GENERAL NOTES

GENERAL

1. THIS BUILDING HAS BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE MICHIGAN BUILDING CODE, 2009 EDITION.
2. THE CONTRACTOR SHALL VISIT THE SITE TO BECOME FAMILIAR WITH THE EXISTING CONDITIONS, THE OWNERS REQUIREMENTS FOR ACCESS TO THE SITE.
3. THE STRUCTURE SHALL BE CONSIDERED TO BE IN AN UNSTABLE CONDITION UNTIL ALL FLOOR, WALL AND ROOF STRUCTURES ARE COMPLETED. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR STABILITY AND TO RESIST LATERAL LOADS DURING ERECTION.
4. ALL NON LOAD BEARING WALLS, EXCEPT INDICATED SHEAR WALLS SHALL BE CONSTRUCTED TO ALLOW FOR VERTICAL DEFLECTION OF THE STRUCTURE ABOVE.

FOUNDATIONS/BACKFILL

1. FOUNDATIONS ARE DESIGNED FOR A MAXIMUM ALLOWABLE BEARING CAPACITY OF 2000 P.S.F. AND ARE TO BEAR ON UNDISTURBED, NATURAL SOILS OR COMPACTED ENGINEERED FILL.
2. THE OWNER WILL RETAIN THE SERVICES OF A GEOTECHNICAL ENGINEER TO MONITOR THE FOUNDATION WORK AND DETERMINE THE QUALITY OF THE SOIL AT ALL FOOTING AND SLAB LOCATIONS. IF UNSUITABLE MATERIALS ARE ENCOUNTERED AT THE FOOTING LOCATIONS, THE UNSUITABLE MATERIAL SHALL BE REMOVED AND REPLACED OR THE FOOTINGS LOWERED AT THE DIRECTION OF THE ENGINEER.
3. CONTRACTOR SHALL BE AWARE OF AND VERIFY LOCATION OF ALL UNDERGROUND UTILITIES, TANKS, ETC. DUE CARE SHALL BE EXERCISED DURING EXCAVATION SUCH THAT EXISTING UTILITIES ARE NOT DAMAGED. GROUND PENETRATING RADAR SHALL BE USED FOR THIS VERIFICATION.
4. ALL EXCAVATIONS SHALL CONFORM TO MI-OSHA REQUIREMENTS. ANY PERCHED GROUNDWATER ENTERING THE EXCAVATION SHALL BE PUMPED PRIOR TO PLACING CONCRETE.
5. ALL BACKFILL MATERIALS SHALL CONFORM TO MDOT CLASS II MATERIAL. ALL BACKFILL SHALL BE PLACED IN 12" LOOSE LIFTS AND COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY DETERMINED IN ACCORDANCE WITH ASTM D-1557 (MODIFIED PROCTOR). FIELD DENSITY TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM D-2922 OR D-1556 WITH A MINIMUM OF 1 TEST PER 1500 SQ. FEET OF AREA PER 12" LIFT (MINIMUM OF (3) TESTS PER LIFT).

CONCRETE

1. THE LATEST REVISION OF THE FOLLOWING CODES GOVERN THE DESIGN, DETAILING, FABRICATION AND CONSTRUCTION OF ALL REINFORCED CONCRETE:
   - A. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE: ACI 318.
   - B. DETAILS & DETAILING OF CONCRETE REINFORCEMENT: ACI 315.
2. ALL DEFORMED BAR REINFORCING SHALL BE ASTM A 615 GRADE 60.
3. ALL WELDED WIRE FABRIC SHALL BE ASTM A 185 FLAT SHEETS ONLY.
4. ALL CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'c) OF 4000 PSI, 3" TO 5" SLUMP AND A WATER/CEMENT RATIO OF 0.44 MAXIMUM. ALL EXTERIOR CONCRETE SHALL BE AIR ENTRAINED (5% ± 1%). CONTRACTOR SHALL SUBMIT A CONCRETE MIX DESIGN WITH APPROPRIATE TEST DATA FOR APPROVAL PRIOR TO CONCRETE PLACEMENT.
5. SPLICES FOR DEFORMED BARS SHALL BE CLASS B WITH APPLICABLE INCREASES FOR BAR SPACING, COVER, TOP BAR EFFECT ETC. PER LATEST EDITION ACI 318.
6. BEFORE PLACING CONCRETE REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPE SLEEVES, EMBEDDED ITEMS, OPENINGS, EQUIPMENT PADS, ELECTRICAL CONDUITS, RECESSES, DRAINS, ETC. ALL OPENINGS FOR PIPE, CONDUITS, ETC. SHALL BE SLEEVED. MINIMUM SLEEVE SPACING SHALL BE 3 SLEEVE DIAMETERS.
7. CONCRETE CONTROL JOINTS SHALL BE CUT AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT DISLODGMENT OF AGGREGATES. SAW A CONTINUOUS SLOT TO A DEPTH OF 1/4 THE THICKNESS OF THE SLAB BUT NOT LESS THAN 1". COMPLETE SAWING WITHIN 12 HOURS AFTER PLACEMENT.
8. PROVIDE A RECESS IN THE TOP OF FOUNDATION WALLS AT DOOR OPENINGS FOR SUPPORT OF THICKENED FLOOR SLABS. DEPTH OF RECESS TO BE 2" MINIMUM GREATER THAN THICKNESS OF THE FLOOR SLAB, UNLESS NOTED OTHERWISE IN DETAILS.
9. SUGGESTED CONSTRUCTION AND CONTROL JOINT LOCATIONS ARE INDICATED ON THE DRAWINGS. CONTRACTOR MAY DEVIATE FROM SUGGESTED JOINT LOCATIONS WITH PRIOR APPROVAL OF THE CONTRACTING OFFICER TECHNICAL REPRESENTATIVE.
10. PROVIDE BENT CORNER BARS IN ALL WALLS AND FOOTINGS OF THE SAME SIZE AND NUMBER AS THE CONTINUOUS REINFORCEMENT.
11. CONCRETE SHALL BE TESTED BY AN INDEPENDENT TESTING AGENCY. A SET OF (3) CONCRETE TEST CYLINDERS SHALL BE MADE AND TESTED FOR COMPRESSION STRENGTH AT 7 AND 28 DAYS OR EVERY 50 CUBIC YARDS OF CONCRETE CAST (MINIMUM OF (1) SET PER DAY OF CASTING). ALSO SLUMP AND UNIT WEIGHT TESTS SHALL BE PERFORMED EVERY OTHER TRUCK LOAD. CONTRACTOR MADE CONCRETE TEST CYLINDERS ARE NOT ACCEPTABLE.

MASONRY

1. THE LATEST REVISION OF THE FOLLOWING CODES GOVERN THE DESIGN, DETAILING AND CONSTRUCTION OF ALL MASONRY:
   - A. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES: ACI 530
   - B. SPECIFICATIONS FOR MASONRY STRUCTURES: ACI 530.1
2. ALL MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF F'm = 2500 PSI.
3. ALL MORTAR SHALL BE TYPE S, PROPORTIONED BY VOLUME ACCORDING TO ASTM C 270.
4. ALL GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI, AND SHALL BE PROPORTIONED BY VOLUME ACCORDING TO ASTM C 476.
5. ALL CONCRETE MASONRY UNITS SHALL BE MEDIUM OR HEAVY WEIGHT ASTM C 90, GRADE N, TYPE 1 UNITS UNLESS NOTED OTHERWISE. UNITS SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2800 PSI.
6. ALL MASONRY WALLS SHALL HAVE HORIZONTAL JOINT REINFORCEMENT (9 GA., HOT DIPPED GALVANIZED) AT 16" O.C. UNLESS NOTED. PROVIDE PREFABRICATED CORNER PIECES AT ALL CORNERS AND INTERSECTIONS OF WALLS.
7. ALL DEFORMED BAR REINFORCING SHALL BE ASTM A 615 GRADE 60. LAP SPLICES IN WALLS SHALL BE DETERMINED IN ACCORDANCE WITH ACI 530, BUT SHALL BE A MINIMUM OF 48 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
8. ALL MASONRY REINFORCING SHALL BE SECURED IN PLACE W/ REBAR POSITIONERS AND SPACERS.
9. ALL VERTICAL MASONRY WALL REINFORCEMENT SHALL BE CENTERED ON THE WALL, DOWELED INTO THE FOOTINGS, SECURED IN PLACE, AND GROUTED SOLID, UNLESS NOTED OTHERWISE ON DETAILS.
10. IN ADDITION TO ALL OTHER REINFORCING IN MASONRY WALLS PROVIDE MINIMUM (1) #5 BAR AT EACH SIDE OF ALL OPENINGS, EACH SIDE OF CONTROL JOINTS, AT CORNERS OR ENDS OF WALLS AND AT BEAM OR LINTEL BEARING. ROD TO BE FULL HEIGHT OF WALL.
11. ALL MASONRY WALLS SHALL HAVE A CONTINUOUSLY REINFORCED BOND BEAM NEAR THE TOP OF THE WALL, WITH (2) #5 BARS U.N.O. PROVIDE BENT CORNER BARS AT ALL BOND BEAM INTERSECTIONS. REFER TO APPROPRIATE DETAILS FOR LOCATION OF BOND BEAM.
12. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING WALL BRACING ADEQUATE TO RESIST LATERAL LOADS.
13. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF WALL CONTROL JOINTS AND EXPANSION JOINTS.
14. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR TYPE, SIZE, LOCATION AND ATTACHMENT REQUIREMENTS FOR MASONRY FASCIA AND OTHER CLADDING.

STRUCTURAL STEEL

1. ALL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST REVISION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDING ALLOWABLE STRESS DESIGN" AND AISC "CODE OF STANDARD PRACTICE STEEL BUILDINGS & BRIDGES".
2. STRUCTURAL STEEL MATERIAL SPECIFICATIONS:
   - A. W AND WT SHAPES: A 992, GRADE 50.
   - B. MISCELLANEOUS SHAPES AND PLATES: ASTM A 36 (Fy=36 KSI).
   - C. PIPE: ASTM A 53, GRADE B, TYPE E OR S (Fy=35 KSI).
   - D. STRUCTURAL STEEL TUBES ASTM A 500 GRADE B (Fy=46 KSI).
3. ALL BUILDING COLUMN ANCHOR RODS SHALL BE ASTM F 1554 (Fy=36 KSI).
4. ALL WELDING SHALL BE PERFORMED USING THE ELECTRIC ARC METHOD IN ACCORDANCE WITH THE LATEST REVISION OF THE AMERICAN WELDING SOCIETY (AWS) D1.1 "STRUCTURAL WELDING CODE". E70xx ELECTRODES CONFORMING TO AWS A5.1 OR A5.5 SHALL BE USED FOR SHIELDED METAL ARC METHOD AND FX7-ECXX FLUX - ELECTRODE COMBINATION CONFORMING TO AWS A5.17 FOR SUBMERGED ARC METHOD.
5. ALL BOLTS SHALL BE 3/4"ø ASTM A325 TYPE N BOLTS. ALL BOLTED CONNECTIONS SHALL BE SNUG-TIGHT BEARING TYPE BOLTS UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS TO HSS SHALL BE 1/2"ø STAINLESS STEEL HOLO-BOLTS.
6. PROVIDE "SLIP-CRITICAL" CONNECTIONS AT BRACING, WHERE BOLTS ARE IN TENSION AND AT MOMENT CONNECTIONS.
7. ALL BEAM CONNECTIONS ARE TO CONFORM TO AISC STANDARD TWO ANGLE WEB CONNECTIONS CAPABLE OF SUPPORTING 75% OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM OR FOR LOADS INDICATED ON DRAWING. NO CONNECTION SHALL CONSIST OF LESS THAN (2) 3/4"ø BOLTS OR A WELD DEVELOPING LESS THAN 10 KIPS.
8. DESIGN HORIZONTAL AND VERTICAL BRACING END CONNECTIONS FOR LOADS INDICATED ON THE DRAWINGS OR 50% OF THE TENSILE CAPACITY OF THE MEMBER WHICHEVER IS GREATER.
9. ALL CONNECTIONS NOT SPECIFICALLY DETAILED, SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE GENERAL DETAILS SHOWN ON THE DRAWINGS ARE APPROXIMATE ONLY AND DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNLESS SPECIFICALLY NOTED.
10. ALL FIELD CONNECTIONS SHALL BE BOLTED UNLESS NOTED OTHERWISE. FIELD WELDING IS NOT ALLOWED EXCEPT WHERE SPECIFICALLY INDICATED OR APPROVED.
11. FIELD WELD (WHERE NOTED) SHALL BE APPLIED TO SURFACES FREE OF PAINT, GREASE, DIRT AND OTHER DELETERIOUS MATERIAL.
12. ALL SHOP AND FIELD WELDS SHALL BE VISUALLY INSPECTED PER AWS D1.1. ALL DEFICIENT OR NON CONFORMING ITEMS SHALL BE REPORTED TO THE ENGINEER WHO WILL DETERMINE THE CORRECTIVE ACTION REQUIRED.
13. GROUT REQUIRED UNDER COLUMN BASE PLATES AS SHOWN IN THE DETAILS SHALL BE A STANDARD NON-SHRINK GROUT SUCH AS "SONOGROUT 10K" BY SONNEBORN INC.
14. PROVIDE AND HAVE IN PLACE ADEQUATE LATERAL BRACING AND VERTICAL SUPPORTS FOR THE SAFE ERECTION AND TRUE ALIGNMENT OF THE STRUCTURAL STEEL. THIS CONTRACTOR ASSUMES FULL RESPONSIBILITY FOR THE SAFE ERECTION AND TEMPORARY BRACING OF STRUCTURAL STEEL.
15. VERIFY NUMBER AND SIZE OF OPENINGS IN ROOF, WALLS AND FLOOR WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. SEE DETAILS, AND SPECIFICATIONS, FOR STRUCTURAL REQUIREMENTS. VERIFY ALL INFORMATION WITH THE APPROPRIATE CONTRACTOR.
16. PROVIDE L3x3x1/4 SHELF ANGLES AT TOPS OF COLUMNS AS REQUIRED TO SUPPORT ROOF DECK.
17. ALL FREE EDGES OF METAL DECK SHALL BE SUPPORTED WITH AN EDGE ANGLE L4x4x1/4 OR OTHER SUITABLE SUPPORT. THIS SHALL BE PROVIDED WHETHER SHOWN ON DRAWINGS OR NOT.
18. ALL BOLTS FOR BOLTED ATTACHMENTS TO HOLLOW STRUCTURAL SHAPES SHALL BE HOLLO-BOLTS BY LINDAPTER. BOLT DETAILING AND INSTALLATION SHALL BE PER THE MANUFACTURERS RECOMMENDATION.

STEEL DECK

1. ALL STEEL DECK SHALL BE DESIGNED, MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE LATEST REVISION OF THE STEEL DECK INSTITUTE (SDI) DESIGN MANUAL.
2. ROOF DECK:
   - A. ALL ROOF DECK SHALL BE 1 1/2" DEEP, 20 GA. WIDE RIB GALVANIZED DECK (1.5B BY VULCRAFT OR APPROVED EQUAL). ALL DECK SHALL SPAN A MINIMUM OF 3 SPANS.
   - B. FASTEN ROOF DECK TO ALL SUPPORTS AND AT PERIMETER WITH 5/8"ø PUDDLE WELDS. 6" O.C. AT SUPPORTS AND 6" O.C. AT PERIMETER. FASTEN SIDE LAPS WITH #10 TEK SCREWS, 2 PER SPAN.
3. PROVIDE ADDITIONAL SUPPORT FRAMING AT OPENINGS IN ROOF PER TYPICAL DETAILS.

COLD FORMED METAL

1. CONTRACTOR SHALL PROVIDE COMPLETE DESIGN, DRAWINGS AND CALCULATIONS FOR ALL COLD FORMED METAL FRAMING. ALL FRAMING MEMBERS, SPACING AND CONNECTIONS SHALL BE DESIGNED, DETAILED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MICHIGAN.
2. COMPLY WITH ALL LOADING REQUIREMENTS AS ESTABLISHED BY THE MICHIGAN BUILDING CODE FOR EXTERIOR FRAMING.
3. LIMIT MAXIMUM LATERAL DEFLECTION TO 1/720 OF SPAN WHERE EXTERIOR FINISH MATERIAL IS MASONRY AND 1/360 FOR SPAN OTHERWISE.
4. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONAL REQUIREMENTS, OPENING LOCATIONS, ETC., AND TO SPECIFICATION FOR ADDITIONAL REQUIREMENTS. COLD FORMED METAL FRAMING SHALL BE DETAILED TO ACCOMMODATE MOVEMENT OF THE STRUCTURE THROUGH THE USE OF VERTICAL SIDE CLIPS, SLIP CONNECTIONS, ETC.

DIVISION 6 - WOOD FRAMING

1. ALL CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN FOREST PRODUCTS ASSOCIATION (AFPA) "MANUAL FOR ENGINEERED WOOD CONSTRUCTION" AND FRAMING STANDARD WCD1 "DETAILS FOR CONVENTIONAL WOOD FRAME CONSTRUCTION.
2. WOOD FRAMING SIZES, VERTICAL FRAMING, HORIZONTAL FRAMING, FIRESTOPS, ANCHORAGE, FURRING AND CONNECTORS NOT SHOWN ON DOCUMENTS SHALL BE PER MBC CHAPTER 23 MINIMUM REQUIREMENTS.
3. ROOF SHEATHING SHALL BE 1/2" THICK, APA RATED SHEATHING, EXPOSURE 1, PANEL INDEX 32/16 AND STAMPED WITH DFPA GRADE-TRADEMARK.
   - A. ROOF SHEATHING PANELS SHALL BE NAILED TO SUPPORTS WITH 8d COMMON NAILS.
   - B. ROOF SHEATHING NAIL SPACING SHALL BE AS FOLLOWS:
     1. GABLE OVERHANGS AND ALL SHEATHING WITHIN 4 FEET OF THE GABLE WALL SHALL HAVE ALL PANEL EDGES NAILED AT 4" O.C. AND INTERMEDIATE SUPPORTS NAILED AT 6" O.C.
     2. STANDARD EAVE OVERHANG AND ALL SHEATHING WITHIN 4 FEET OF THE SUPPORTING WALL SHALL HAVE ALL PANEL EDGES NAILED AT 6" O.C. AND INTERMEDIATE SUPPORTS NAILED AT 6" O.C.
     3. ALL SHEATHING WITHIN 4 FEET OF THE RIDGE SHALL HAVE ALL PANEL EDGES NAILED AT 6" O.C. AND INTERMEDIATE SUPPORTS NAILED AT 6" O.C.
     4. THE REMAINDER OF THE ROOF SHALL HAVE ALL PANELS EDGES NAILED AT 6" O.C. AND INTERMEDIATE SUPPORTS NAILED AT 12" O.C.

DIVISION 6 - WOOD FRAMING - CONTINUED

4. ROOF DECKING NOTES
   1. ROOF DECKING SHALL BE 3X6 NOMINAL SOUTHERN PINE OR DG. FIR SELECT GRADE, KILN DRIED, TONGUE AND GROOVED, END-MATCHED, EDGE VEE 1 SIDE, SMOOTH TEXTURE, FACTORY STAINED WITH ONE COAT
   2. DECK LAY-UP SHALL BE CONTROLLED RANDOM EXCEPT AT LOCATIONS ON PLAN INDICATING TWO SPAN LAY-UP WITH END JOINTS OVER SUPPORTS.
   3. RANDOM LAY-UP DECKING SHALL BE CONTINUOUS OVER THREE OR MORE SPANS. EACH PIECE OF DECK MUST BEAR ON AT LEAST ONE SUPPORT.
   4. RANDOM LAY-UP DECKING SHALL BE LAID TO DISPERSE END-JOINTS SUCH THAT
      - THE DISTANCE BETWEEN END-JOINTS IN ADJACENT ROWS OF DECKING IS AT LEAST TWO FEET.
      - THE DISTANCE BETWEEN END-JOINTS IN ROWS OF DECKING SEPARATED BY ONLY ONE ROW IS AT LEAST ONE FOOT.
   5. DECKING AT CANTILEVERED OVERHANGS SHALL BE FREE OF END-JOINTS
   6. DECKING SHALL BE TOENAILED WITHIN 12" OF ALL ENDS
   7. NAILING SCHEDULE:
      - TOENAILING ALONG COURSES: 6D #30" O.C. FOR 2" NOM DECKING
      - FACE NAILING TO SUPPORTS: 16D FOR 2" NOM DECKING
5. ALL STRUCTURAL LUMBER SHALL BE OF THE FOLLOWING MINIMUM GRADES AND ALLOWABLE STRESSES OR EQUIVALENT AS PER NATIONAL FOREST PRODUCTS ASSOCIATION. MOISTURE CONTENT TO BE 19% MAX.

STUDS	HEM FIR STRUCTURAL GRADE NO. 2 OR BETTER
BEAM 2"-4" THICK	HEM FIR STRUCTURAL GRADE NO. 2 OR BETTER
POSTS	SAME AS STUDS
PLATE STOCK	SAME AS STUDS

6. STRUCTURAL GLULAM MEMBERS SHALL COMPLY TO THE FOLLOWING:
   - A. ALL GLULAM MEMBERS (BEAMS, PURLINS, & TRUSS MEMBERS) SHALL BE DOUGLAS FIR 24F-V5. LAMINATIONS ARE TO BE 1.5" THICK (2" NOMINAL). MEMBERS ARE TO BE ARCHITECTRUAL SMOOTH TEXTURE APPEARANCE GRADE (AITC).
   - B. ALL GLULAM MEMBER FINISHES SHALL COMPLY WITH THE FOLLOWING:
     1. FINISH (EXPOSED SURFACES): FACTORY STAIN - ONE COAT, ARCHITECT TO SELECT COLOR
     2. FINISH (UNEXPOSED SURFACES): ONE COAT SEALER
     3. PROTECTION (TRANSIT): WRAP INDIVIDUAL MEMBERS WITH WATER RESISTANT PAPER OR OPAQUE POLYETHYLENE, TAPE SEAMS
   - C. ALL GLULAM MATERIALS AND WORKMANSHIP SHALL BE IN CONFORMANCE WITH THE LATEST EDITION OF AITC 119, STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER OF HARDWOOD SPECIES (AITC - AMERICAN INSTITUTE OF TIMBER CONSTRUCTION). MEMBERS SHALL BE MARKED WITH A QUALITY MARK INDICATING CONFORMANCE TO THE LISTED STANDARDS.
   - D. ALL GLULAM TRUSS AND BEAM CONNECTIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MICHIGAN. THE CONTRACTOR IS TO SUBMIT SEALED CALCULATIONS AND SHOP DRAWINGS FOR THESE CONNECTIONS. ALL BOLTS SHALL BE ASTM A-301 AND SHALL BE A MINIMUM OF ½" DIAMETER.
7. ALL LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED TESTING AGENCY.
8. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO WEATHER SHALL BE TREATED LUMBER.
9. FRAMED OPENING: DOUBLE STUD FOR OPENINGS LESS THAN 4' WIDE, TRIPLE STUD FOR OPENINGS 4' WIDE OR MORE.
10. THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2X MEMBER BEAMS SHALL EXCEED THE NUMBER OF MEMBERS IN THE BEAM BY ONE. THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS (UNLESS NOTED OTHERWISE ON PLAN). ALL MICRO-LAM BEAMS SHALL HAVE 3 STUDS (MINIMUM). CONTINUE THESE STUDS TO THE FOUNDATION WITH INTERMEDIATE SUPPORTS THROUGH FLOOR, BETWEEN LOWER WALL TOP PLATE AND UPPER WALL BOTTOM PLATE.
11. ALL FLUSH BEAMS SHALL BE SUPPORTED BY APPROVED HANGER.
12. WHERE NOTED ON DETAILS, CONTRACTOR SHALL PROVIDE CONNECTORS FOR WOOD CONSTRUCTION AS MANUFACTURED BY SIMPSON STRONG TIE CONNECTORS. CONTRACTOR SHALL VERIFY TYPE INDICATED ON DRAWINGS. ANY SUBSTITUTION SHALL BE APPROVED BY THE ENGINEER. WHERE A TYPE IS NOT INDICATED OR TO BE PROVIDED BY THE TRUSS MANUFACTURER, THE CONTRACTOR SHALL SUBMIT PROPOSED CONNECTOR FOR APPROVAL.
13. ALL NAILS FOR NAILING OF STRUCTURAL LUMBER SHALL BE COMMON NAILS. ALL NAILING SHALL COMPLY WITH THE RECOMMENDED FASTENING SCHEDULE (MBC TABLE 2304.9.1) UNLESS NOTED OTHERWISE.
14. ROOF FRAMING LAYOUTS ARE PROVIDED TO ILLUSTRATE CONDITIONS OF CONSTRUCTION AND DO NOT NECESSARILY INDICATE SPECIFIC QUANTITIES OF MATERIALS OR COMPONENTS REQUIRED FOR CONSTRUCTION.
15. CONSTRUCTION BRACING SHALL BE PROVIDED BY THE CONTRACTOR TO MAINTAIN THE BUILDING PLUMB AND TRUE. THIS BRACING SHALL REMAIN UNTIL THE SPECIFIED SHEAR WALLS ARE TOTALLY INSTALLED.

DIVISION 6 - PREFABRICATED WOOD TRUSSES

1. ALL WOOD TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
   - TOP CHORD DEAD LOAD — 8 PSF
   - TOP CHORD LIVE LOAD — 30 PSF
   - BOTTOM CHORD DEAD LOAD — 13 PSF
   - DESIGN ALL ROOF TRUSSES FOR A NET UPLIFT OF 5.5 PSF.
2. THE EXTENT OF ROOF TRUSSES SHOWN ON THE PLANS IS FOR REFERENCE ONLY. THE FABRICATOR SHALL VERIFY ALL DIMENSIONS, TRUSS LAYOUT, CONFIGURATION, NUMBER OF EACH TYPE OF TRUSS REQUIRED, LOADING AND DETAILS.
3. WOOD TRUSSES SHALL BE DESIGNED, FABRICATED AND INSTALLED PER TRUSS PLATE INSTITUTE, INC. SPECIFICATIONS AND NFPA NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.
4. ALL TRUSSES SHALL BE ANCHORED TO SUPPORTS AS INDICATED AND IF NOT INDICATED, PER MANUFACTURERS RECOMMENDATIONS.

   + WEIGHT OF TRUSSES
   OR 25 PSF + DRIFTING PER CODE
   + WEIGHT OF TRUSSES
5. DEFLECTION OF TRUSSES SHALL BE LIMITED TO A MAXIMUM TOTAL LOAD DEFLECTION OF SPAN/360.
6. SUBMITTALS:
   - A. SHOP DRAWINGS SHOWING SIZES, DESIGN VALUES, LOADS, MATERIALS, TEMPORARY BRACING AND DIMENSIONAL RELATIONSHIPS OF COMPONENT AS WELL AS BEARING AND ANCHORAGE DETAILS.
   - B. TO EXTENT ENGINEERING DESIGN CONSIDERATIONS ARE FABRICATOR'S RESPONSIBILITY, SUBMIT DESIGN ANALYSIS AND TEST REPORTS INDICATING TRUSS PERFORMANCE CHARACTERISTICS.
   - C. PROVIDE SHOP DRAWINGS WHICH HAVE BEEN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MICHIGAN.
   - D. PROVIDE DESIGN CALCULATIONS WHICH HAVE BEEN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MICHIGAN.
   - E. ALL TRUSSES MUST BE DESIGNED FOR UPLIFT LOADS. UPLIFT VALUES AT EACH TRUSS BEARING POINT MUST BE SHOWN ON THE TRUSS DRAWINGS.
7. FOR WOOD ROOF TRUSSES: INSTALL (1) SIMPSON H2.5T HURRICANE TIE AND (1) SIMPSON LS30 ANGLE AT EACH BEARING LOCATION, IN ADDITION TO (3) 8d COMMON TOE NAILS.

DIVISION 6 - PREFABRICATED WOOD TRUSS BRACING

1. ALL TEMPORARY BRACING IS THE RESPONSIBILITY OF AND IS TO BE DESIGNED BY THE TRUSS MANUFACTURER.
2. AS A MINIMUM ALL TRUSSES SHALL BE BRACED DURING ERECTION PER "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91" BY THE TRUSS PLATE INSTITUTE. THIS BRACING SHALL REMAIN AS PERMANENT BRACING.
3. FABRICATOR SHALL CLEARLY INDICATE ON TRUSS DRAWINGS AND DIAGRAMS ALL REQUIRED PERMANENT BRACING REQUIRED TO BRACE ANY TRUSS CHORD OR WEB MEMBERS. SUCH BRACING MAY BE INCORPORATED WITH OTHER PERMANENT BRACING BY THE CONTRACTOR. ALL SUCH BRACING SHALL RUN TO AND TERMINATE AT BEARING WALLS. WHERE CONFIGURATION OF TRUSSES CHANGE, EFFECTING SUPPORT OF THIS BRACING, ADDITIONAL FRAMING FASTENED TO THE TRUSSES SHALL BE INSTALLED TO SUPPORT THE BRACING.
4. BOTTOM CHORD OF WOOD TRUSSES SHALL BE DESIGNED AS UNBRACED FOR A LENGTH EQUAL TO THE SPACING BETWEEN BOTTOM CHORD BRACES. BOTTOM CHORD BRACES SHALL BE SUPPLIED BY TRUSS MANUFACTURER.
5. ALL BRACING SHOWN OR DESCRIBED SHALL BE MINIMUM 2x4 WITH (2) 16d NAILS IN EVERY TRUSS IT CROSSES.
6. ALL TRUSS TOP CHORDS SHALL BE CONTINUOUSLY BRACED BY THE ROOF DECKING.
7. ALL TRUSS WEB MEMBERS SHALL BE BRACED AT 4'-0" O.C. UNLESS CALCULATIONS SHOW OTHERWISE.
8. ALL HORIZONTAL BRACING SHALL BE STIFFENED AT 20'-0" O.C. WITH EITHER:
   - A. DIAGONAL BRACING EXTENDED TO A SHEAR WALL PARALLEL TO THE ORIGINAL BRACING.
   - B. A 5/8" PLYWOOD SHEET EXTENDED TO ROOF DECK OR SHEAR WALL.
9. ALL TRUSS BOTTOM CHORDS SHALL BE BRACED AT 6'-0" O.C. UNLESS CALCULATIONS SHOW OTHERWISE. CONTINUOUS SHEATHING APPLIED TO BOTTOM CHORD WILL SATISFY THE BRACING REQUIREMENT.

DESIGN CRITERIA

1. DROP-OFF CANOPY ROOF DEAD LOADS
   - A. STANDING SEAM METAL ROOF — 2 PSF
   - B. 5/8" PLYWOOD — 2 PSF
   - C. METAL DECK — 2 PSF
   - D. STEEL FRAMING — 5 PSF
   - E. STEEL GIRDERS — 5 PSF
   - F. SUSPENDED CEILING — 3 PSF
   - G. ELECTRICAL — 3 PSF
   - TOTAL 22 PSF
2. CANTILEVERED CANOPY ROOF DEAD LOADS
   - A. STANDING SEAM METAL ROOF — 2 PSF
   - B. ELECTRICAL — 2 PSF
   - C. STEEL FRAMING — 6 PSF
   - D. STEEL GIRDERS — 4 PSF
   - TOTAL 14 PSF
3. PEDESTRIAN CANOPY ROOF DEAD LOADS
   - A. PRE-ENGINEERED CANOPY SELF WEIGHT
   - B. ELECTRICAL — 2 PSF
   - TOTAL 2 PSF + PRE-ENGINEERED CANOPY SELF WEIGHT
4. EMPLOYEE ENTRANCE ROOF DEAD LOADS
   - A. ROOFING MEMBRANE — 2 PSF
   - B. RIGID INSULATION — 1 PSF
   - C. METAL DECK — 2 PSF
   - D. STEEL FRAMING — 5 PSF
   - E. STEEL GIRDERS — 5 PSF
   - F. MECHANICAL & ELECTRICAL — 3 PSF
   - G. SUSPENDED CEILING — 2 PSF
   - TOTAL 20 PSF
5. PAVILLION ROOF DEAD LOADS
   - A. STANDING SEAM METAL ROOF — 2 PSF
   - B. INSULATED PANELS — 3 PSF
   - C. 1/2" PLYWOOD SHEATHING — 2 PSF
   - D. 3" WOOD DECKING — 8 PSF
   - E. CEILING — 2 PSF
   - F. MECHANICAL & ELECTRICAL — 5 PSF
   - G. GLULAM FRAMING — 5 PSF
   - TOTAL 27 PSF
6. SNOW LOADS
   - A. GROUND SNOW LOAD — Pg = 35 PSF
   - B. SNOW EXPOSURE FACTOR — Ce = 1.0
   - C. IMPORTANCE FACTOR — I = 1.2
   - D. THERMAL FACTOR — Ct = 1.0
   - E. DRIFTING AND/OR SLIDING HAS BEEN & SHALL BE INCLUDED IN DESIGN
7. WIND LOADS
   - A. BASIC WIND SPEED — 90 MPH (3 SEC. GUST)
   - B. EXPOSURE FACTOR — B
   - C. IMPORTANCE FACTOR — I = 1.15
   - D. COMPONENTS AND CLADDING PER MBC 2008
8. SEISMIC DESIGN DATA
   - A. OCCUPANCY CATEGORY — IV
   - B. SITE CLASS — D
   - C. RESPONSE COEFFICIENTS — SDS = 0.11, SDI = 0.064
   - D. SEISMIC DESIGN CATEGORY — A

QUALITY ASSURANCE

1. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO BE INFORMED, TO PROVIDE FOR, AND TO MAINTAIN SAFETY OF OPERATING EQUIPMENT AND HANDLING OF MATERIALS. THE CONTRACTORS SHALL COMPLY WITH ALL SAFETY STANDARDS OF OSHA, MIOSHA AND ANY OTHER STANDARD ENFORCEABLE BY LAW.
2. THE ARCHITECT AND ANY OF HIS CONSULTANTS ARE NOT RESPONSIBLE FOR METHODS, TECHNIQUES, SEQUENCE OF ERECTION, OR OTHER PROCEDURES OF THE CONSTRUCTION WORK.

WARNING:

3. THE ERECTION OF THIS STRUCTURE IS NOT INCORPORATED IN THE DESIGN OR IN ANY DETAILS.

Revisions:	Date
ADDENDUM #1	08-21-14

ARCHITECT/ENGINEERS:
WAK Engineers-Architects-Co
William A. Kibbe & Associates
1475 S. Washington Avenue, Saginaw, MI

Department of Veterans Affairs
Aleda E Lutz Medical Center
1500 Weiss Street
Saginaw, MI 48602

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File Name

File Location

Drawing Title: STRUCTURAL GENERAL NOTES

Approved: Project Director

Project Title: RECONSTRUCTION OF FRONT PARKING AREAS, SITE LIGHTING AND STORM DRAINAGE

Location: SAGINAW, MI

Date: 04/29/13 | Checked | Drawn

Project Number: 655-13-111

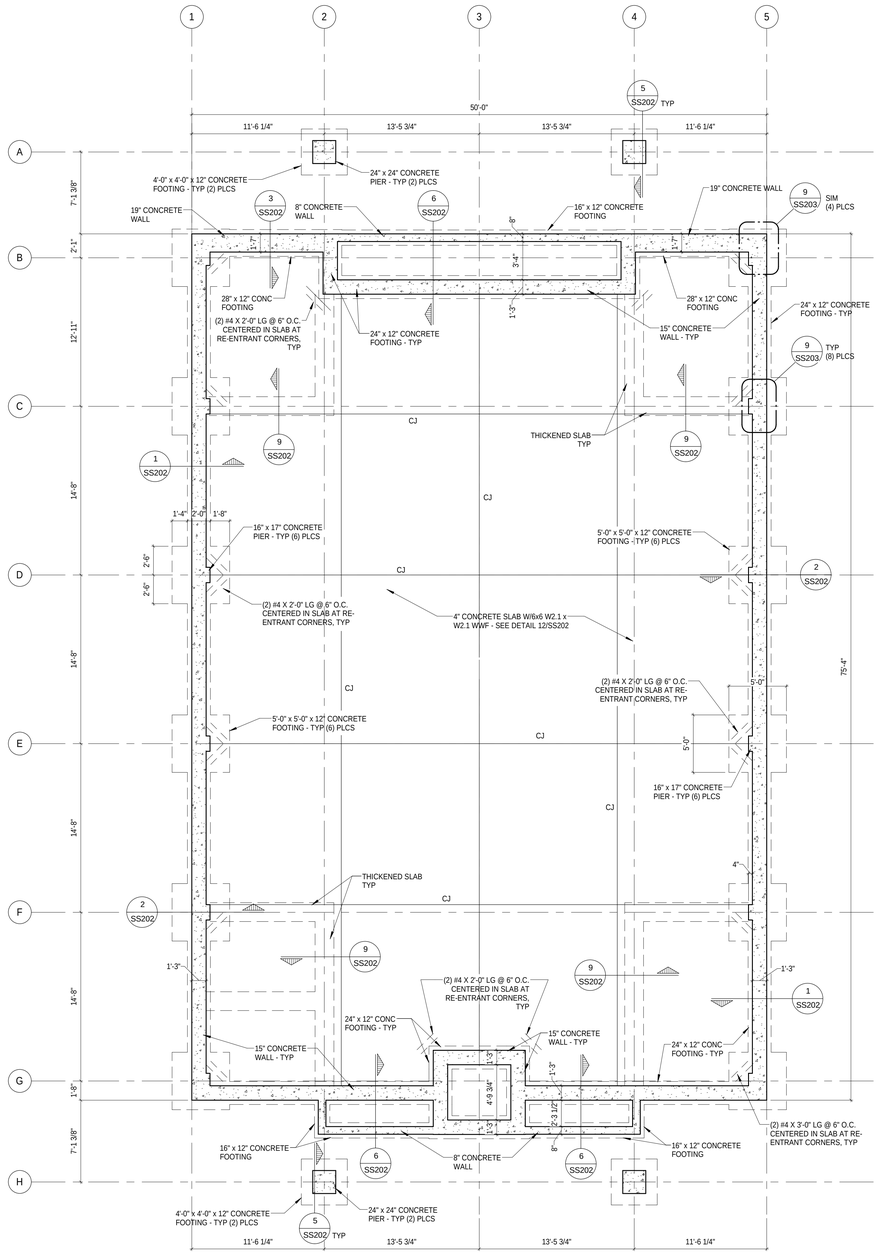
Building Number: SITE

Drawing Number: SS101

Dwg. of 14

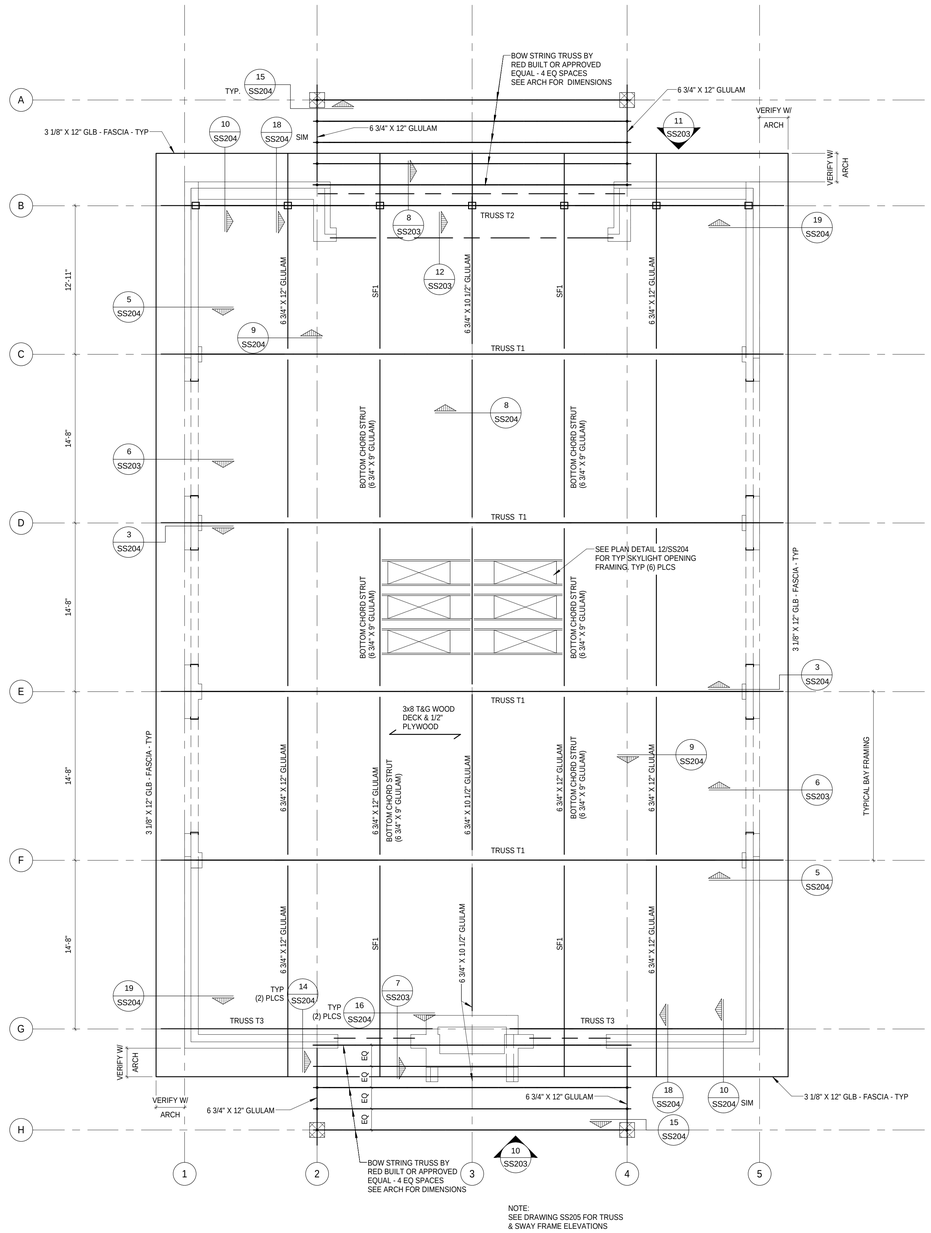
Office of Construction and Facilities Management

Department of Veterans Affairs



FOUNDATION PLAN

3/16" = 1'-0"



NOTE:
SEE DRAWING SS205 FOR TRUSS & SWAY FRAME ELEVATIONS

FRAMING PLAN

3/16" = 1'-0"

A.B.I. NUMBER 7

ADDENDUM #1	**08-21-14**
ISSUED FOR BID	
Revisions:	Date

ARCHITECT/ENGINEERS:

WAK Engineers-Architects-Co
William A. Kibbe & Associates
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Saginaw, MI 48602

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File Name
File Location

Drawing Title: FOUNDATION & FRAMING PLAN

Approved: Project Director

Project Title: PAVING OF PARKING LOTS, STORM DRAINAGE AND SITE LIGHTING

Location: SAGINAW, MI

Date: 02/21/14 | Checked | Drawn

Project Number: 655-10-106

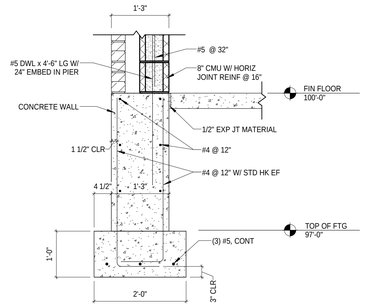
Building Number: SITE

Drawing Number: SS201
Dwg. or 62

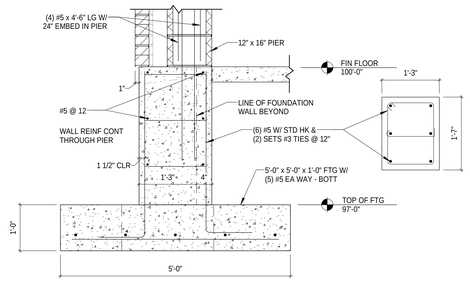
Office of Construction and Facilities Management

Department of Veterans Affairs

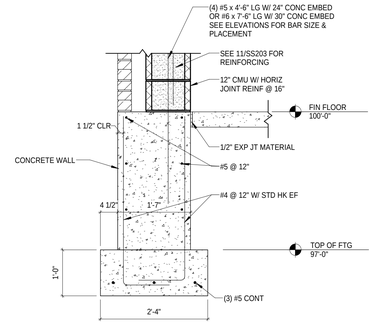
VA FORM 08-6231



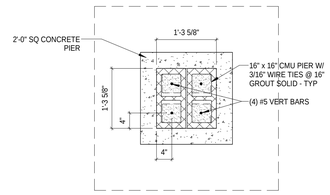
1 SS202 TYPICAL FOOTING DETAIL
3/4" = 1'-0"



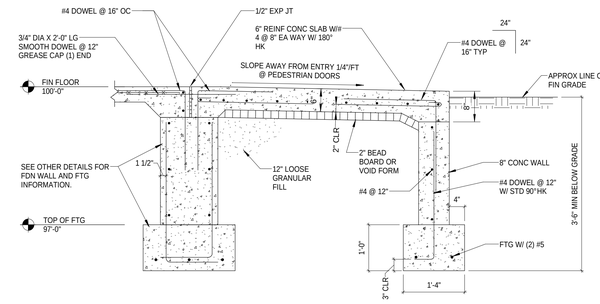
2 SS202 TYPICAL PIER DETAIL
3/4" = 1'-0"



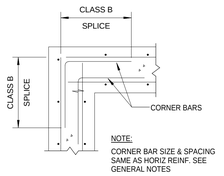
3 SS202 FOOTING DETAIL
3/4" = 1'-0"



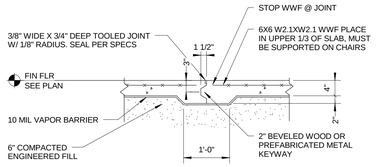
4 SS202 MASONRY PIER - PLAN
3/4" = 1'-0"



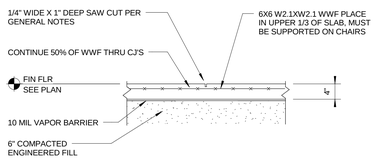
6 SS202 TYP FROST PROTECTED SLAB
3/4" = 1'-0"



7 SS202 TYP WALL & FOOTING CORNER DETAIL
3/4" = 1'-0"

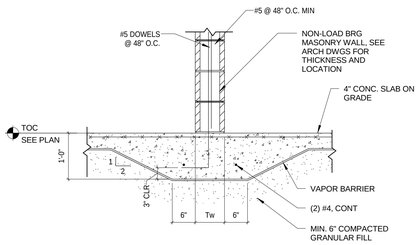


TYPICAL CONSTRUCTION JOINT (CSJ)

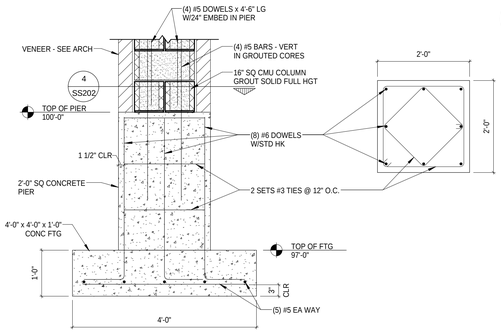


TYPICAL CONTROL JOINT (CJ)

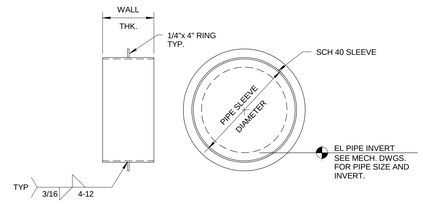
8 SS202 TYPICAL SLAB ON GRADE DETAILS
3/4" = 1'-0"



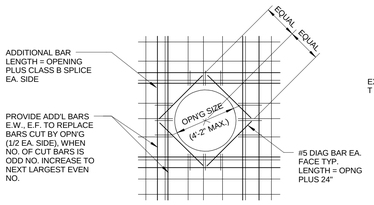
9 SS202 THICKENED SLAB DETAIL
3/4" = 1'-0"



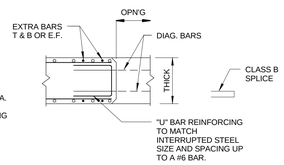
5 SS202 MASONRY PIER SECTION
3/4" = 1'-0"



10 SS202 WALL SLEEVE DETAIL
3/4" = 1'-0"

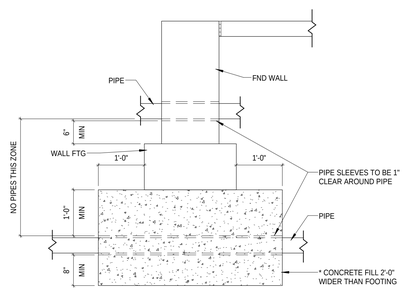


CIRCULAR OPENING
NO SCALE



OPENING EDGE DETAIL
NO SCALE

11 SS202 TYPICAL SLAB AND WALL OPENINGS
3/4" = 1'-0"



* PLACE CONCRETE FILL AROUND SLEEVE PRIOR TO CASTING FOUNDATION.
FILL IS TO BE A MIN OF 2'-0" WIDE AND A MIN OF 12" WIDER THAN SLEEVE.
SLOPE UP BOTTOM OF CONCRETE FILL AT A MIN OF 2" HORIZ TO 1" VERTICAL.

12 SS202 TYP FOUNDATION DETAIL AT UTILITY
3/4" = 1'-0"

A.B.I. NUMBER 7

ADDENDUM #1	08-21-14
ISSUED FOR BID	
Revisions:	Date

ARCHITECT/ENGINEERS:
WAK Engineers-Architects-Co
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1500 Weiss Street
Saginaw, MI 48602

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File Name
File Location

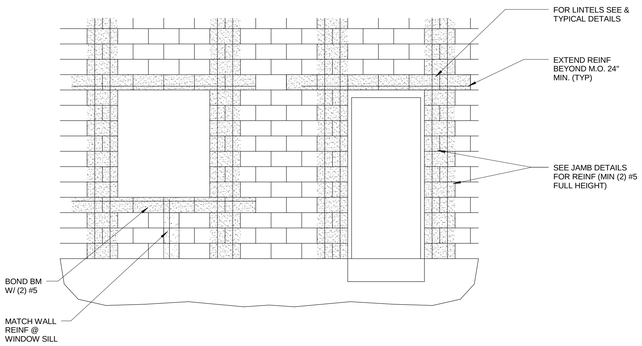
Drawing Title: FOUNDATION DETAILS
Approved: Project Director

Project Title: PAVING OF PARKING LOTS, STORM DRAINAGE AND SITE LIGHTING
Location: SAGINAW, MI
Date: 02/21/14
Checked
Drawn

Project Number: 655-10-106
Building Number: SITE
Drawing Number: SS202
Dwg. of 62

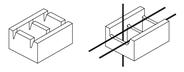
Office of Construction and Facilities Management
Department of Veterans Affairs

VA FORM 08-6231



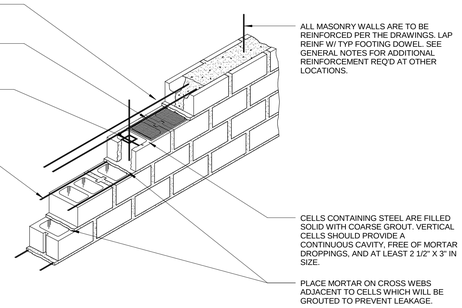
5 SS203 TYPICAL MASONRY OPENING DETAILS
3/4" = 1'-0"

NOTE:
BETWEEN GROUT POURS A HORIZ CONST JT SHALL BE FORMED BY STOPPING ALL WYTHES AT THE SAME ELEV WITH GROUT STOPPING A MINIMUM OF 1 1/2" BELOW A MORTAR JT EXCEPT AT TOP OF WALL.



SLOTTED **UNIT** WITH SECTIONS REMOVED

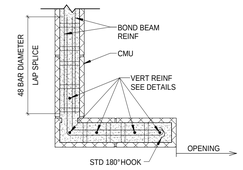
NOTE:
KNOCKOUT SLOTS MAY BE CAST IN UNIT WHEN MOLDED
OR
CUT OUT WITH A MASONRY SAW AFTER UNIT HAS BEEN CURED



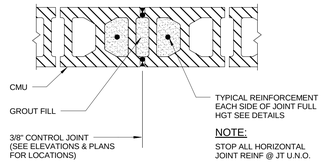
NOTE:
HIGH LIFT GROUTING SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 530.1 SPECIFICATIONS FOR MASONRY STRUCTURES. CLEANOUTS SHALL BE PROVIDED AND INSPECTED.

NOTE:
LOW LIFT-GROUTING TECHNIQUE-
GROUT IS PLACED IN LIFTS UP TO 5'-0"

4 SS203 MASONRY GROUTING TECHNIQUE
3/4" = 1'-0"



2 SS203 MASONRY CORNER DETAIL
3/4" = 1'-0"



NOTE:
STOP ALL HORIZONTAL JOINT REINF @ JT U.N.O.

3 SS203 MASONRY CONTROL JOINT (MCJ)
3/4" = 1'-0"

LAP SPLICE LENGTHS FOR MASONRY WALL (INCHES)

TABLE #1

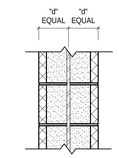
BARS CENTERED IN WALL (SINGLE REINFORCING)		
BAR SIZE	8" CMU	12" CMU
#3	18	18
#4	24	24
#5	30	30
#6	38	36
#7	-	42
#8	-	50
#9	-	64

DESIGN CRITERIA:
Fy = 60,000 PSI
F'm = 2,000 PSI

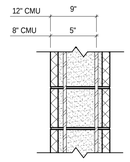
TABLE #2

BARS PLACED FOR MAX "d" (DOUBLE REINFORCING)		
BAR SIZE	8" CMU	12" CMU
#3	18	18
#4	24	24
#5	30	30
#6	57	57
#7	80	80
#8	-	-
#9	-	-

DESIGN CRITERIA:
Fy = 60,000 PSI
F'm = 2,000 PSI

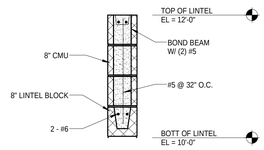


WALL SECTION - SINGLE REINFORCING
"d" = 1/2 x SPECIFIED UNIT THICKNESS

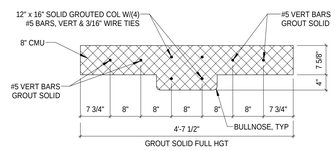


WALL SECTION - DOUBLE REINFORCING
"d" = NOMINAL UNIT THICKNESS MINUS 3.00 INCHES

1 SS203 VERT REINF LOCATION & LAP SPLICE LENGTHS
1" = 1'-0"

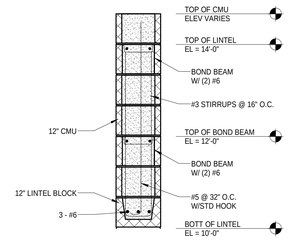


12 SS203 LINTEL DETAIL @ INT WALL
3/4" = 1'-0"

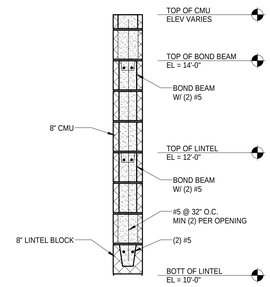


*PROVIDE DOWELS AT FDN WALL TO MATCH VERT REINF #5 x 4'-8" LG (24" EMBED IN FDN WALL)

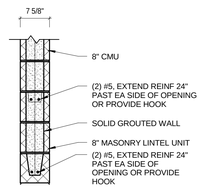
9 SS203 MASONRY PILASTER & JAMB DETAIL
3/4" = 1'-0"



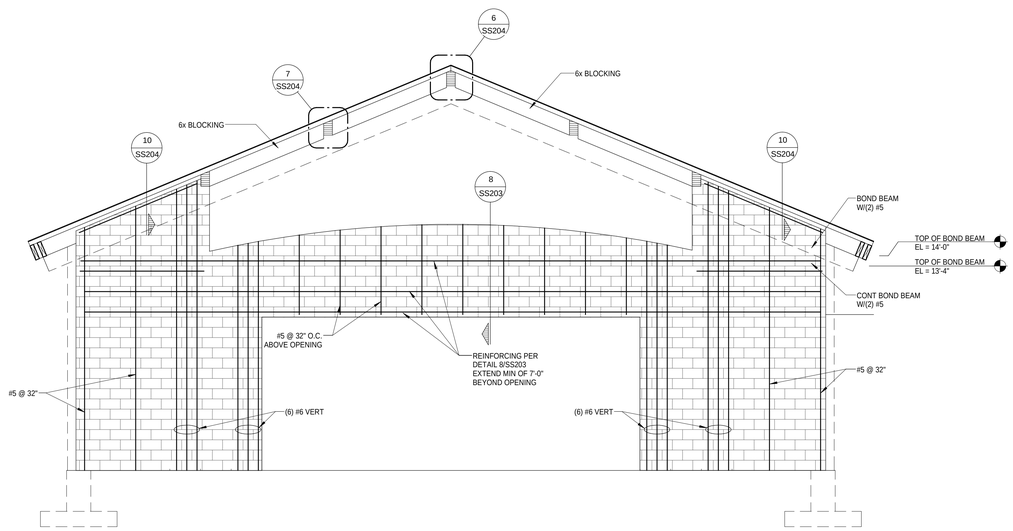
8 SS203 LINTEL DETAIL
3/4" = 1'-0"



7 SS203 LINTEL DETAIL
3/4" = 1'-0"

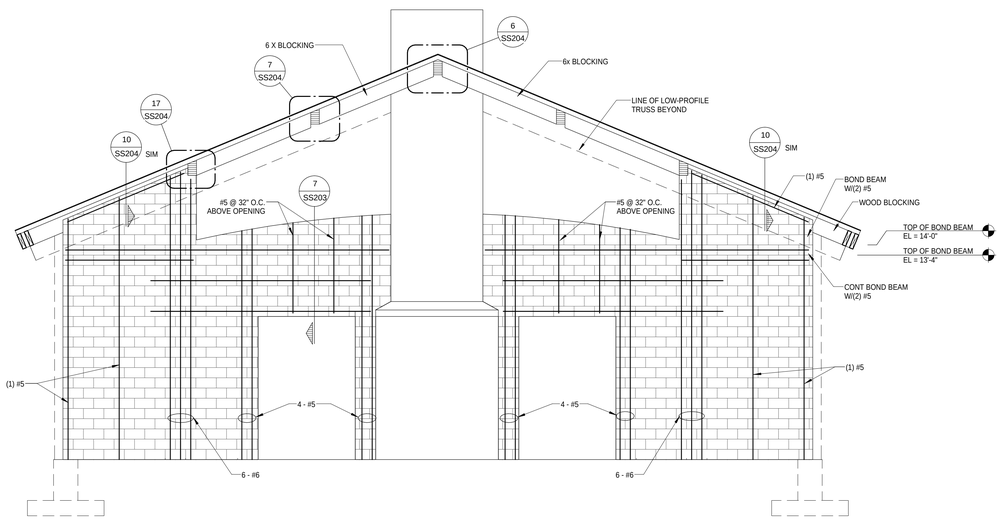


6 SS203 8" LINTEL DETAIL
3/4" = 1'-0"



11 SS203 NORTH ELEVATION
1/4" = 1'-0"

NOTE:
GROUT ENTIRE WALL SOLID - ALL LINTEL COURSES TO BE GROUTED SOLID IN A SINGLE LIFT. (NO COLD JOINTS)



10 SS203 SOUTH ELEVATION
1/4" = 1'-0"

NOTE:
GROUT ENTIRE WALL SOLID - ALL LINTEL COURSES TO BE GROUTED SOLID IN A SINGLE LIFT. (NO COLD JOINTS)

A.B.I. NUMBER 7

ADDENDUM #1	**08-21-14**
ISSUED FOR BID	
Revisions:	Date

ARCHITECT/ENGINEERS:
WAK Engineers-Architects-Co
William A. Kibbe & Associates
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1500 Weiss Street
Saginaw, MI 48602

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File Name
File Location

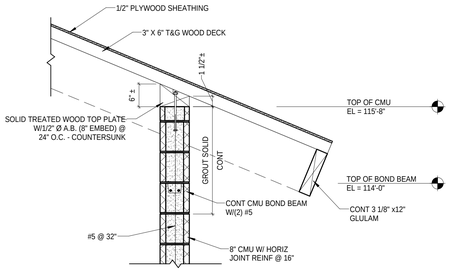
Drawing Title: TYPICAL MASONRY DETAILS
Approved: Project Director

Project Title: PAVING OF PARKING LOTS, STORM DRAINAGE AND SITE LIGHTING
Location: SAGINAW, MI
Date: 02/21/14
Checked
Drawn

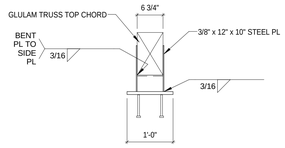
Project Number: 655-10-106
Building Number: SITE
Drawing Number: SS203
Dwg. or 62

Office of Construction and Facilities Management
Department of Veterans Affairs

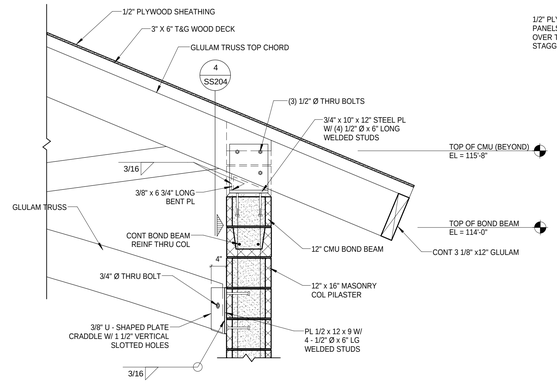
VA FORM 08-6231



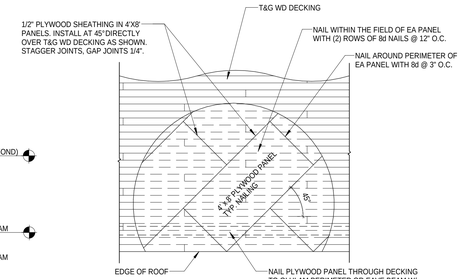
5 SS204 DECK BRG WALL SECTION
3/4" = 1'-0"



4 SS204 TRUSS BRG PL DETAIL
3/4" = 1'-0"



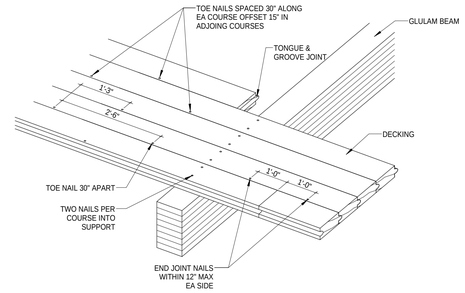
3 SS204 TRUSS BRG WALL SECTION
3/4" = 1'-0"



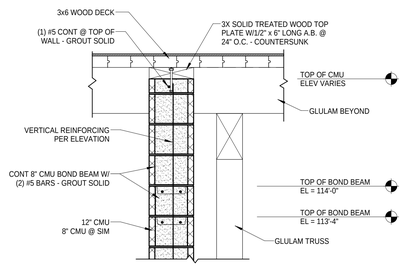
GENERAL NOTES REGARDING ROOF DIAPHRAGM

1. INSTALL DIAPHRAGM PANELS (APA RATED PLYWOOD) DIRECTLY OVER T&G WOOD DECK. ATTACH WITH 8d X 1 3/4" COMMON NAILS
2. PROTECT SHEATING WITH MIN 15# FELT IMMEDIATELY AFTER INSTALLATION
3. NAILS FOR ATTACHMENT OF RIGID INSULATION PANEL ARE TO HAVE SUFFICIENT LENGTH TO ANCHOR THRU DIAPHRAGM PANEL & MIN 1" INTO T&G WOOD DECKING

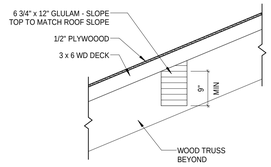
2 SS204 ROOF DIAPHRAGM AT GLULAM FRAMING
1/4" = 1'-0"



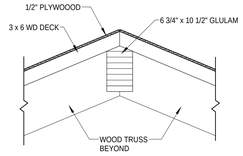
1 SS204 DECK NAILING DETAIL
3/4" = 1'-0"



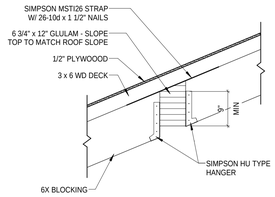
10 SS204 GABLE WALL DETAIL
3/4" = 1'-0"



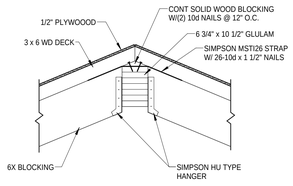
9 SS204 TYP GLULAM PURLIN
3/4" = 1'-0"



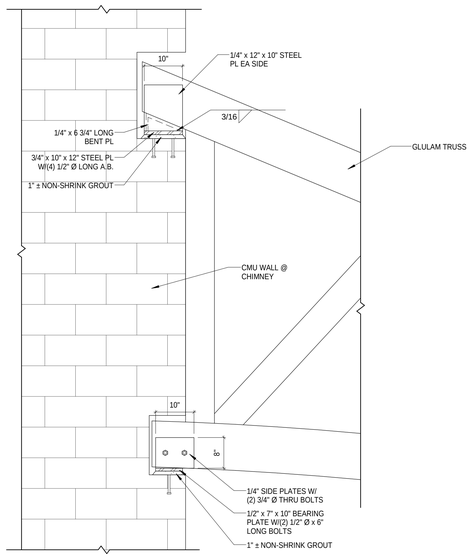
8 SS204 GLULAM PURLIN @ PEAK
3/4" = 1'-0"



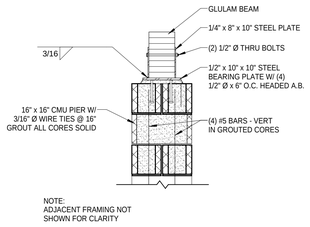
7 SS204 TYP GLULAM OUTLOOKER
3/4" = 1'-0"



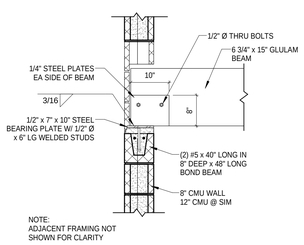
6 SS204 GLULAM OUTLOOKER @ PEAK
3/4" = 1'-0"



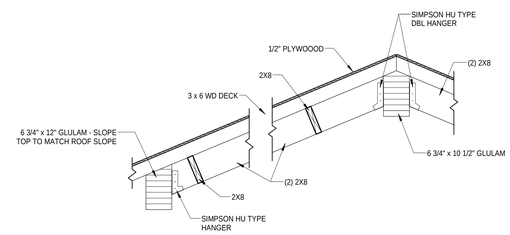
16 SS204 TRUSS BEARING @ CHIMNEY
3/4" = 1'-0"



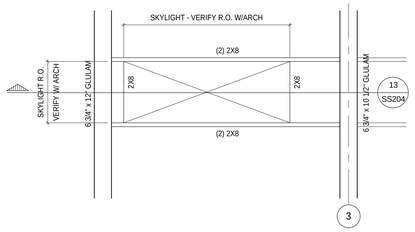
15 SS204 GLULAM BRG PLATE DETAIL
3/4" = 1'-0"



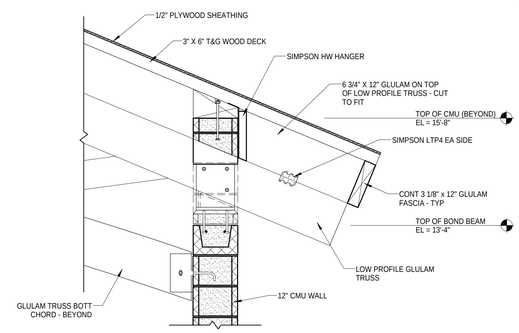
14 SS204 GLULAM BEAM BRG DETAIL
3/4" = 1'-0"



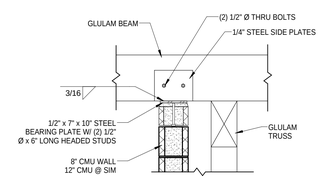
13 SS204 SKYLIGHT SECTION
3/4" = 1'-0"



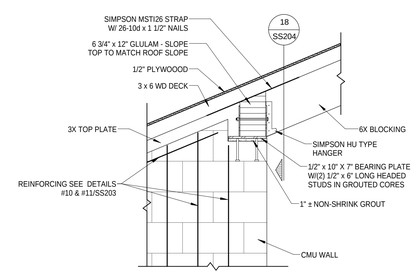
12 SS204 ENLARGED SKYLIGHT PLAN
1/2" = 1'-0"



19 SS204 FASCIA DETAIL @ LOW PROFILE TRUSS
3/4" = 1'-0"



18 SS204 GLULAM BRG @ CMU WALL
3/4" = 1'-0"



17 SS204 GLULAM BRG @ CMU WALL
3/4" = 1'-0"

A.B.I. NUMBER 7

ADDENDUM #1	08-21-14
ISSUED FOR BID	
Revisions:	Date

ARCHITECT/ENGINEERS:

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1500 Weiss Street
Saginaw, MI 48602

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File Name
File Location

Drawing Title: FRAMING DETAILS

Approved: Project Director

Project Title: PAVING OF PARKING LOTS, STORM DRAINAGE AND SITE LIGHTING

Location: SAGINAW, MI

Date: 02/21/14 | Checked | Drawn

Project Number: 655-10-106

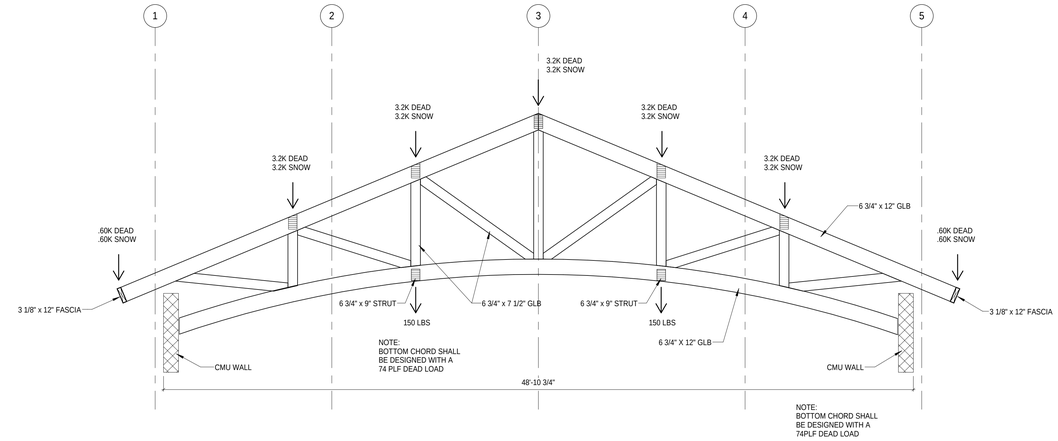
Building Number: SITE

Drawing Number: SS204 Dwg. of 62

Office of Construction and Facilities Management

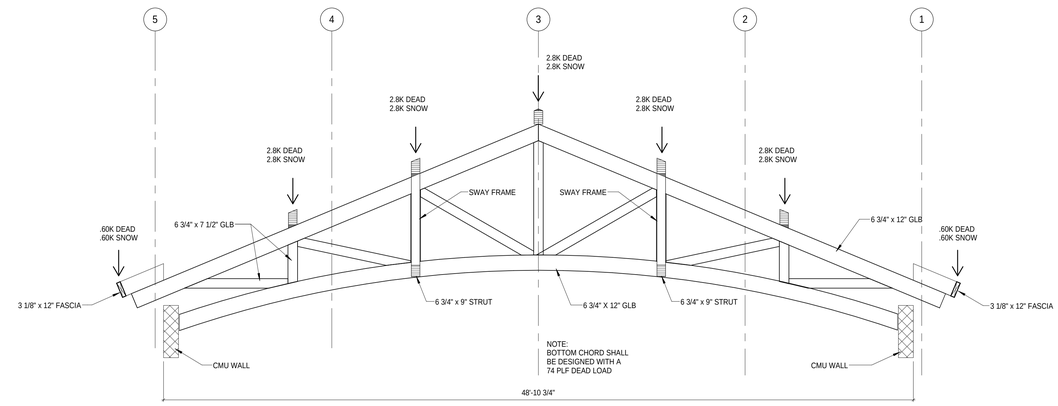
Department of Veterans Affairs

VA FORM 08-6231



TRUSS "T1"

1/4" = 1'-0"



TRUSS "T2"

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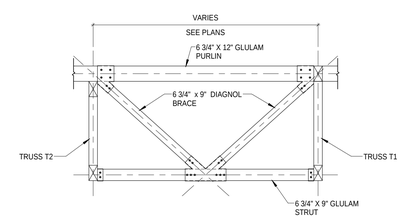
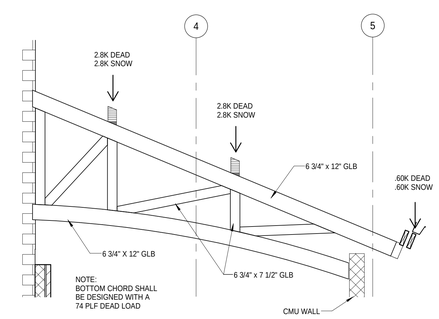


2 SS205 **SWAY FRAME 1**

1/4" = 1'-0"



TRUSS "T3"

1/4" = 1'-0"

A.B.I. NUMBER 7

ADDENDEM #1	**08-13-14**
ISSUED FOR BID	
Revisions:	Date

ARCHITECT/ENGINEERS:

WAK Engineers-Architects-Co

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File Name

File Location

Drawing Title: TRUSS ELEVATIONS

Approved: Project Director

Project Title: PAVING OF PARKING LOTS, STORM DRAINAGE AND SITE LIGHTING

Location: SAGINAW, MI

Date: 02/21/14

Checked

Drawn

Project Number: 655-10-106

Building Number: SITE

Drawing Number: SS205

Dwg. or 62

Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08-6231