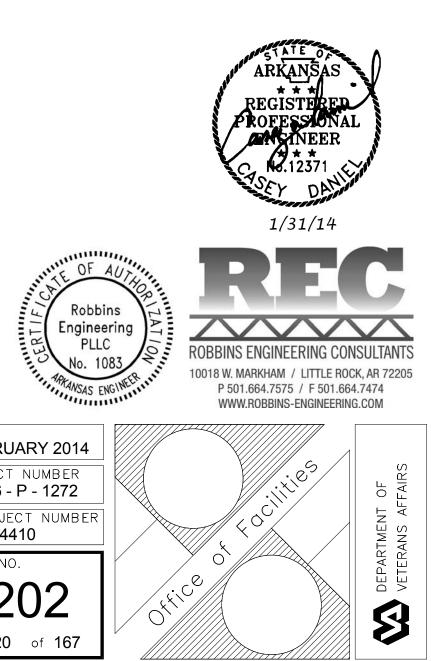


):
ROVIDE (2) #7 DOWELS W/STD HOOK @ TOP AS SHOWN @ PIERS
UPPORTING GRADE BEAMS WITHOUT COLUMN PEDESTALS.
HERE SPLICING OF VERT BARS IS REQUIRED, USE COMPRESSION LAP
PLICES OR MECHANICAL SPLICES.
EE PLAN FOR LOCATION, SIZE, & TYPE OF PIERS.
EE SPECIFIC SECTION FOR TOP OF DRILLED PIERS.
OCKET BOTTOM OF PIER A MINIMUM OF 3'-0" OR ONE PIER DIAMETER
ITO THE SHALE STRATA.

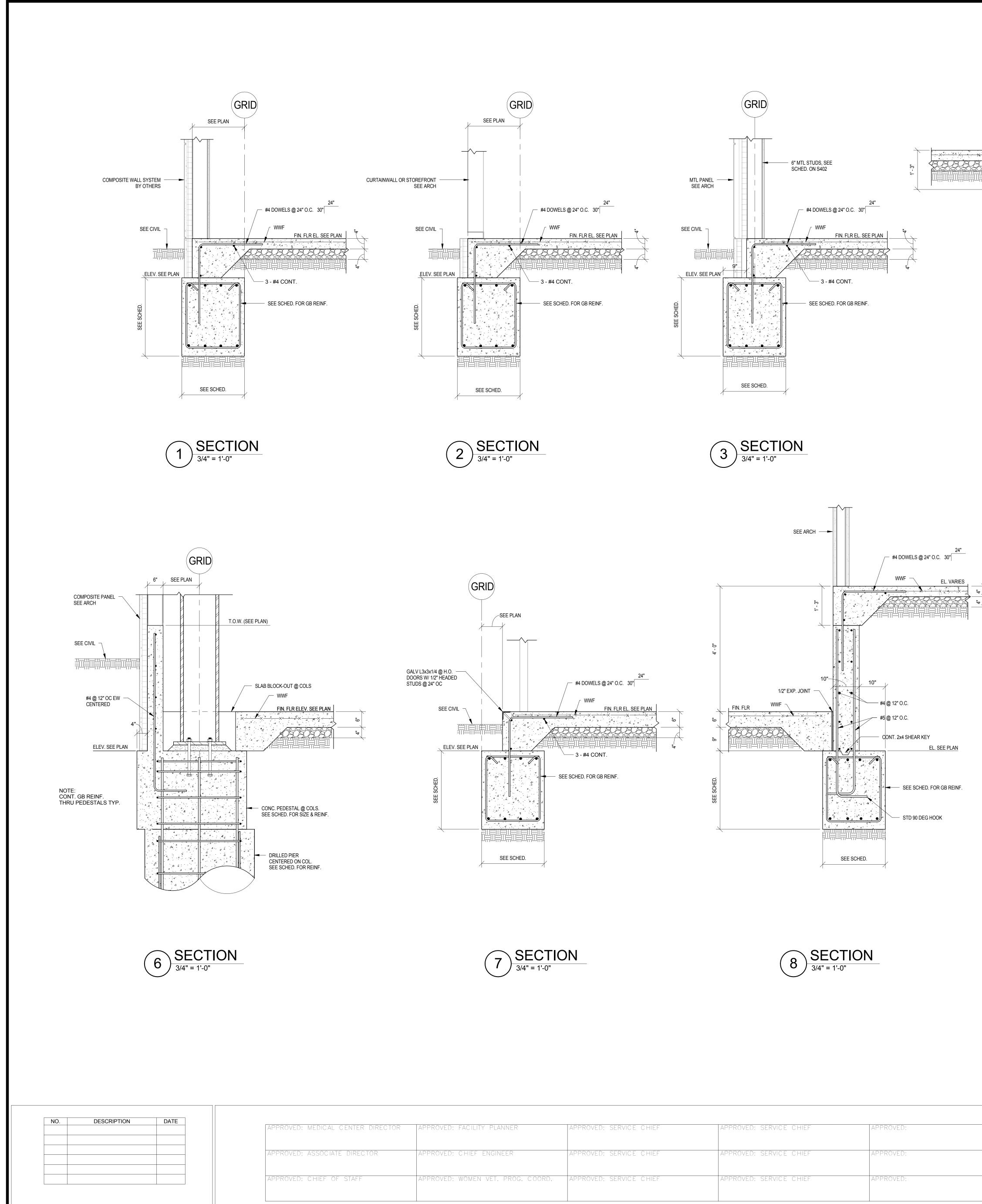
DRIL	DRILLED PIER REINFORCEMENT					
PIER DIA	VERT REINF	TIE REINF				
18"	6-#5	#3 @ 10" OC				
24"	6-#6	#3 @ 12" OC				
30"	6-#7	#3 @ 14" OC				
36"	7-#8	#3 @ 16" OC				
42"	7-#9	#3 @ 18" OC				
48"	7-#10	#4 @ 18" OC				
54"	8-#10	#4 @ 18" OC				

TYP FOUNDATION DETAILS		EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE		
	BUILDING NUMBER	C HEC KED CDD	DRAWN MWO	
	LOCATION C.A.V.H.S., LR DIVISION			





	DRAWING TITLE ELEVATOR PIT DETAILS		PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE BUILDING			
			BUILDING			TFCG PROJECT NU 84410
		BU	JILDING NUMBER	CHECKED CDD	DRAWN MWO	DRAWING NO.
			C.A.V.H.S., LR DIVISION			Dwg 20 of 1



DVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:	
DVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:	
DVED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:	

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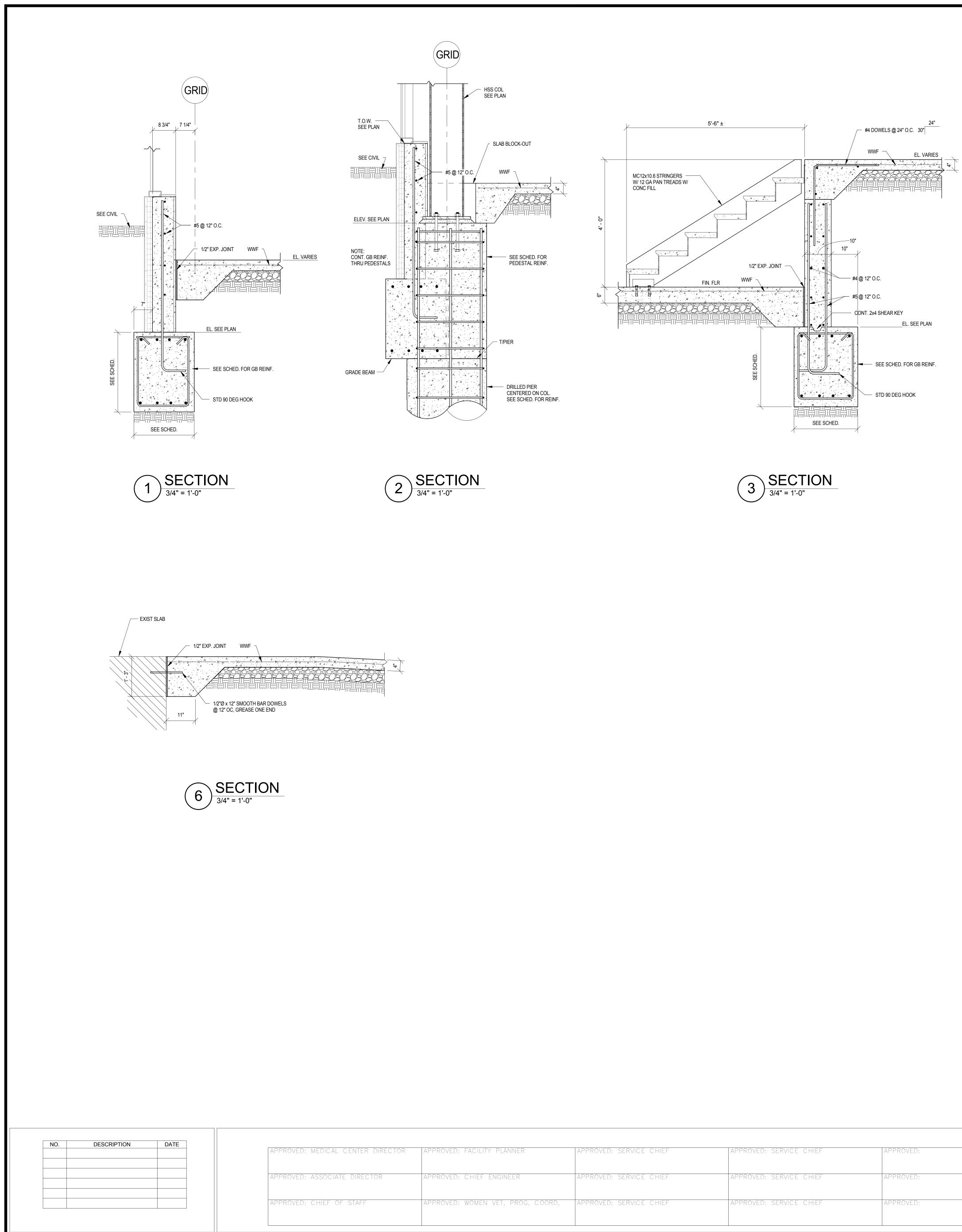
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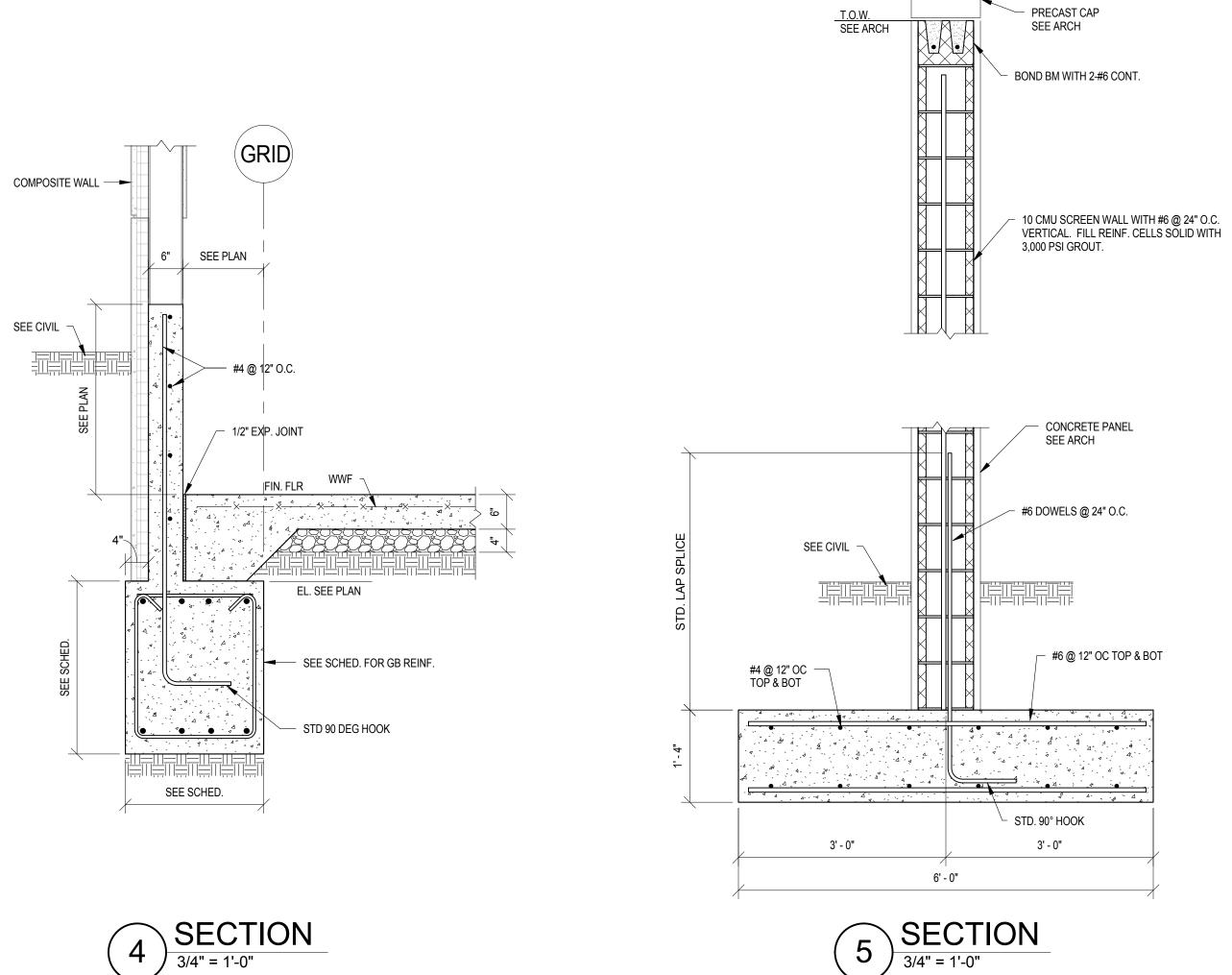
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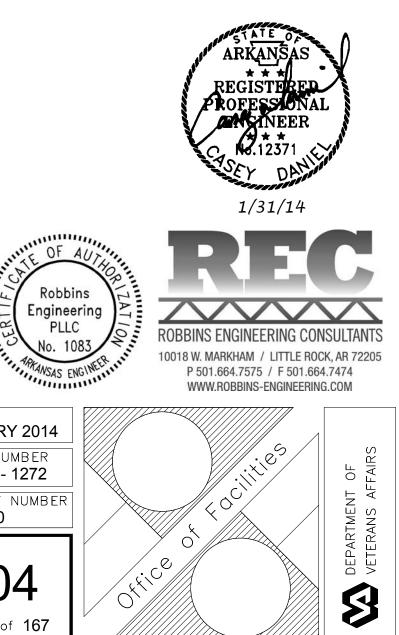


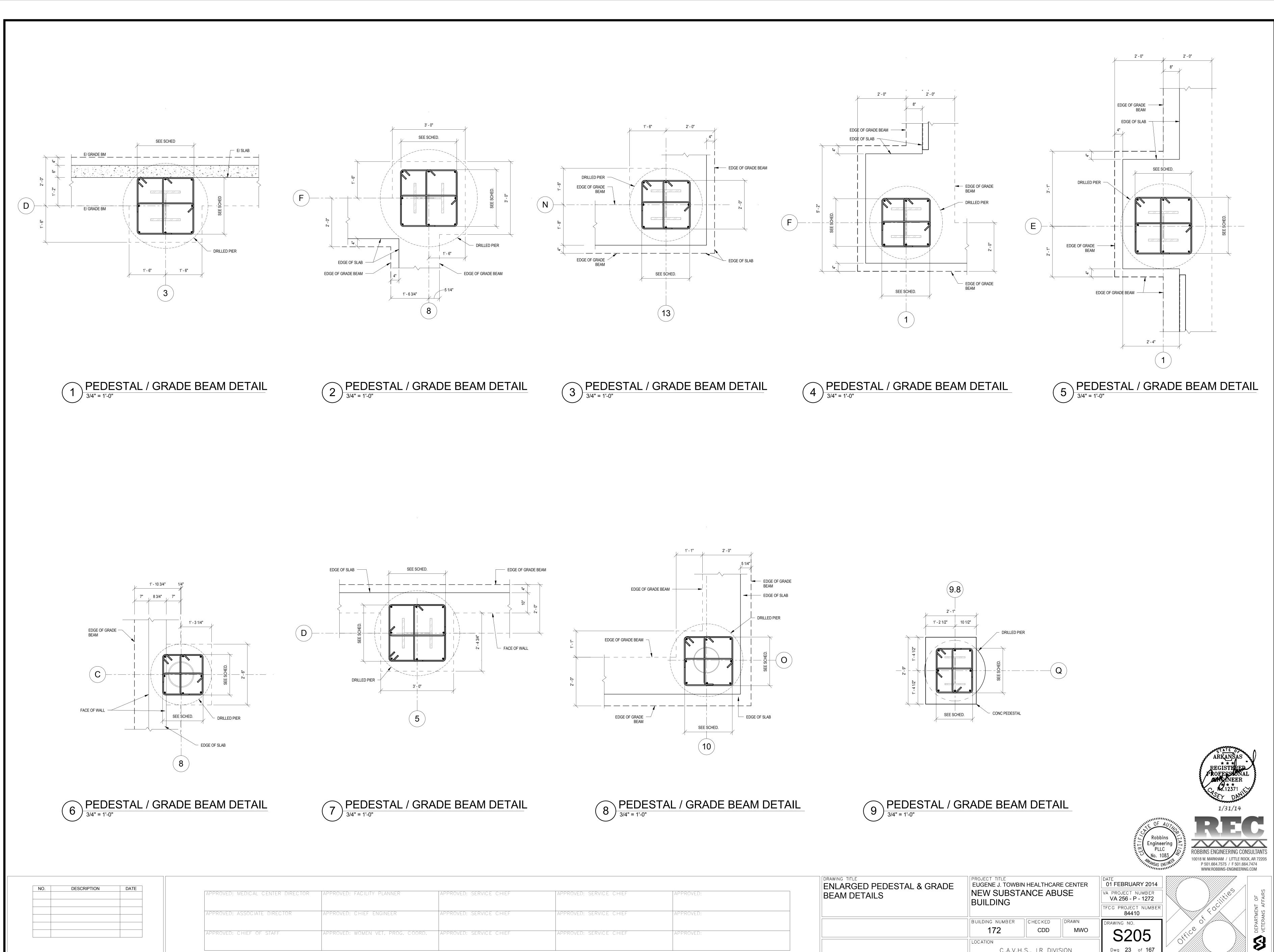
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IVED: CHIEF ENGINEER		APPROVED: SERVICE CHIEF	APPROVED:
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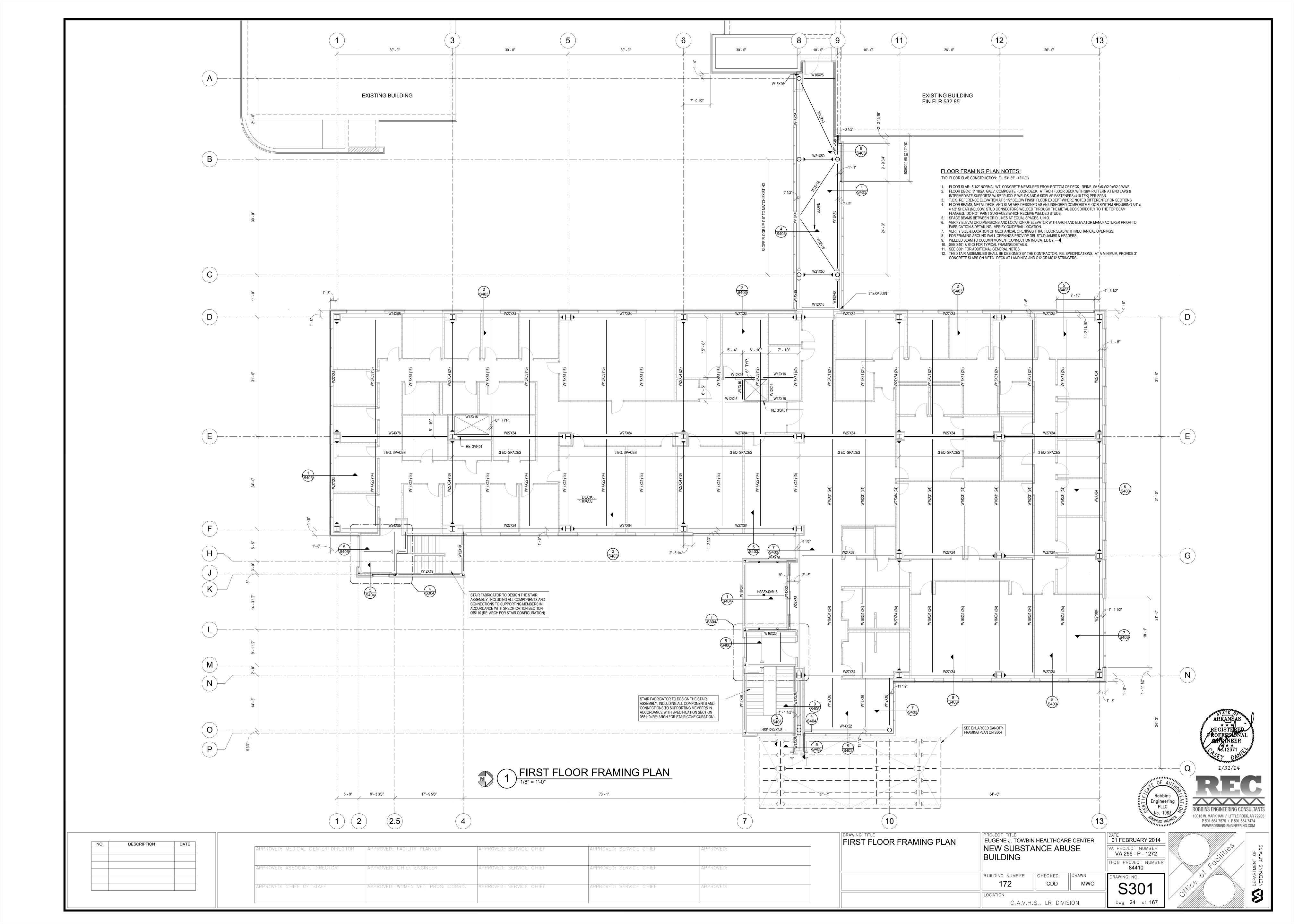
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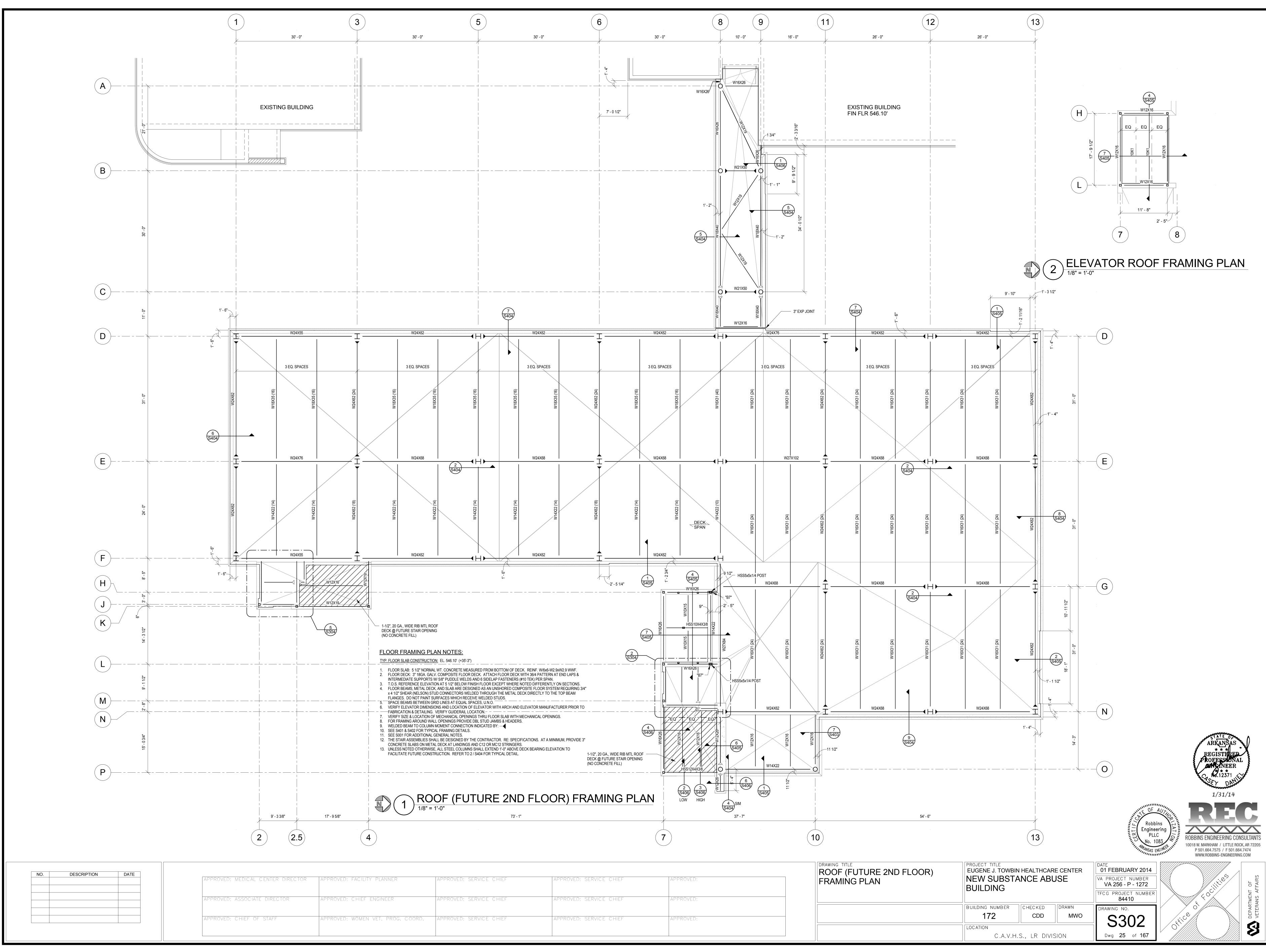
DRAWING TITLE FOUNDATION SECTIONS	PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE BUILDING		DATE 01 FEBRUARY 2014 VA PROJECT NUMBER VA 256 - P - 1272	
				TFCG PROJECT NUMBER 84410
	BUILDING NUMBER	CHECKED	DRAWN	DRAWING NO.
	172	CDD	MWO	S204
	LOCATION			
C.A.V.H.S., LR DIVISION				Dwg 22 of 167



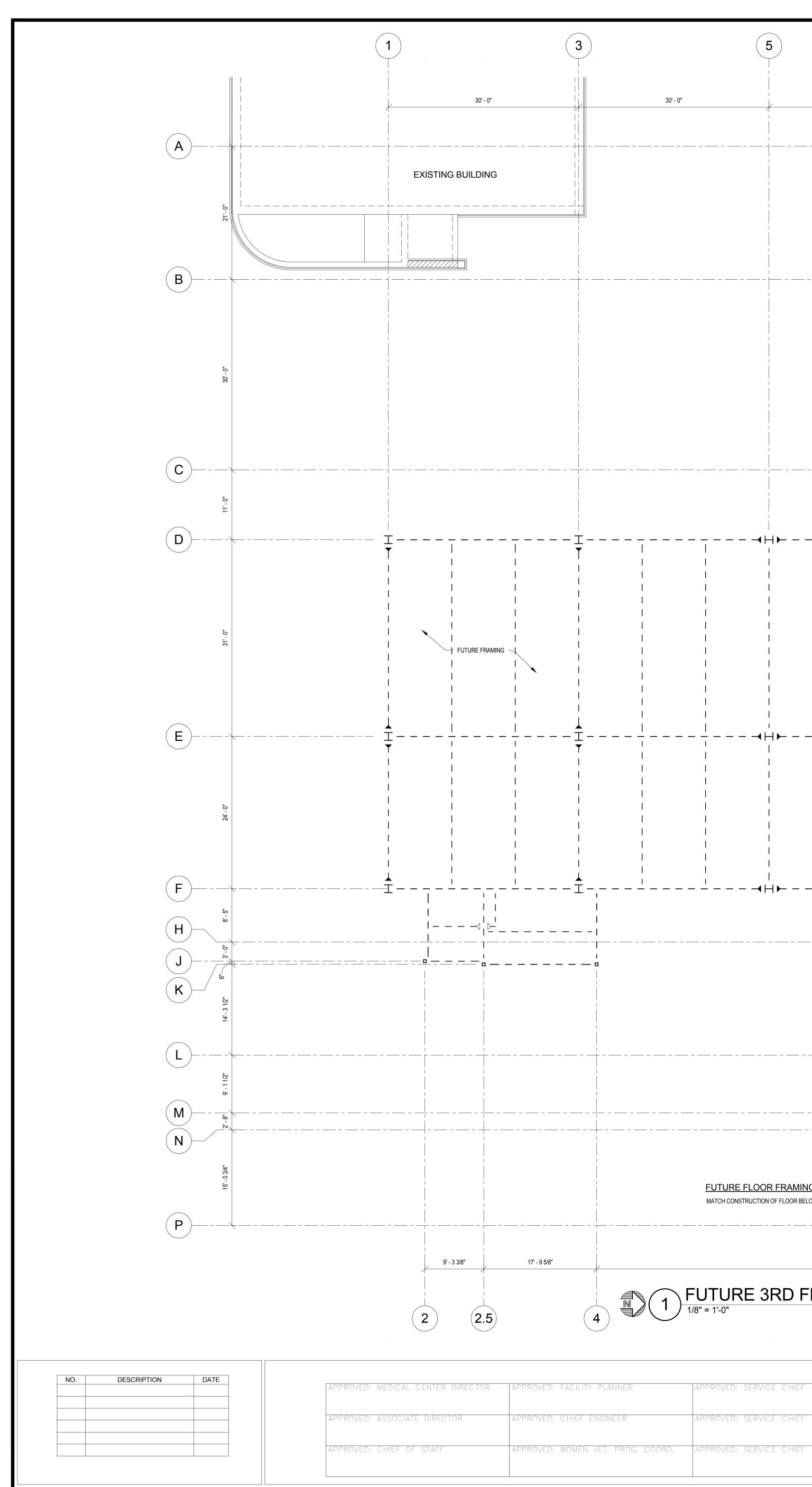


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OVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:	BEAM DETAILS	BUILDING	VA 256 - P - 1272 TFCG PROJECT NUM 84410
OVED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		BUILDING NUMBER CHECKED DRAWN 172 CDD MWO	drawing no. S205
					C.A.V.H.S., LR DIVISION	Dwg 23 of 16





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D: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
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FUTURE FLOOR FRAMING PLAN: D.B.E. 560.35' (+49'-6") I

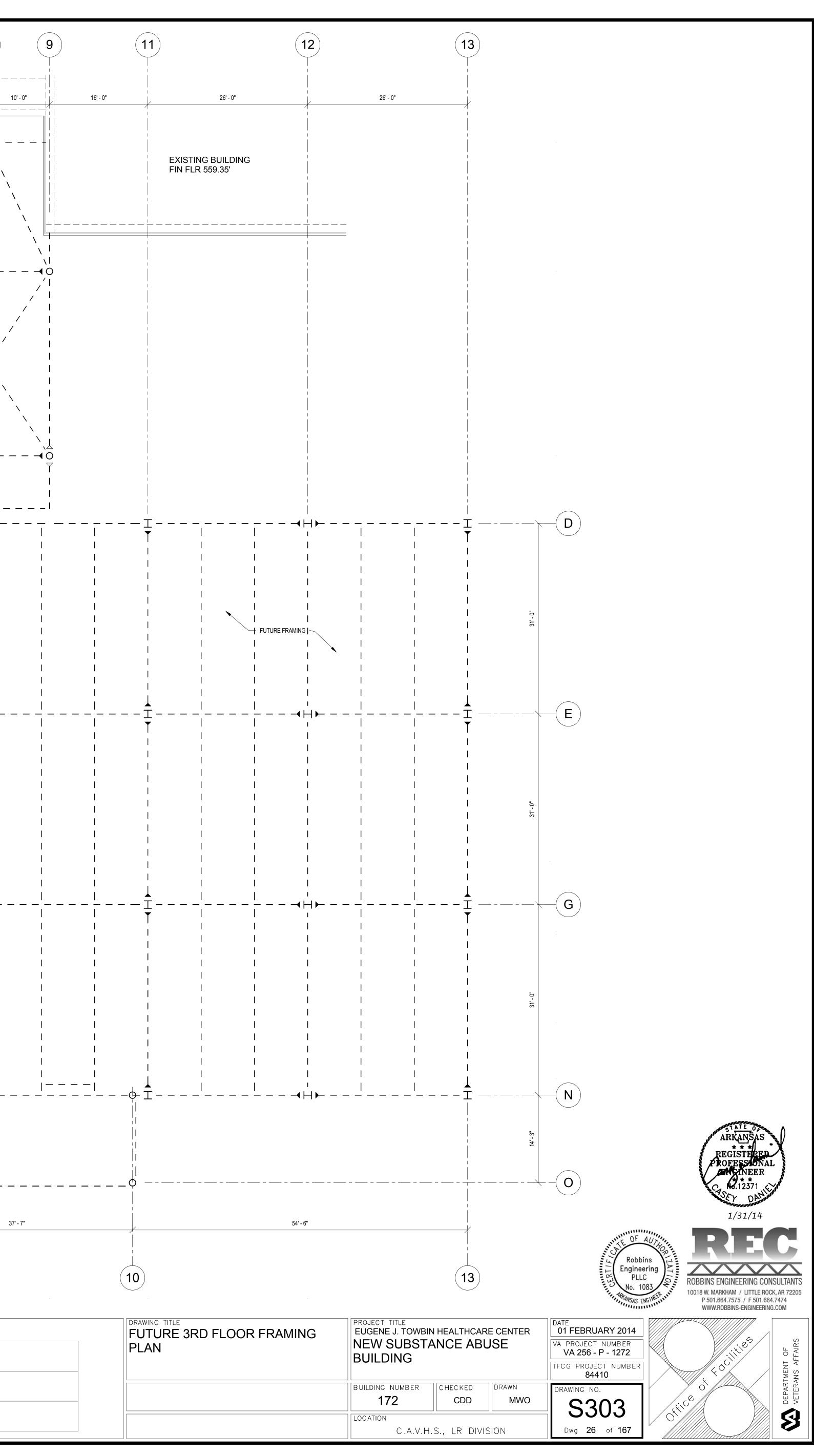
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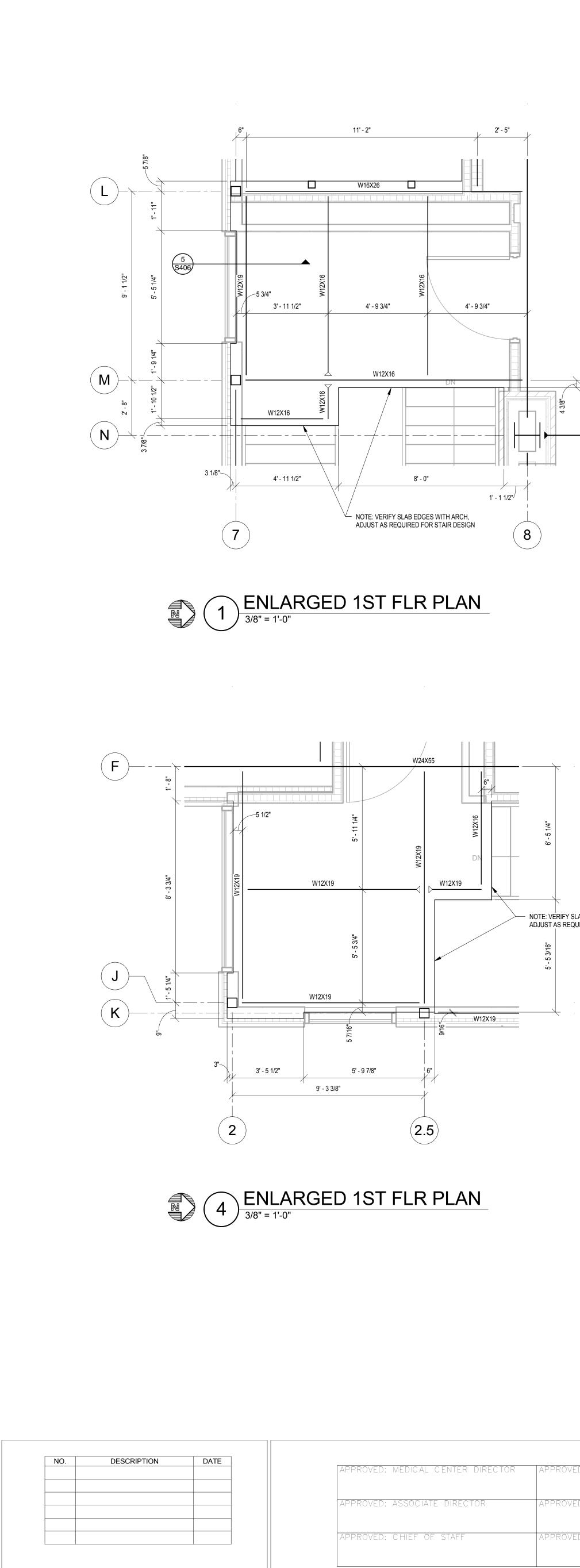
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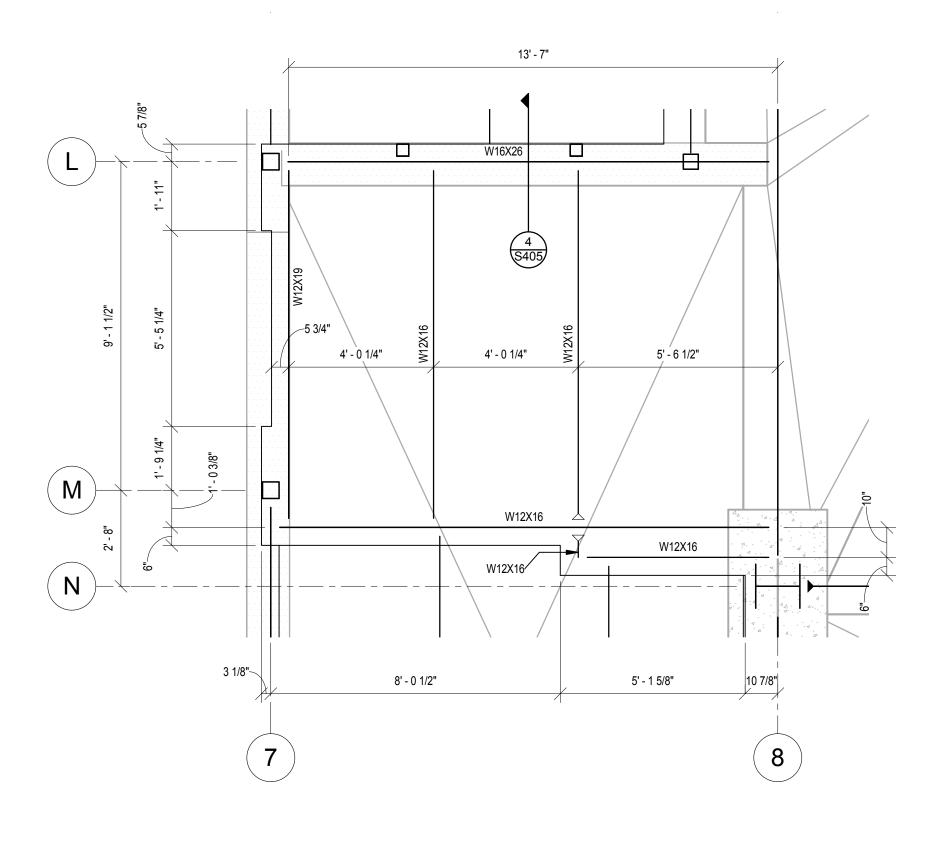
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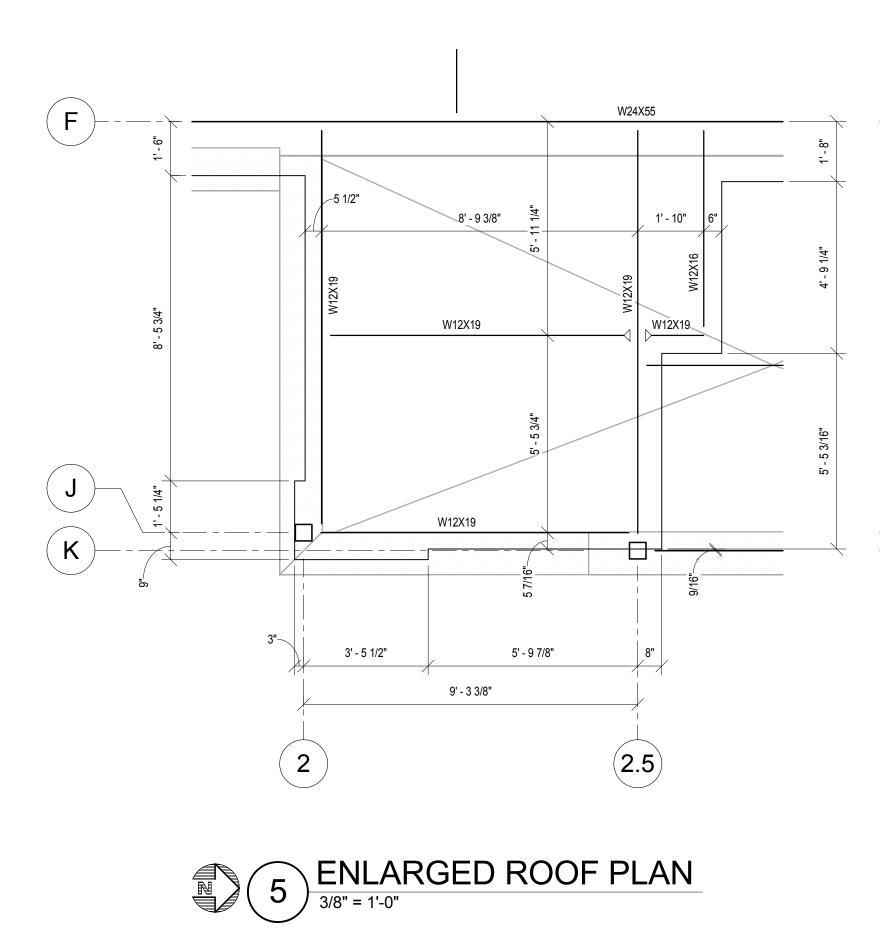
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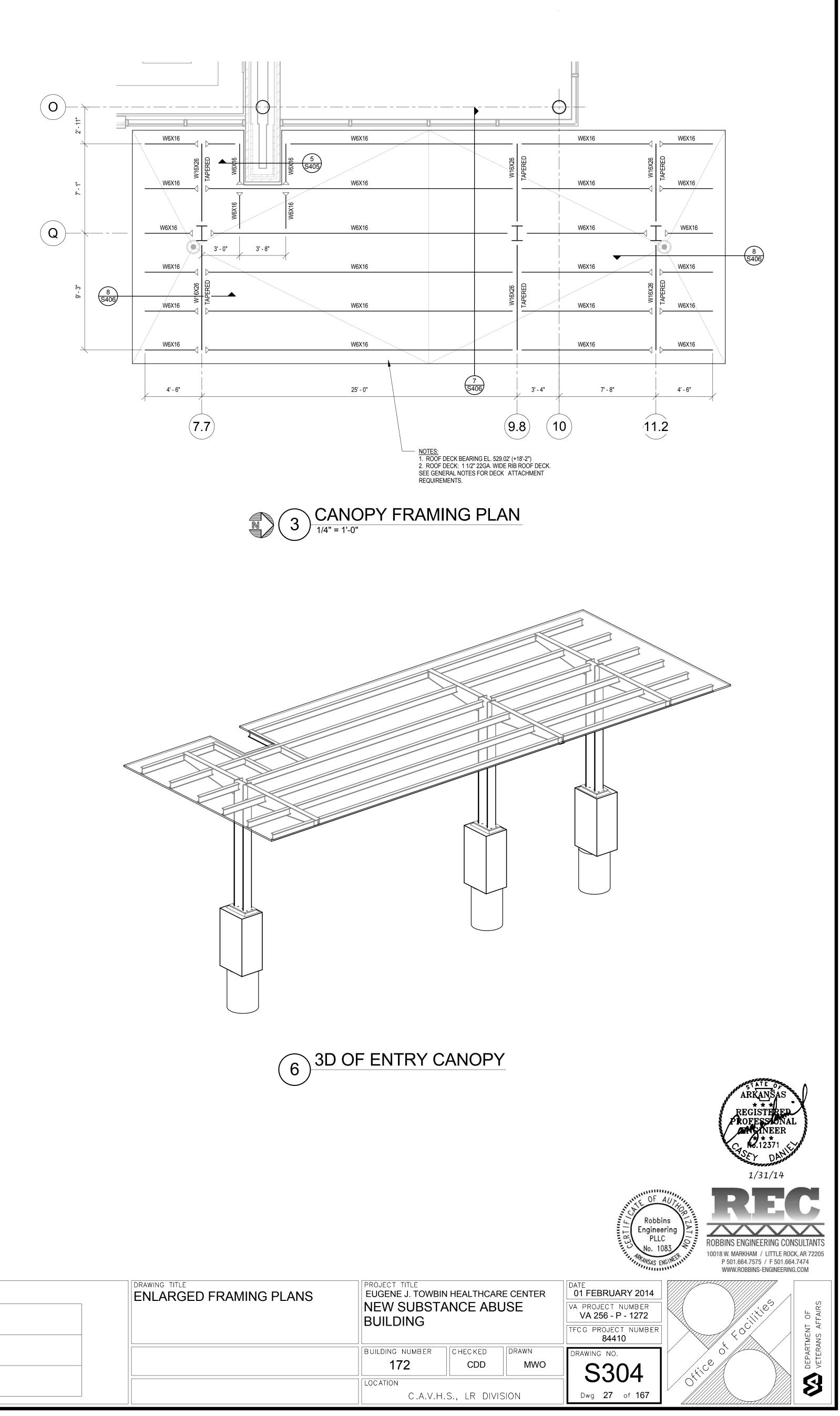


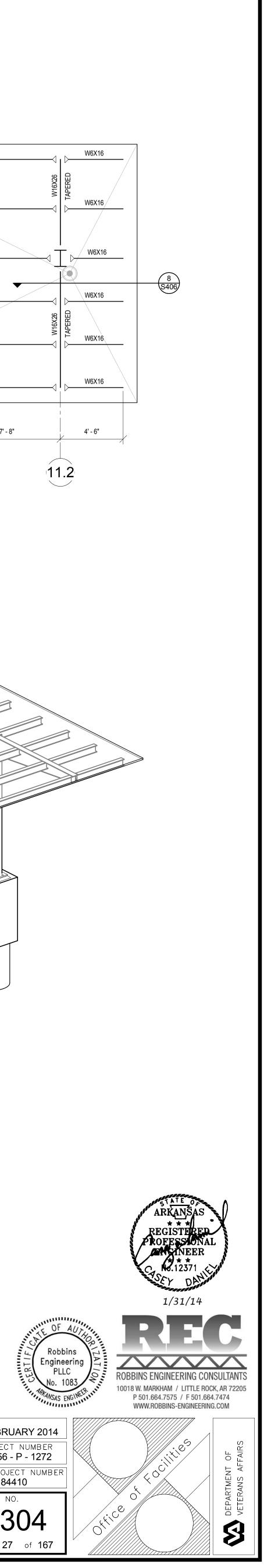




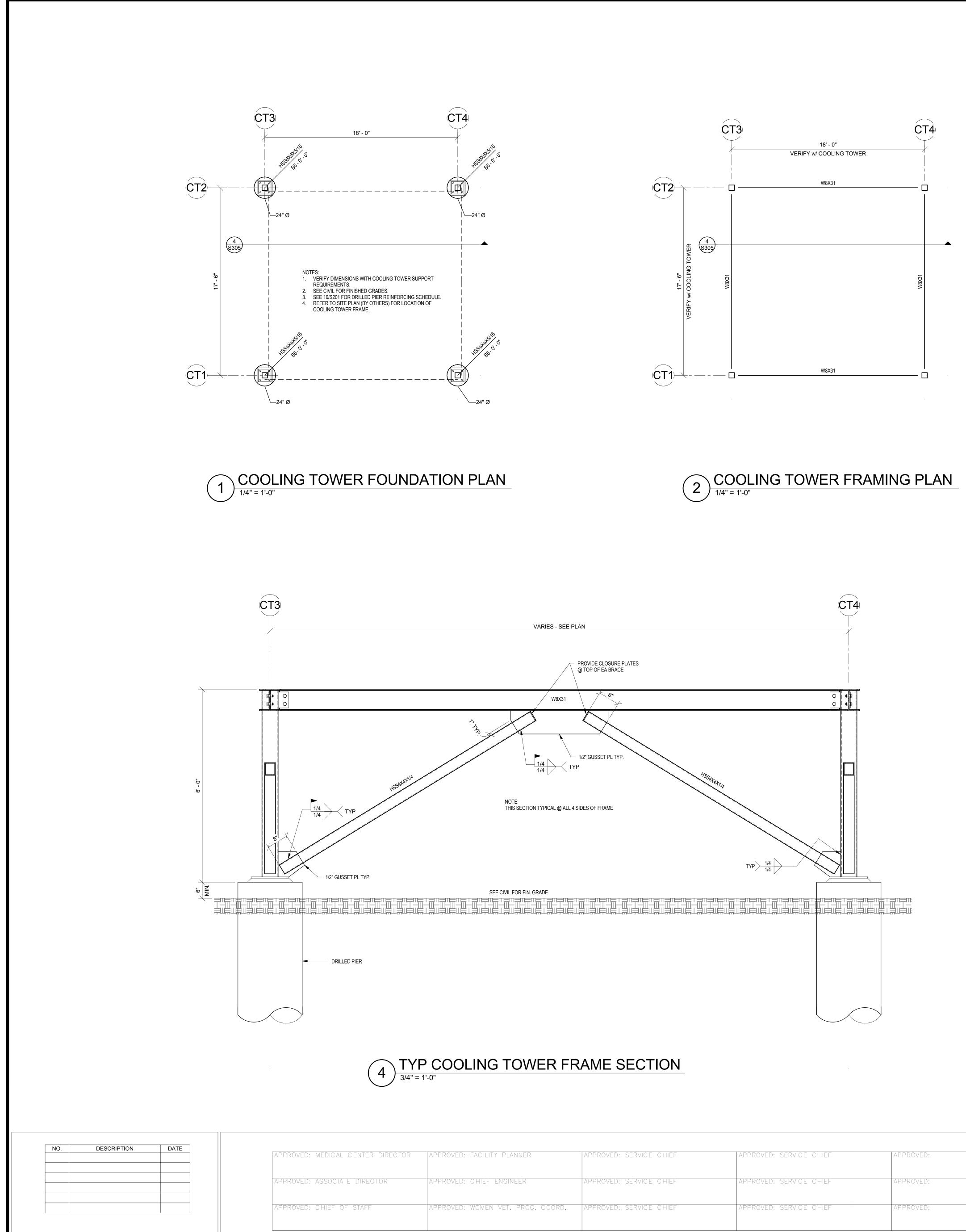
- NOTE: VERIFY SLAB EDGES WITH ARCH, ADJUST AS REQUIRED FOR STAIR DESIGN

IVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
IVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
VED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

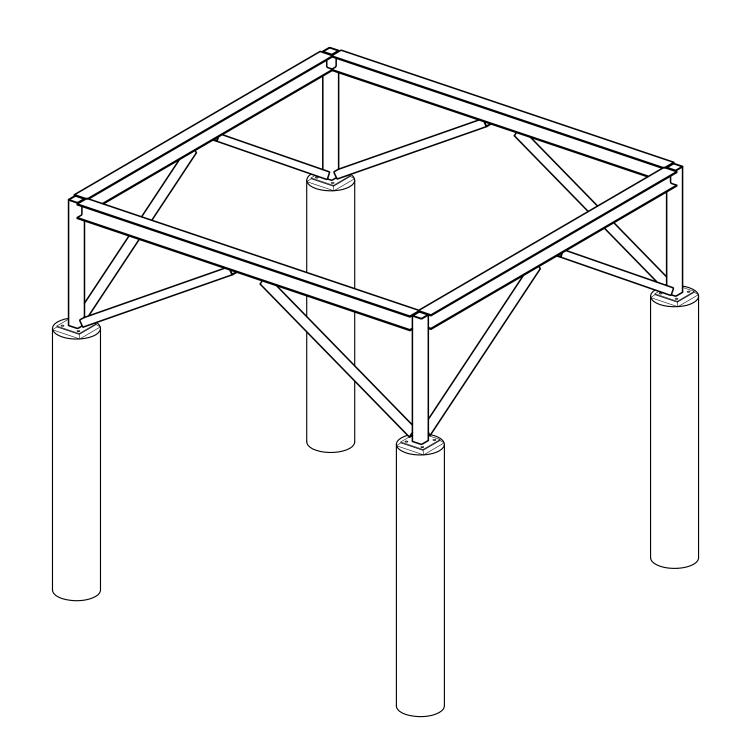




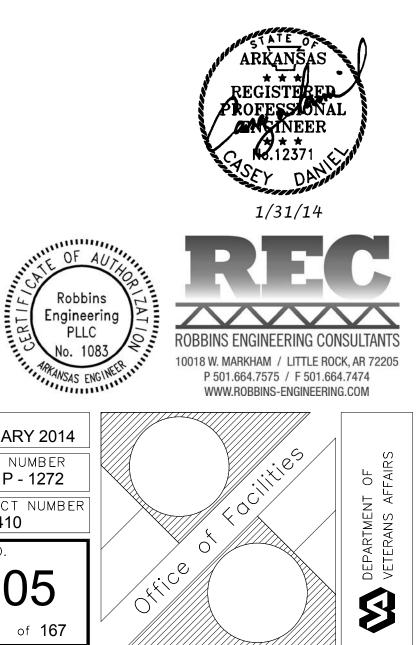
	DRAWING TITLE ENLARGED FRAMING PLANS		EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE		
_					TFCG PROJECT NUM
		BUILDING NUMBER	CHECKED	DRAWN	DRAWING NO.
_		172	CDD	MWO	6304
		LOCATION			J 3304
		│ C.A.V.⊢	I.S., LR DIV	ISION	Dwg 27 of 16



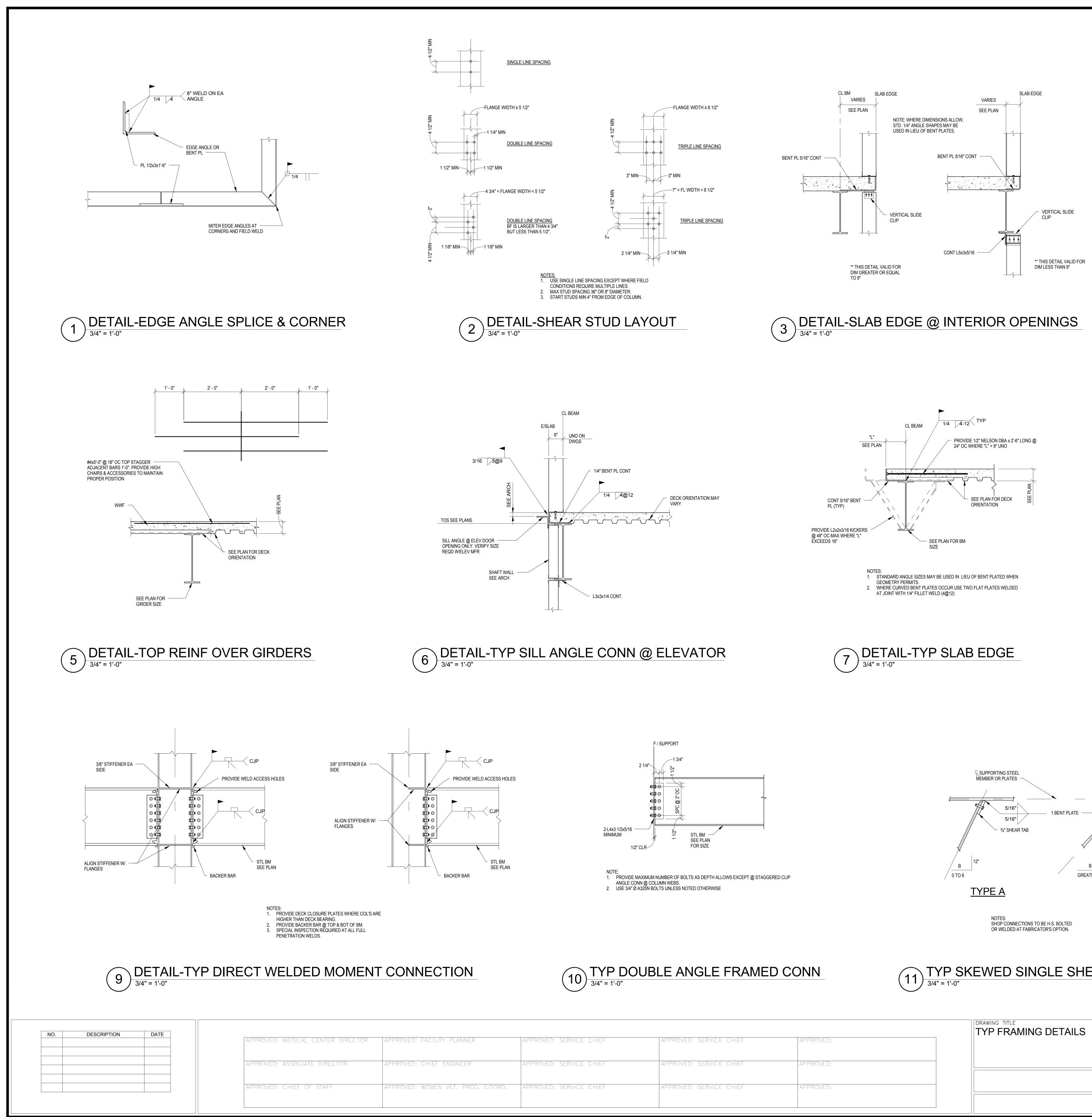
OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
OVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
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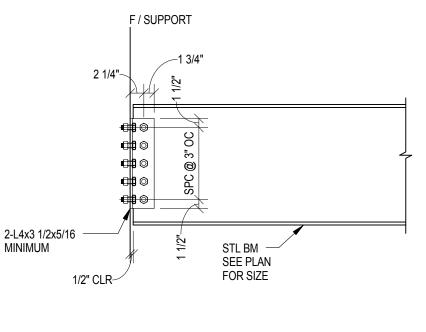
3 3D PERSPECTIVE OF COOLING TOWER FRAME

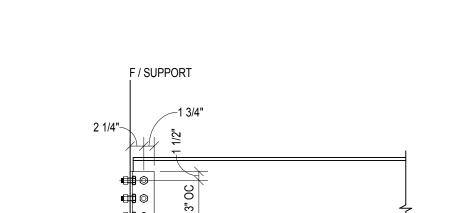


BUILDING NUMBER CHECKED DRAWN DRAWING NO.	DRAWING TITLE COOLING TOWER FRAMING PLAN	PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE BUILDING			DATE 01 FEBRUARY 2 VA PROJECT NUMB VA 256 - P - 12 TFCG PROJECT NU 84410
		BUILDING NUMBER	CHECKED CDD	DRAWN MWO	DRAWING NO.
		C.A.V.H	I.S., LR DIV	ISION	Dwg 28 of 1

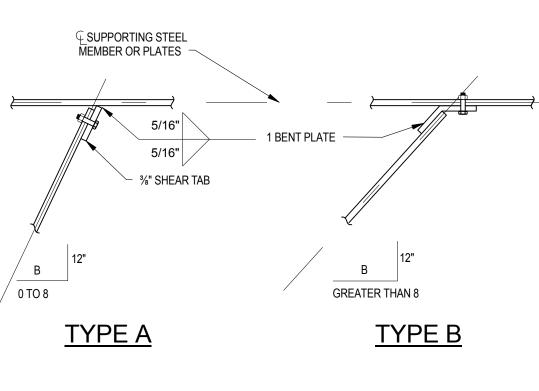


				DRAWING TITLE TYP FRAMING DETAILS	PROJECT TITLE EUGENE J. TOWB			DATE 01 FEBRUARY 2014
OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		BUILDING		VA PROJECT NUMBER VA 256 - P - 1272	
IVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:				TFCG PROJECT NUMB 84410	
) VED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		BUILDING NUMBER	CHECKED CDD	DRAWN MWO	DRAWING NO. S401
					LOCATION	H.S., LR DIVI	SION	J4UI Dwg 29 of 167



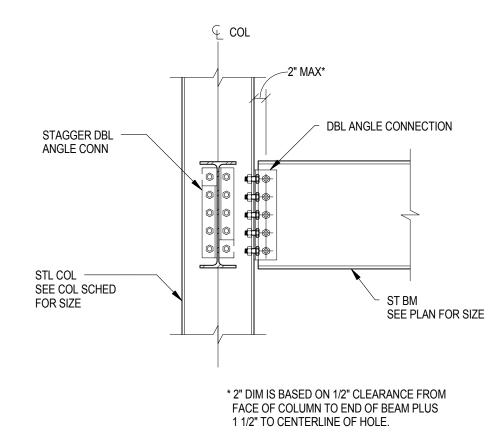


11 TYP SKEWED SINGLE SHEAR PLATE BM CONN

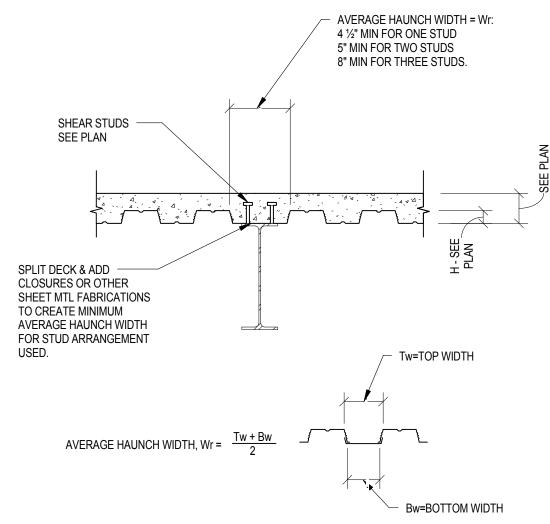


8 TYP BEAM TO COL CONN

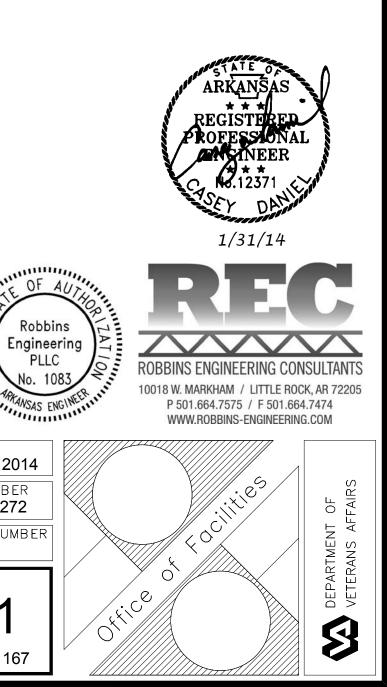
NOTES: 1. PROVIDE MAXIMUM NUMBER OF BOLTS AS WEB DEPTH ALLOWS. 2. OVERSIZED & LONG-SLOTTED HOLES NOT PERMITTED.

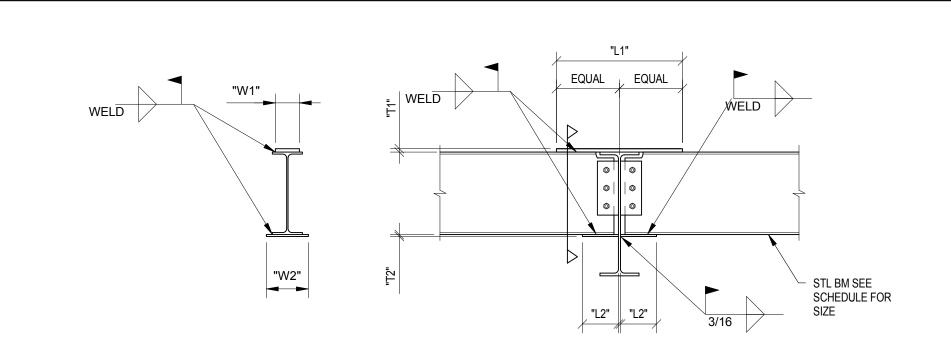


4 TYP HAUNCH @ COMPOSITE GIRDERS



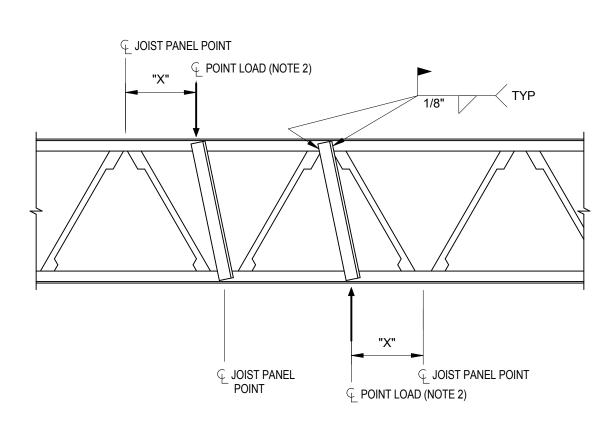






SCHEDULE							
BM SIZE	L1 (IN)	T1 (IN)	W1 (IN)	L2 (IN)	T2 (IN)	W2 (IN)	WELD (IN)
W12	21	11/16	3	6	3/8	6	1/4
W14	21	9/16	4	6	5/16	7	1/4
W16	25	3/4	4 1/2	8	7/16	7 1/2	5/16
W18	30	13/16	5	10	9/16	8	5/16
W21	33	7/8	5 1/2	12	5/8	8 1/2	5/16
W24	35	7/8	6	13	5/8	9	5/16



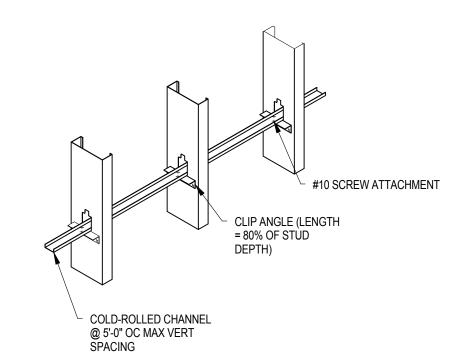


NOTES: POINT LOADS ARE NOT PERMITTED EXCEPT WHERE INDICATED ON STRUCTURAL DRAWINGS OR SPECIFICATIONS. 2. FOR POINT LOADS IN EXCESS OF 250#, PROVIDE 2"x2"x1/4" STRUT WHEN

DISTANCE "X" EXCEEDS 4". 3. IF STRUT FOULS WITH EXISTING DOUBLE ANGLE WEBS, A 1"Ø ROD MAY BE PLACED BETWEEN JOIST CHORDS FOR BOTTOM CHORD LOADS. CONTACT STRUCTURAL ENGINEER FOR ALTERNATIVE STRUT CONFIG FOR TOP CHORD

LOADS.





SPAN 0'-0" TO 6'-0" 6'-0" TO 8'-0" 8'-0" TO 10'-0" 10'-0" TO 12"-0"

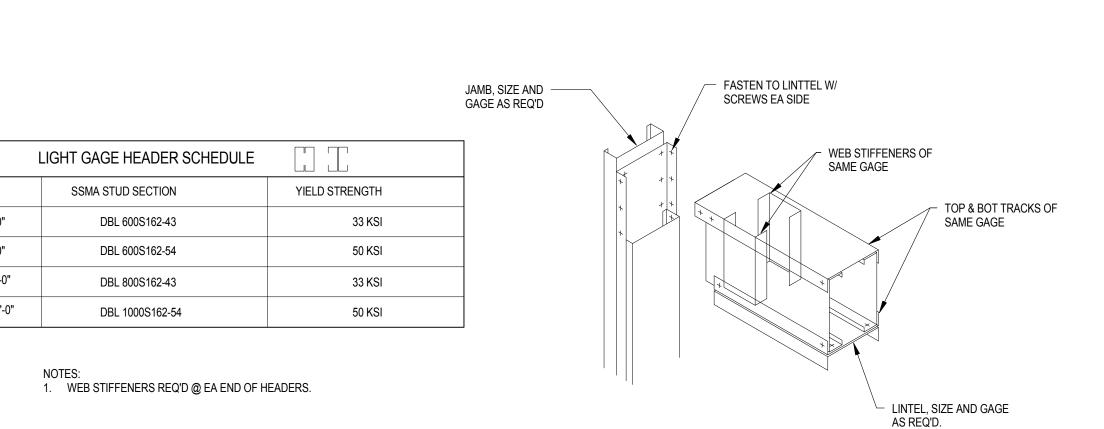
9 DETAIL-TYP WALL BRIDGING



NO.	DESCRIPTION	DATE		
			APPROVED: MEDICAL CENTER DIRECTOR	APPROVE
			APPROVED: ASSOCIATE DIRECTOR	APPROVE
			APPROVED: CHIEF OF STAFF	APPROVI

ED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
ED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
'ED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

$\underbrace{10}_{3/4" = 1'-0"} DETAIL-NON LOAD BEARING LIGHT GAGE HEADER SCHED.$

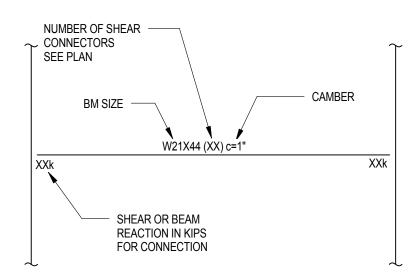


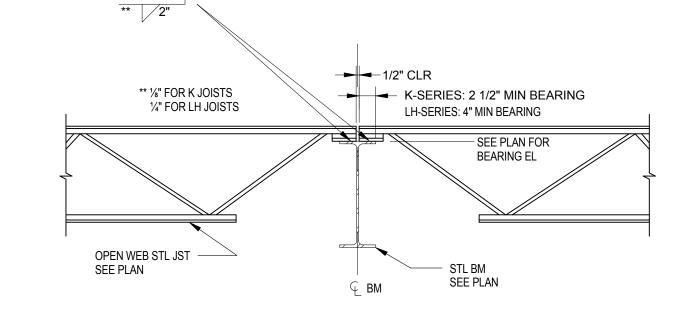




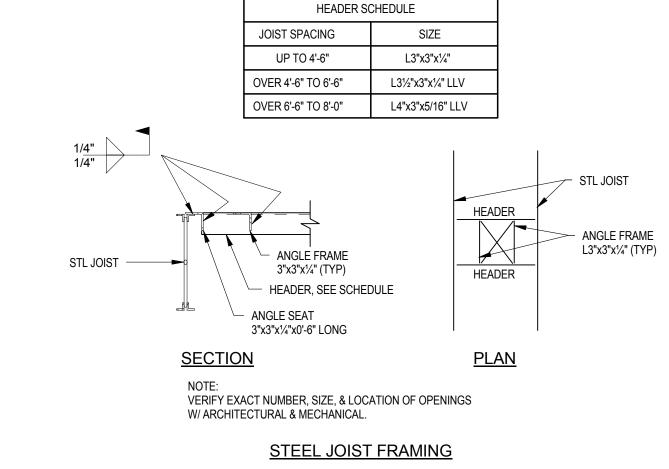
ONE SIDE.

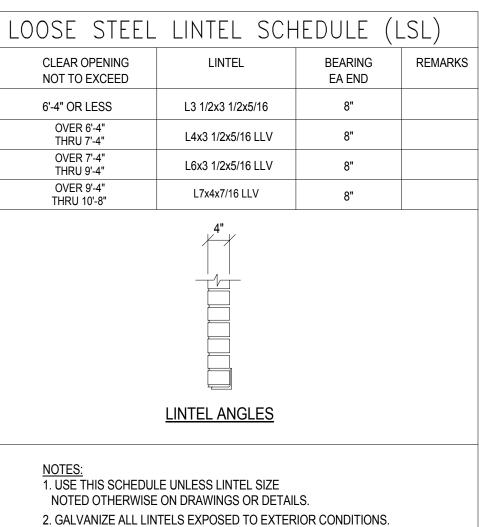
TO OBTAIN MIN BEARING.



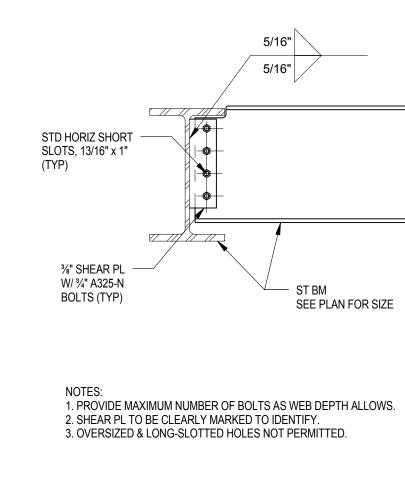




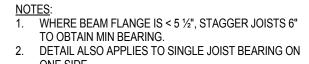




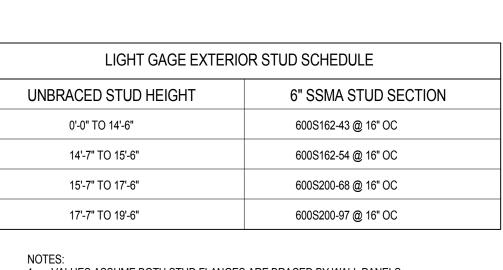
3 LOOSE LINTEL SCHEDULE





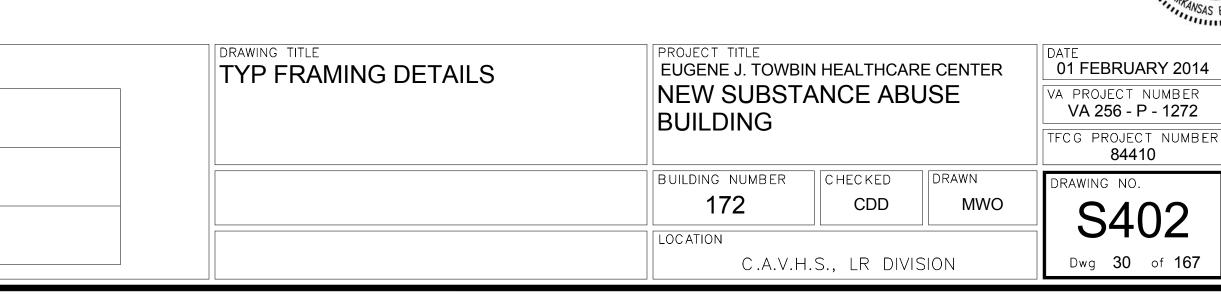


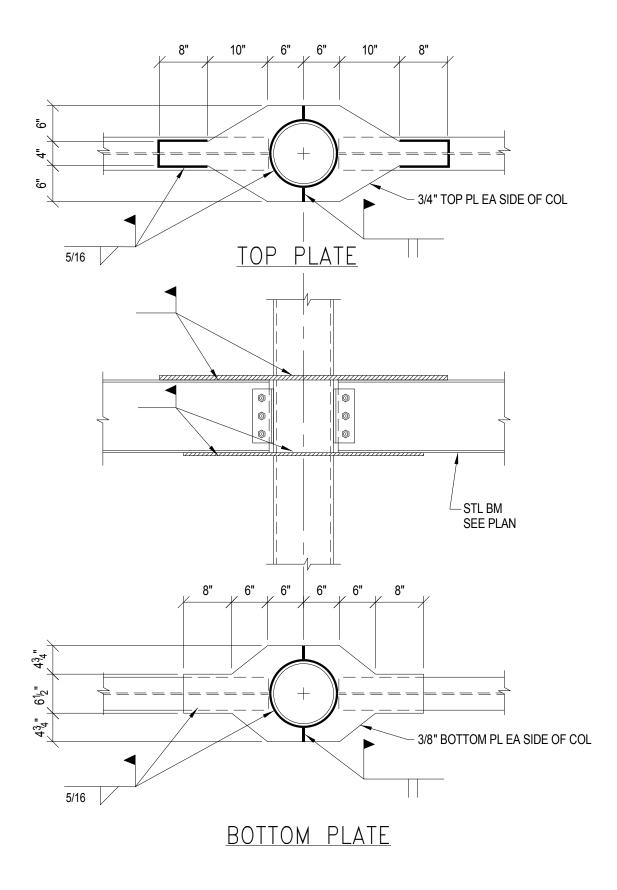
7 TYP JOIST BEARING ON STL BEAMS



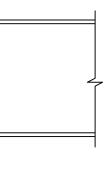
 VALUES ASSUME BOTH STUD FLANGES ARE BRACED BY WALL PANELS.
 HORIZONTAL BRIDGING ASSUMED @ 48" OC MAX SPACING. 3. SEE S001 FOR ADDITIONAL LIGHT GAGE GENERAL NOTES, INCLUDING THE REQUIREMENTS FOR DEFLECTION CLIPS.

11 DETAIL-EXTERIOR WALL STUD SCHEDULE





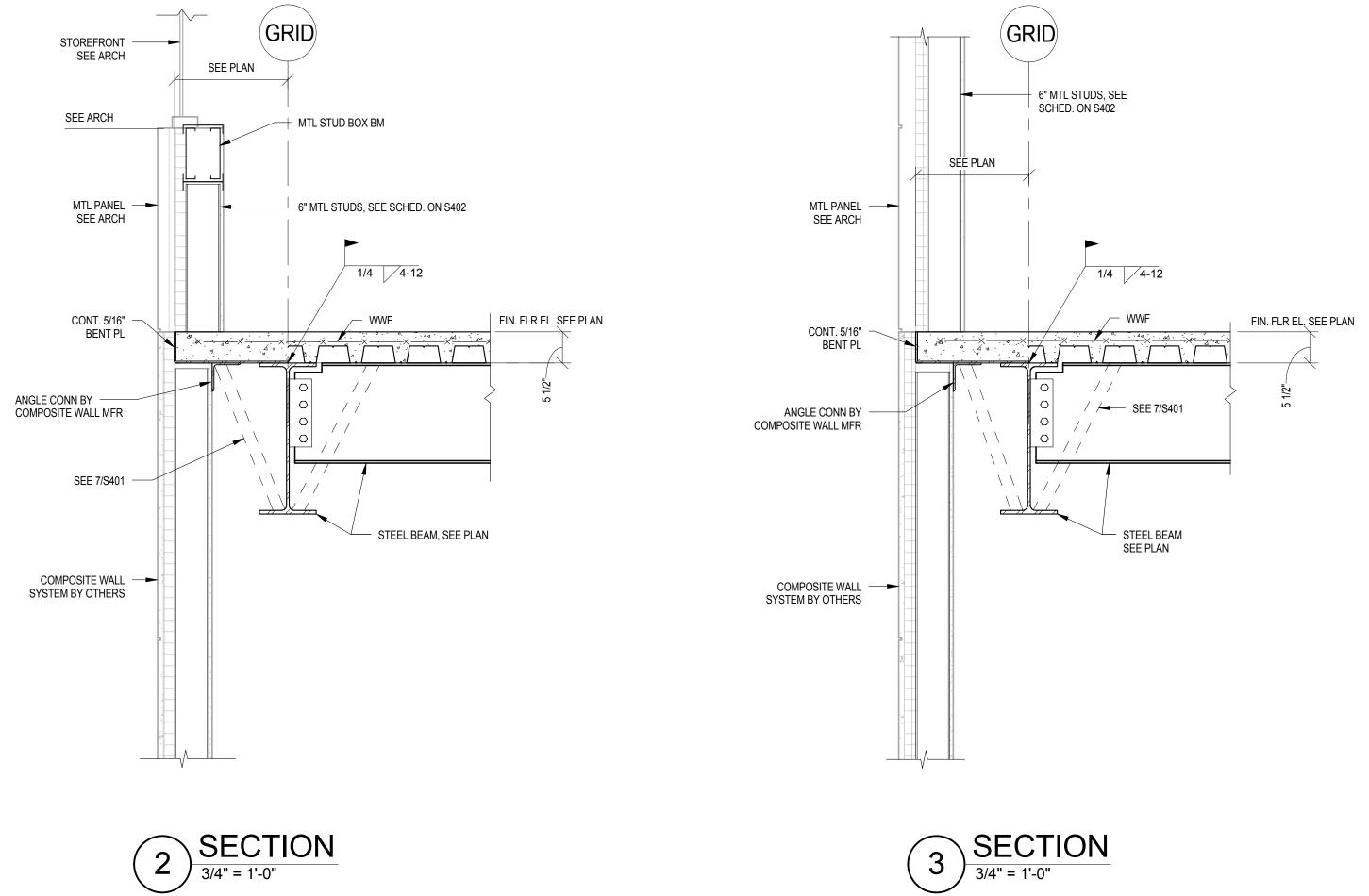


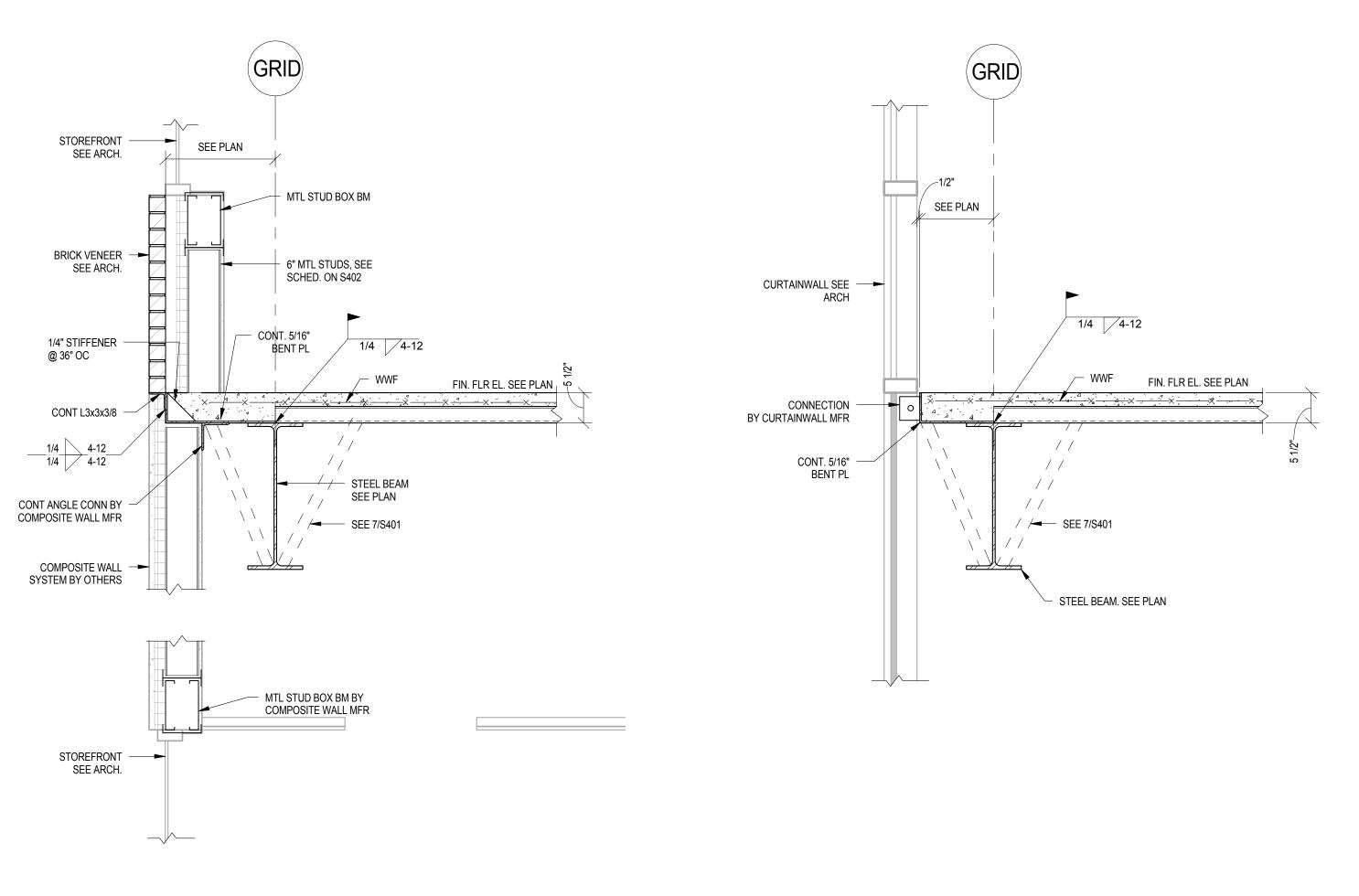






STOREFRONT SEE ARCH ELEV. SEE ARCH MTL PANEL CONT. 5/16" BENT PL COMPOSITE WALL SYSTEM BY OTHERS	SEE 7/S401
1	SECTION 3/4" = 1'-0"
CURTAINWALL SEE ARCH. CONNECTION BY CURTAINWALL MFR CONT. 5/16" BENT PL SEE 7/S401	PLAN
	SECTION 3/4" = 1'-0"

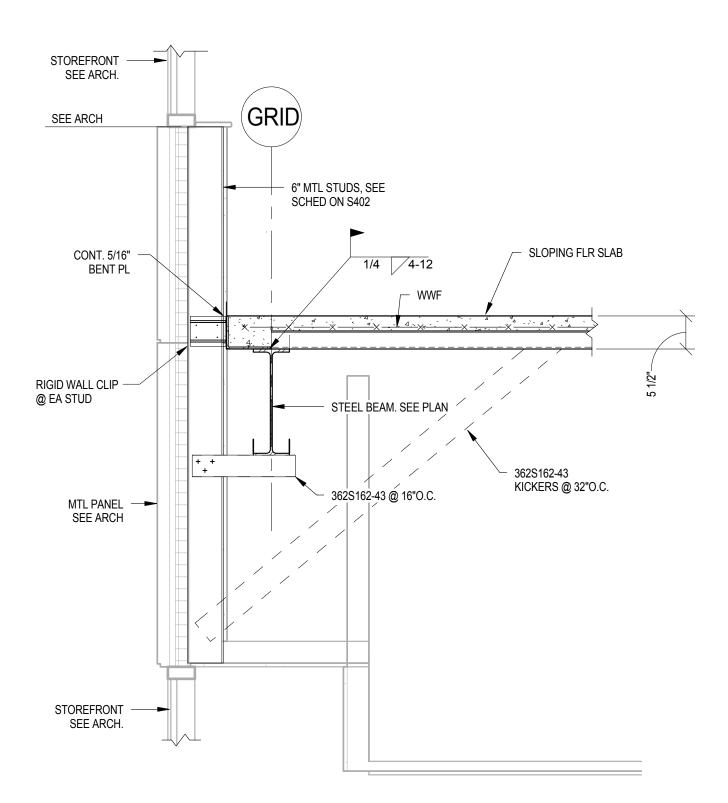


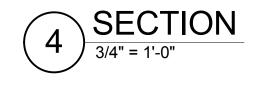


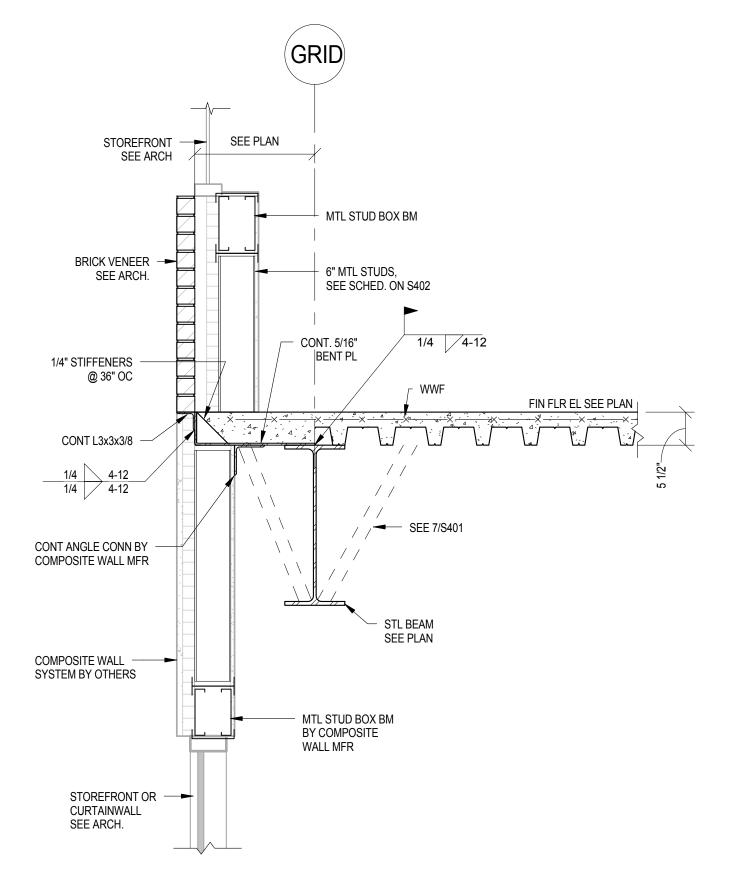


$\overline{7}$	SECT
\bigcirc	3/4" = 1'-0"

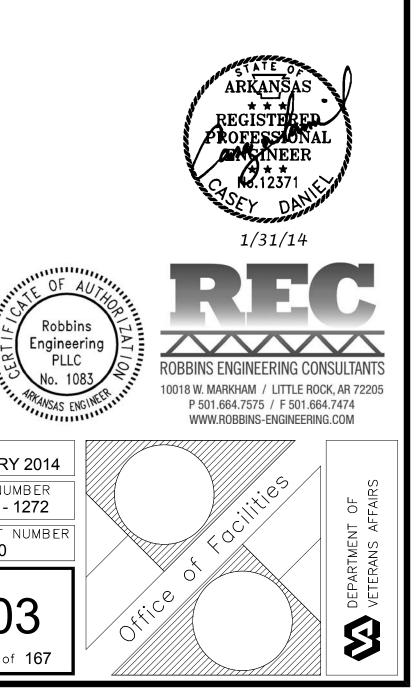
				DRAWING TITLE FRAMING SECTIONS	PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER			R DATE 01 FEBRUARY 201	
OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		NEW SUBST	NEW SUBSTANCE ABUSE			
OVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		BUILDING			TFCG PROJECT NUMB 84410	
					BUILDING NUMBER	CHECKED		DRAWING NO.	
OVED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:		LOCATION	CDD	MWO	S403	
					C.A.V.H	I.S., LR DIVI	SION	Dwg 31 of 167	

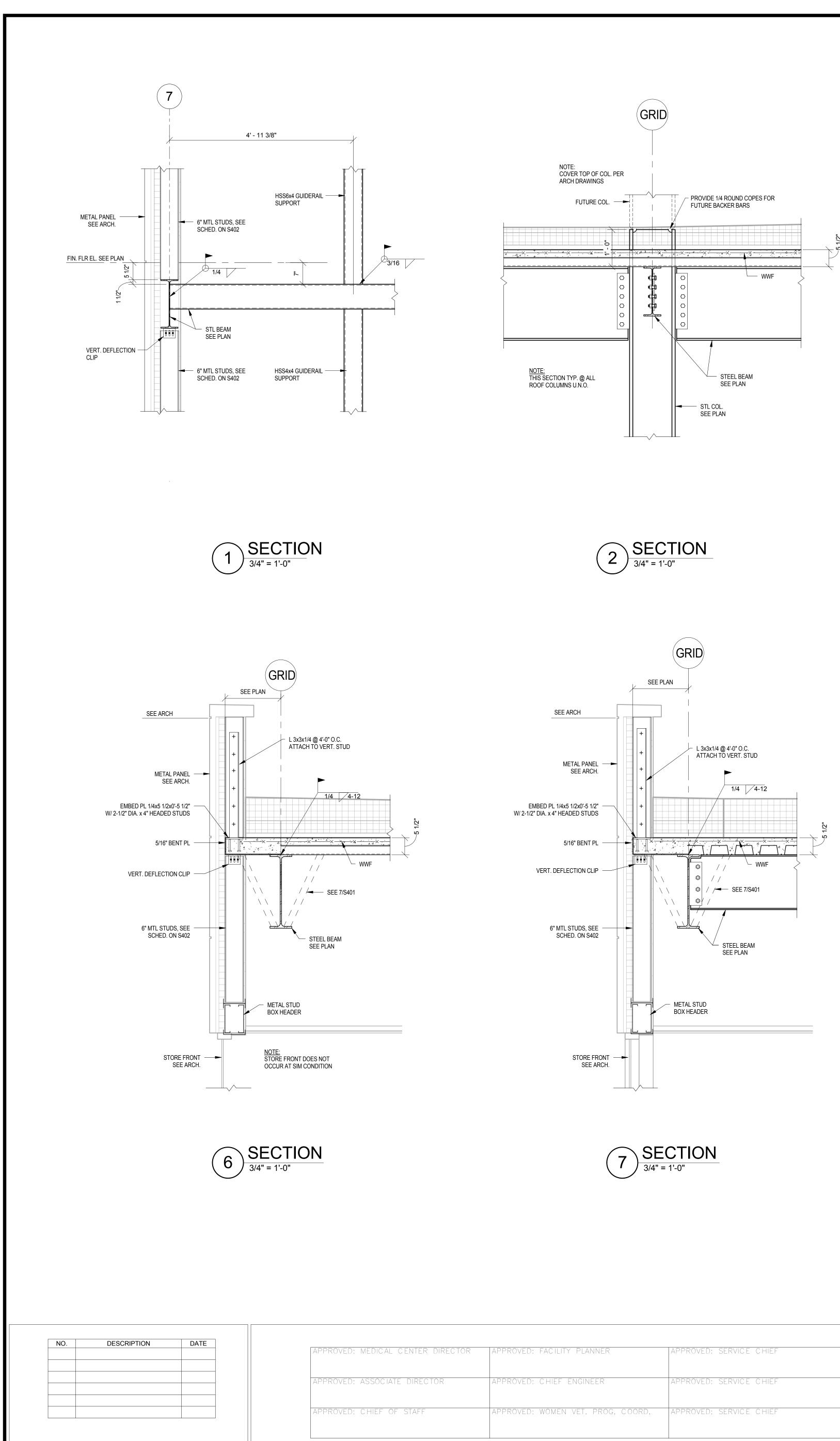


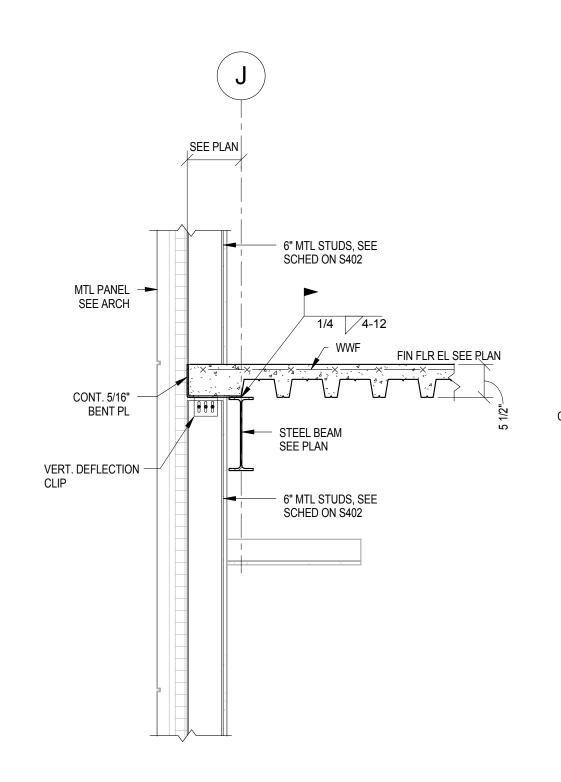


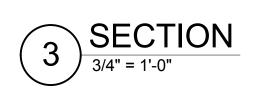


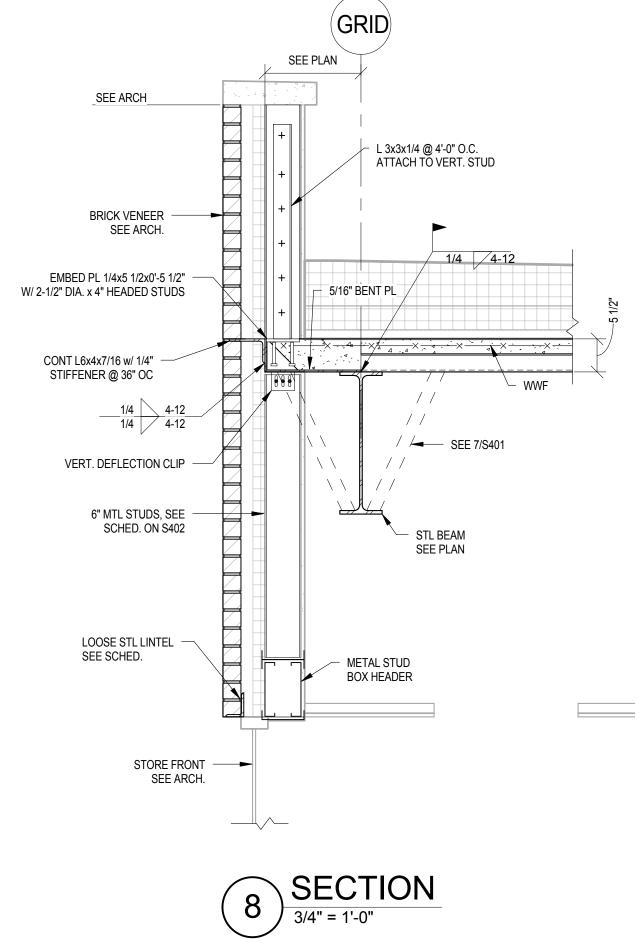
8 SECTION 3/4" = 1'-0"



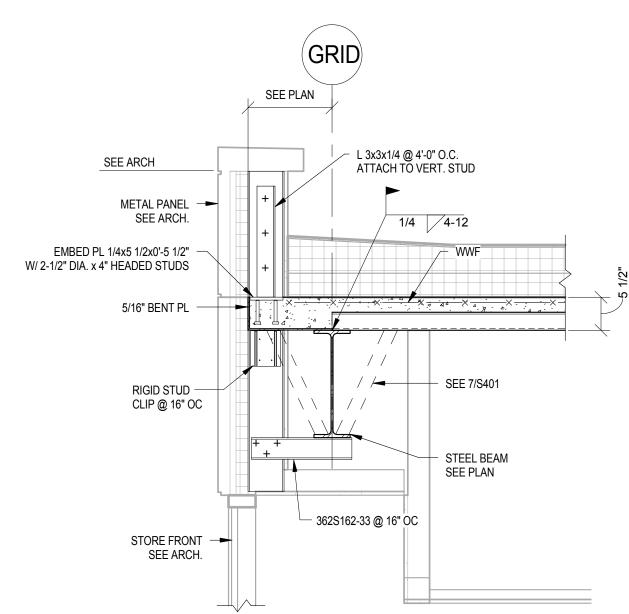


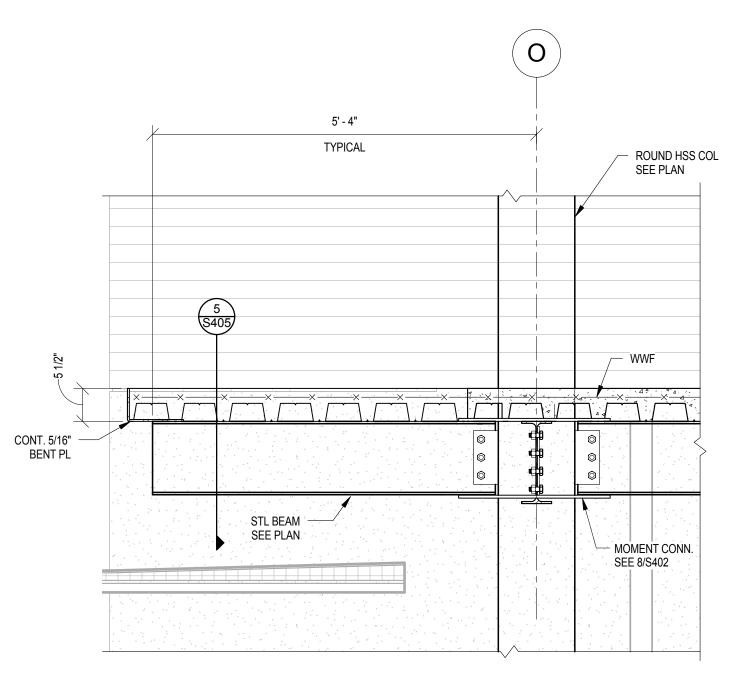




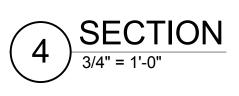


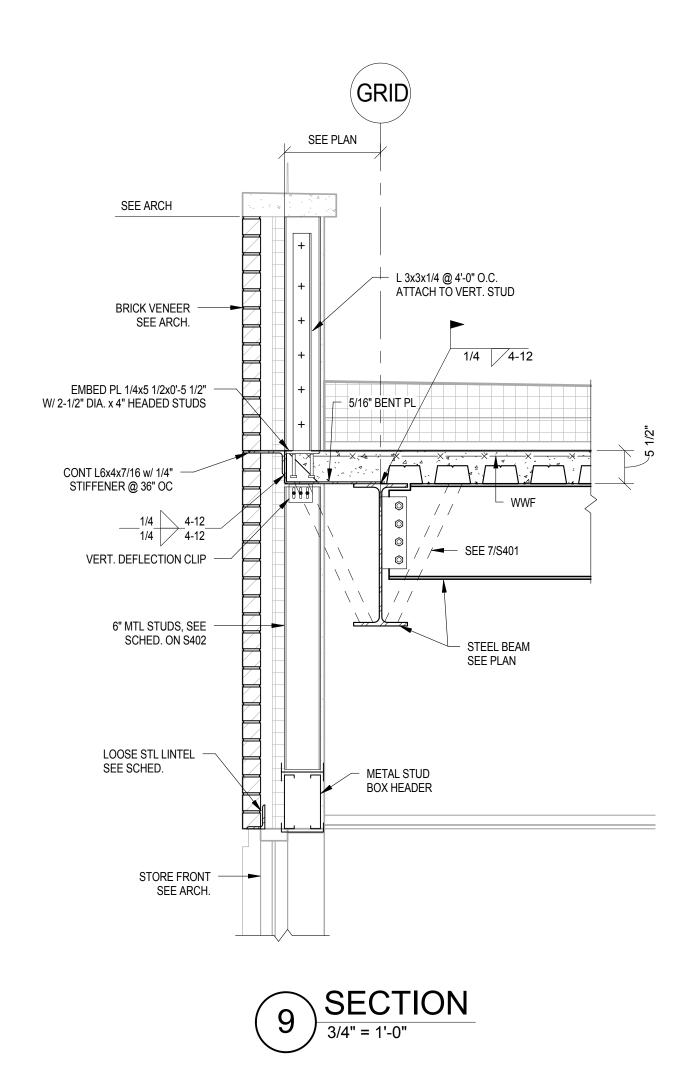
'ED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
'ED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

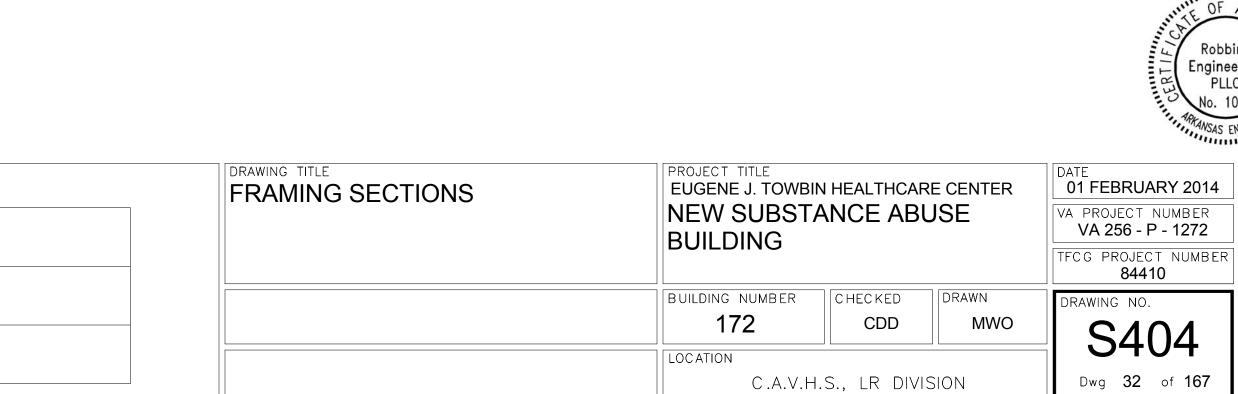


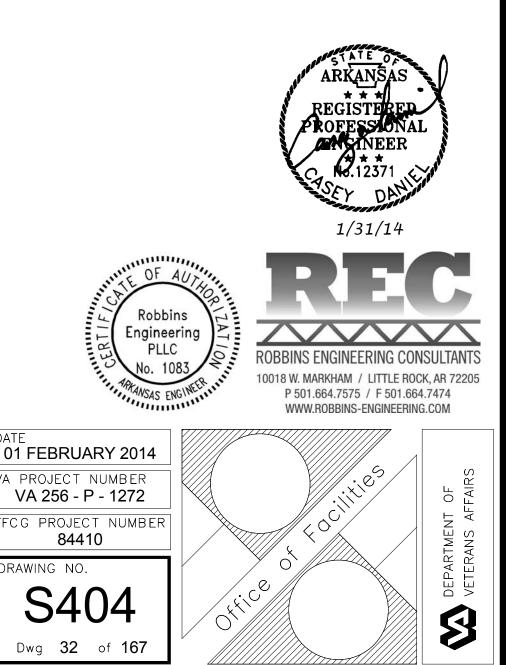


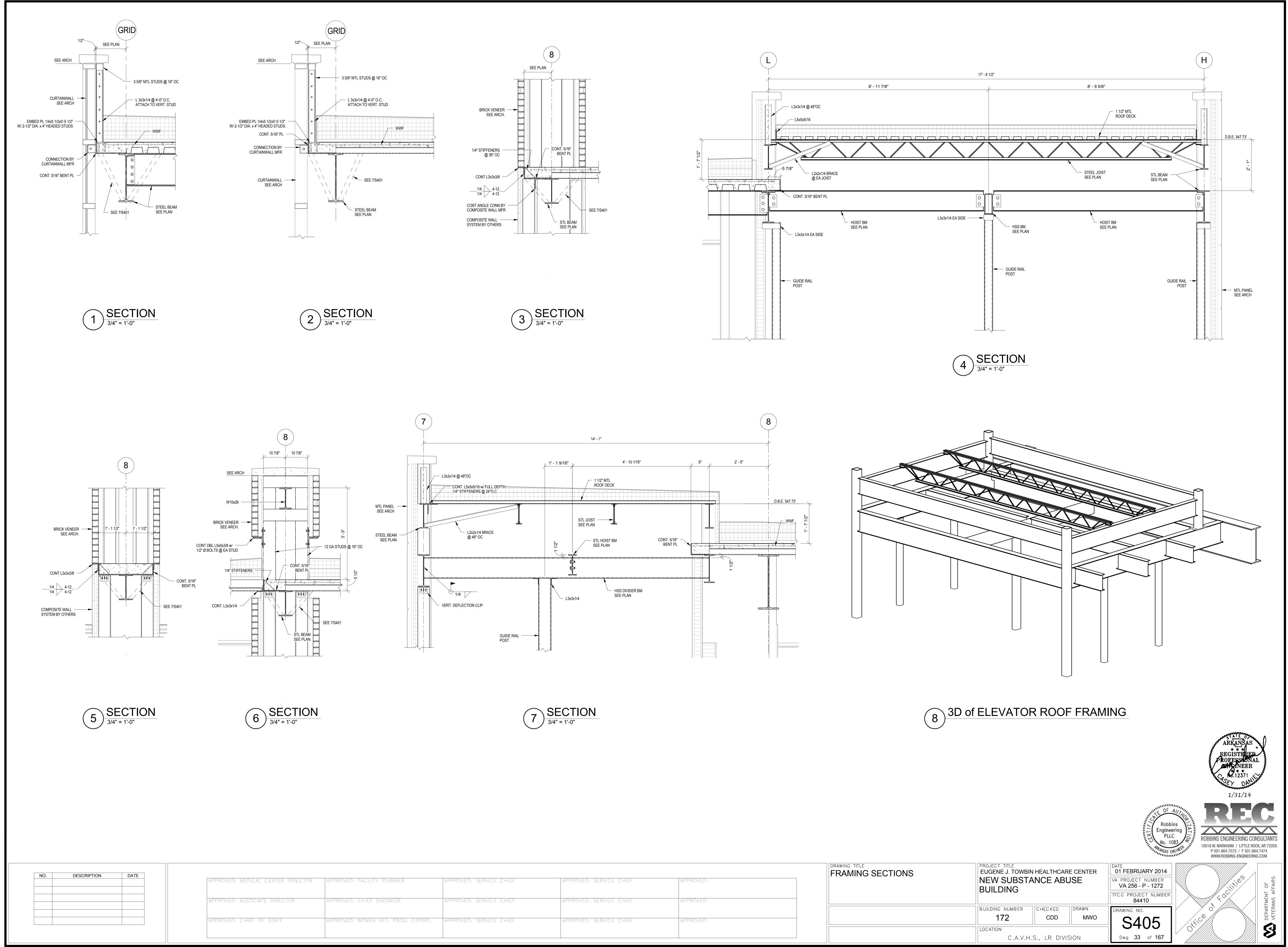
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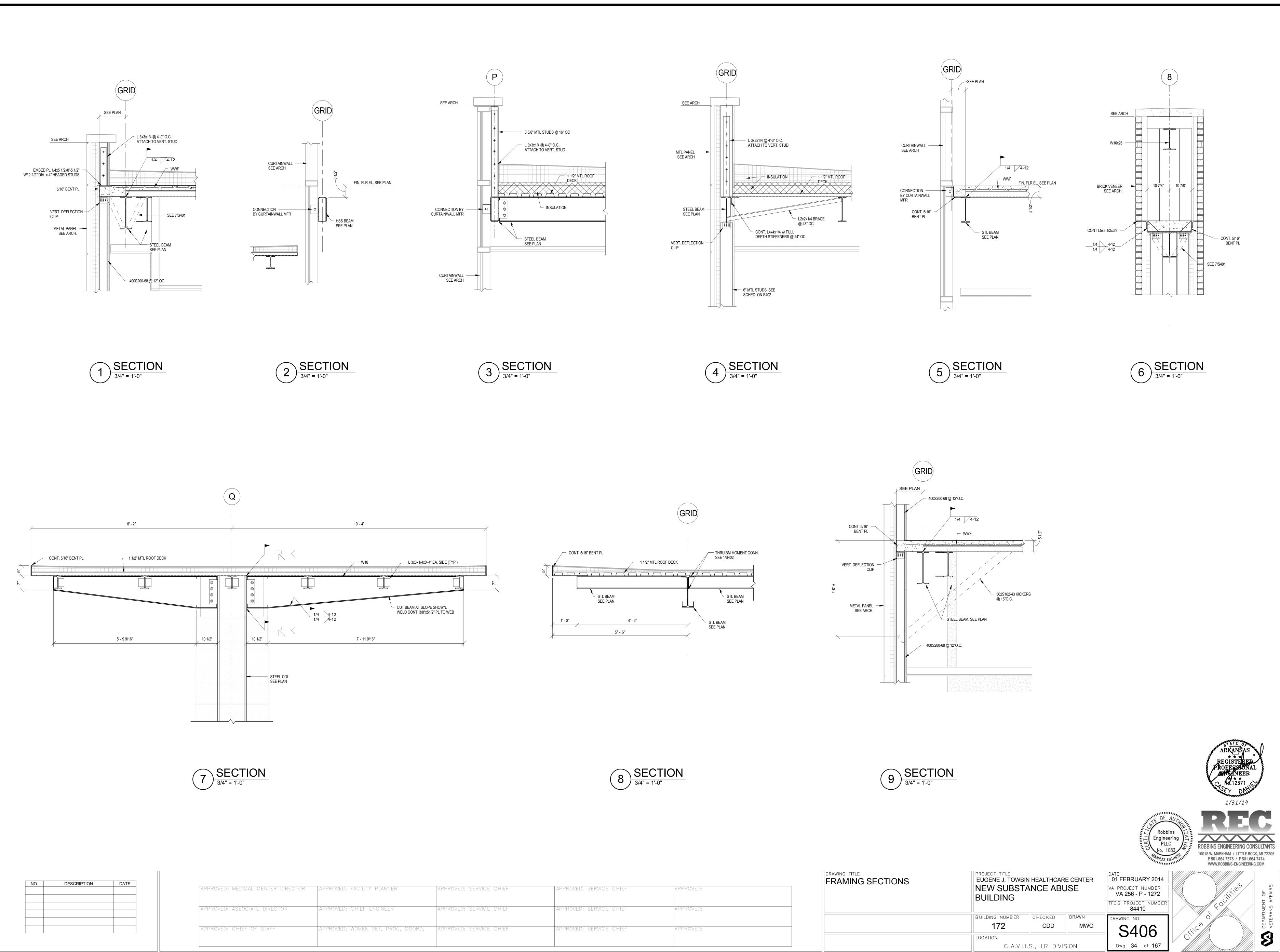




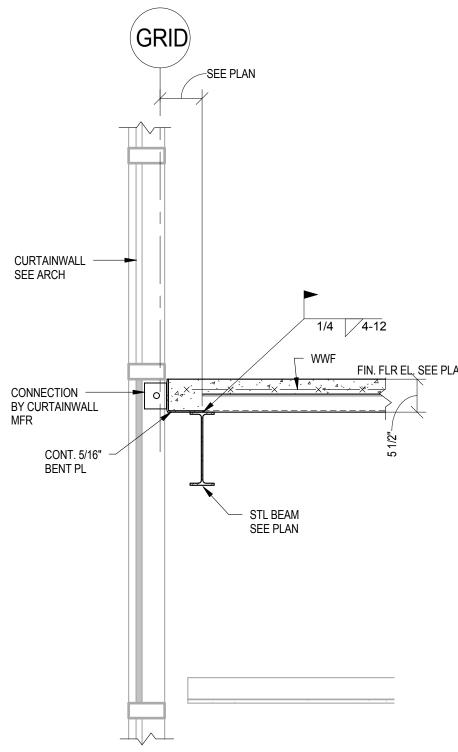
VED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
VED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
VED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

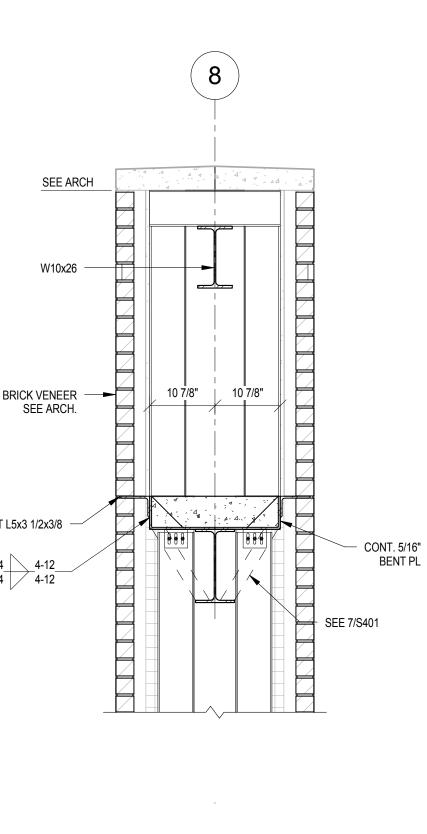
$\overline{7}$	SECTION
\bigcup	3/4" = 1'-0"

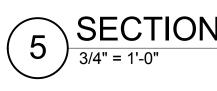
				· · · · ·
DRAWING TITLE FRAMING SECTIONS	PROJECT TITLE EUGENE J. TOWBI	IN HEALTHCA	RE CENTER	DATE 01 FEBRUARY 201
	NEW SUBST	ANCE AB	USE	VA PROJECT NUMBER VA 256 - P - 1272
	DOILDING			TFCG PROJECT NUMB 84410
	BUILDING NUMBER	CHECKED	DRAWN	DRAWING NO.
	172	CDD	MWO	S405
	LOCATION			
	C.A.V.⊢	I.S., LR DIVI	ISION	Dwg 33 of 167

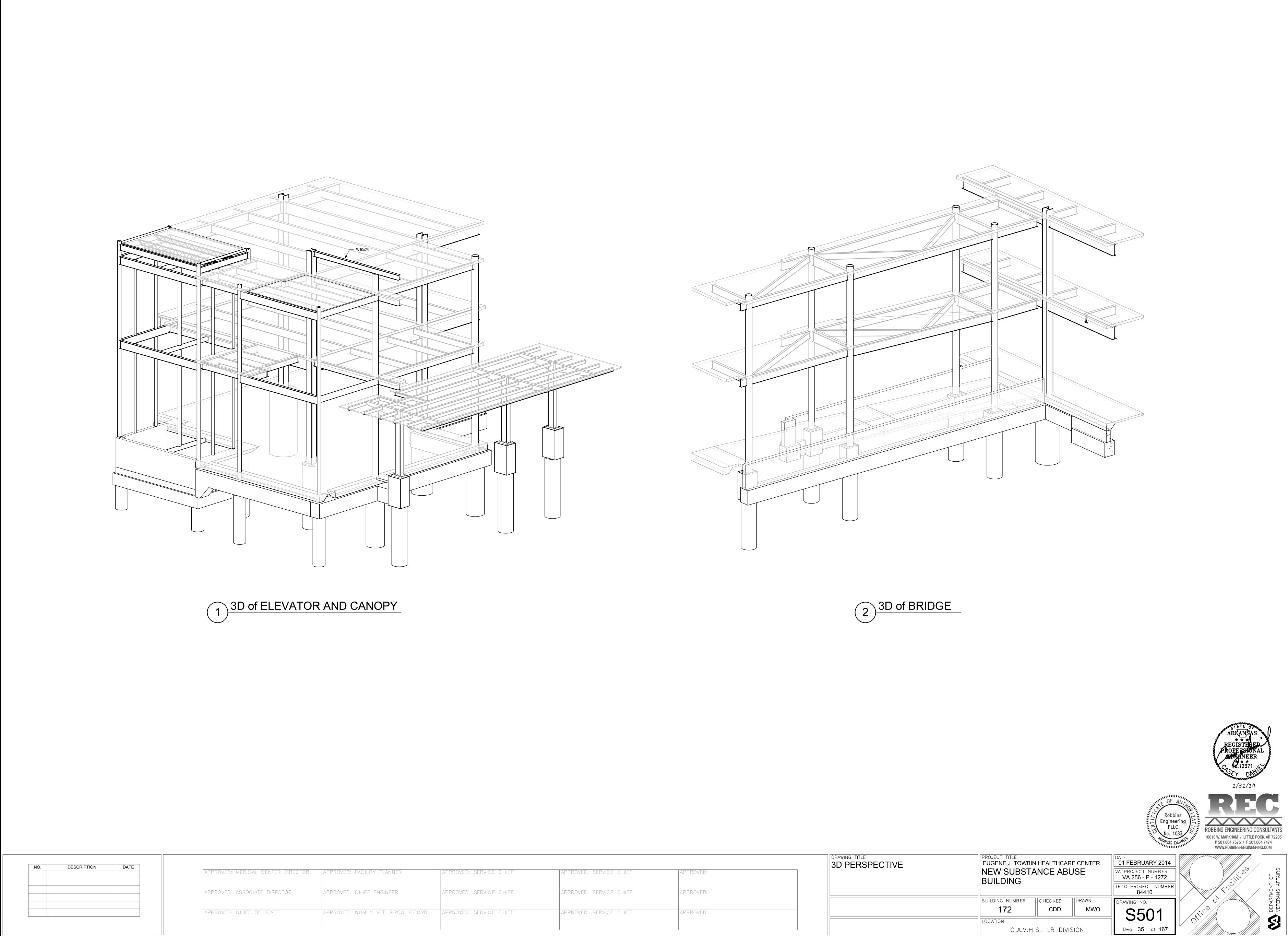


OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
DVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
DVED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:



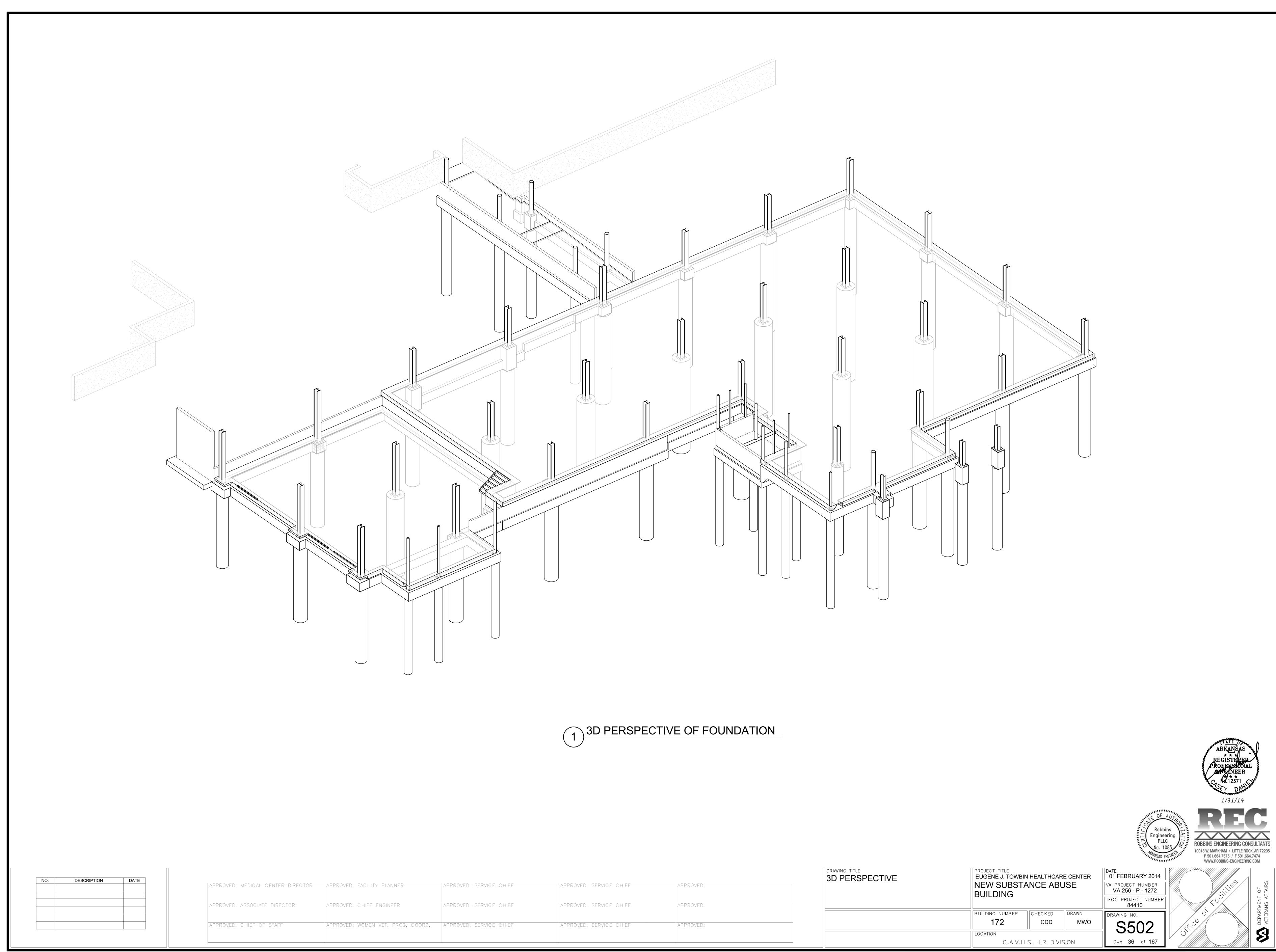






OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
)VED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
)VED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:

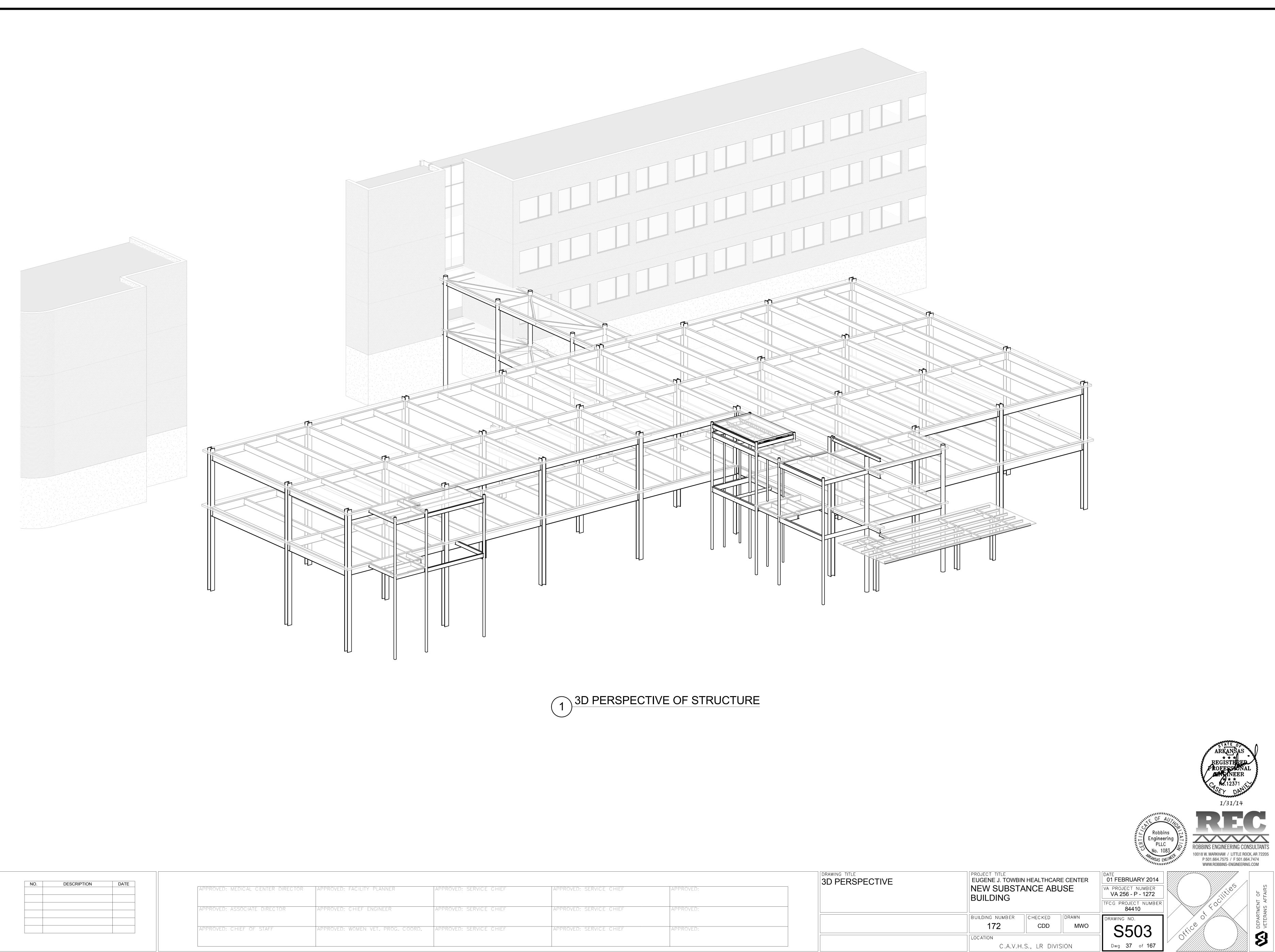
	DRAWING TITLE 3D PERSPECTIVE	PROJECT TITLE EUGENE J. TOWBIN HEALTHCARE CENTER NEW SUBSTANCE ABUSE BUILDING			DATE 01 FEBRUARY 20 VA PROJECT NUMBE VA 256 - P - 127	
_		DOILDING			TFCG PROJECT NUM	
		BUILDING NUMBER	CHECKED	DRAWN	DRAWING NO.	
_		172	CDD	MWO	S501	
		С.А.V.Н	.S., LR DIVI	SION	Dwg 35 of 16	



OVED: FACILITY PLANNER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
OVED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
DVED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:







ved: facility planner	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
VED: CHIEF ENGINEER	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED:
VED: WOMEN VET. PROG. COORD.	APPROVED: SERVICE CHIEF	APPROVED: SERVICE CHIEF	APPROVED: