

GENERAL NOTES

- 1. ALL ELEVATIONS ARE REFERENCED TO ABSOLUTE ELEVATION 0'-0". SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR ADDITIONAL INFORMATION.
2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH SPECIFICATIONS, ARCH. AND MEP DRAWINGS DURING ALL PHASES OF CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER OF RECORD (SER) FOR CLARIFICATION BEFORE COMMENCING WORK.
3. TYPICAL DETAILS APPLY REPETITIVELY ON THE PROJECT. CONTRACTOR SHALL COORDINATE THE GENERAL REQUIREMENTS OF TYPICAL DETAILS WITH PROJECT CONDITIONS, PLANS, SPECIFICATIONS, AND SECTIONS. IF CERTAIN DETAILS ARE NOT FULLY SHOWN ON DRAWINGS OR SPECIFICATIONS THEIR CONFIGURATION SHALL BE SIMILAR TO TYPICAL DETAILS, SUBJECT TO THE APPROVAL OF THE SER.
4. REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RESUBMITTAL AS SHOP DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED.
5. SHOP DRAWINGS SUBMITTED FOR STRUCTURAL REVIEW MAY BE SUBMITTED ELECTRONICALLY AND IN .PDF FORMAT. ONLY ONE MARKED UP SET OF PRINTS WITH THE SER'S COMMENTS WILL BE RETURNED TO THE ARCHITECT.
6. SUBMIT SHOP DRAWINGS AT LEAST 15 BUSINESS DAYS PRIOR TO THE DATE WHICH REVIEWED SUBMITTALS WILL BE REQUIRED. SHOP DRAWINGS SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL WHICH SHALL CONSTITUTE CERTIFICATION THAT HE HAS VERIFIED ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, MATERIALS AND SIMILAR DATA, AND HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS.
7. THESE DRAWINGS REPRESENT THE COMPLETED PROJECT, WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS AND FOR THE SUPERIMPOSED LOADS INDICATED IN THE DESIGN LOADS SECTION OF THE GENERAL NOTES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS, AND TO PROVIDE THE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGINGS, BRACING SHEETING AND SHORING, ETC. THE CONTRACTOR MUST COMPLY WITH ALL DEPARTMENT OF BUILDINGS OR STATE BUILDING CODE RULES REGARDING STRUCTURAL STABILITY INTEGRITY DURING CONSTRUCTION OPERATIONS.
8. IMPLEMENTING JOB SITE SAFETY AND CONSTRUCTION PROCEDURES (MEANS AND METHODS), TEMPORARY SHORING, AND BRACING OF EXISTING CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT ANY DAMAGE TO ADJACENT STRUCTURES AND UTILITIES.
9. ALL COSTS OF INVESTIGATION AND/OR REDESIGN, DUE TO CONTRACTOR'S MISLOCATION OF STRUCTURAL ELEMENTS OR OTHER LACK OF CONFORMANCE WITH THE PROJECT DOCUMENTS, SHALL BE AT THE CONTRACTOR'S EXPENSE.
10. CONTRACTOR SHALL REFER TO ARCHITECTURAL, CIVIL, LANDSCAPE, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS, EMBEDS, SLAB DEPRESSIONS, CHASES, CURBS, ANCHOR BOLTS, EXPANSION JOINT DETAILS, ANGLE FRAMES, AND OTHER REQUIREMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
11. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR DETAILED INFORMATION REGARDING FINISHES, PAINT, FIREPROOFING, FLOOR PITCHING, DRAIN LOCATIONS, WATERPROOFING AND DAMPPROOFING DETAILS.
12. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY AND ALL OTHER NON-LOAD BEARING PARTITIONS. PROVIDE SLIP CONNECTIONS THAT ALLOW VERTICAL MOVEMENT AT THE HEADS OF ALL SUCH PARTITIONS. CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE TOP OF WALLS LATERALLY FOR THE CODE-REQUIRED LATERAL LOAD. PROVIDE COMPRESSIBLE FIRESAFING AT THE TOP OF WALLS AS REQUIRED BY THE ARCHITECTURAL DRAWINGS.
13. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES, DETAILS, AND SPECIFICATIONS, THE MOST STRINGENT REQUIREMENTS SHALL APPLY.
14. CONTRACTOR SHALL FURNISH DIMENSIONED COORDINATED SHOP DRAWINGS AT ALL LEVELS LOCATING SLAB EDGES AND ALL SLEEVES AND OPENINGS REQUIRED BY ALL TRADES FOR REVIEW BY THE ARCHITECT AND SER.
15. CONTRACTOR SHALL PROVIDE ANY ALTERATIONS AND/OR ADDITIONAL COMPONENTS NEEDED TO ACCOMMODATE THE INSTALLATION OF EQUIPMENT OF ANY NATURE. COORDINATE SUCH WORK WITH THE EQUIPMENT SUPPLIER. INCORPORATE SUCH REFINEMENTS ON THE SHOP DRAWINGS AND OBTAIN THE EQUIPMENT SUPPLIER'S APPROVAL (CLEARLY DISPLAYED ON SHOP DRAWINGS) PRIOR TO SUBMITTING THE SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL ANCHOR BOLTS, NUTS, WASHERS, GROUT, CONCRETE PADS AND REINFORCING STEEL REQUIRED FOR THE PROPER INSTALLATION OF ALL EQUIPMENT.
16. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS IN THE FIELD PRIOR TO COMMENCING WORK, AND SHALL REPORT ANY DISCREPANCIES BETWEEN CONTRACT DOCUMENTS AND FIELD CONDITIONS TO THE SER.
17. NO CONSTRUCTION SHALL COMMENCE PRIOR TO THE APPROVAL OF SHOP DRAWINGS BY THE ARCHITECT. SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS.
18. NO CHANGES IN SIZE OR DIMENSIONS OF STRUCTURAL ELEMENTS, NOR ANY OPENINGS OR SLEEVES THROUGH STRUCTURAL ELEMENTS EXCEPT AS DETAILED ON THE STRUCTURAL CONTRACT DRAWINGS OR ON APPROVED SHOP DRAWINGS SHALL BE PERMITTED.
19. SCALES ON THE DRAWINGS ARE FOR INFORMATION ONLY. NO DIMENSIONAL INFORMATION SHALL BE OBTAINED BY SCALING FROM THE DRAWINGS.

REFERENCED STANDARDS

THE FOLLOWING CODES AND STANDARDS SHALL APPLY TO THE DESIGN, CONSTRUCTION, AND QUALITY CONTROL OF ALL WORK PERFORMED ON THE PROJECT.

- 1. INTERNATIONAL BUILDING CODE 2009.
2. AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE): MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE/SEI 7-05)
3. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC): SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (ANSI/AISC 360-05) CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (AISC 303-05)
4. AMERICAN CONCRETE INSTITUTE (ACI): BUILDING CODE REQ'S FOR STRUCTURAL CONCRETE & COMMENTARY (ACI 318/R-08) BUILDING CODE REQ'S FOR MASONRY STRUCTURES & COMMENTARY (ACI 530/R-05)
5. RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC): 2004 RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS
6. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM): 2009 ASTM STANDARDS IN BUILDING CODES
7. AMERICAN WELDING SOCIETY (AWS): 2008 AWS D1.1 STRUCTURAL WELDING CODE - STEEL
8. UNITED STATES GENERAL SERVICES ADMINISTRATION - OFFICE OF THE CHIEF ARCHITECT: 2005 FACILITIES STANDARDS FOR THE PUBLIC BUILDINGS SERVICE (PBS P-100)
9. UNITED STATES INTERAGENCY SECURITY COMMITTEE (ISC): 2004 ISC SECURITY DESIGN CRITERIA
10. UNITED STATES GENERAL SERVICES ADMINISTRATION (GSA): 2003 PROGRESSIVE COLLAPSE ANALYSIS AND DESIGN GUIDELINES

DESIGN LOADS

- 1. DEAD LOADS: ACTUAL WEIGHT OF MATERIALS AND CONSTRUCTION WEIGHT OF FIXED SERVICE EQUIPMENT WEIGHT OF CMU PARTITIONS INTERIOR PARTITIONS CONCRETE PADS, CURBS, BLENDS, FILLS (PER 1" THICKNESS) MECHANICAL, ELECTRICAL, PLUMBING & CEILING FLOORING: CARPET, HARDWOOD FLOOR, ETC. RAISED ACCESS FLOORING 1" GRANITE, MARBLE, TERRAZZO + 2" SETTING BED PSF EXTERIOR WALL CURTAIN WALL PRECAST PANELS ROOFING AND INSULATION ROOFING BALLAST OR PAVERS
2. LIVE LOADS: OFFICES STAIRS PSF AREAS OF PUBLIC ASSEMBLY AND LOBBIES MECHANICAL AREAS AND STORAGE AREAS GYM SIDEWALKS (AREAS OF VEHICULAR ACCESS) LOADING DOCK HEAVY STORAGE LIGHT STORAGE
3. ROOF LOADS: MINIMUM ROOF LIVE LOAD MINIMUM ROOF LIVE LOAD (LANDSCAPED ROOFS) RAIN LOAD (CONSIDERING 4" ACCUMULATED WATER)
4. SNOW LOADS (DRIFTING IN ACCORDANCE WITH IBC WHERE APPLICABLE):
5. WIND LOADS: BASIC WIND SPEED (3-SECOND GUST) "V" BUILDING CATEGORY WIND LOAD IMPORTANCE FACTOR "Iw" WIND EXPOSURE CATEGORY WIND DIRECTIONALITY FACTOR "Kd" TOPOGRAPHIC FACTOR "Kzt" GUST EFFECT FACTOR "G" EXTERNAL PRESSURE COEFFICIENTS "Cp" WINDWARD LEeward INTERNAL PRESSURE COEFFICIENT "GCpif"
6. SEISMIC LOADS: OCCUPANCY CATEGORY SITE CLASS (UNKNOWN/ASSUMED PER IBC 1613.5.2) MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS "Ss" MAPPED SPECTRAL RESPONSE ACCELERATION AT 1-SEC PERIOD "S1" SEISMIC LOAD IMPORTANCE FACTOR "Ie" SEISMIC DESIGN CATEGORY DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS "Sds" DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-SEC PERIODS "Sd1" SEISMIC FORCE RESISTING SYSTEM: OFFICE BUILDING- STEEL ORDINARY CANTILEVER COLUMN SYSTEMS DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE (OFFICE AND GARAGE) RESPONSE MODIFICATION FACTOR "R" (OFFICE) SEISMIC RESPONSE COEFFICIENT "Cs" (OFFICE) DESIGN BASE SHEAR "V" (OFFICE) MAXIMUM AMPLIFIED STORY DRIFT (OFFICE)
7. SPECIAL LOADS: CONSTRUCTION LIVE LOAD SOIL LATERAL LOADS: SEE GEOTECHNICAL REPORT HANDRAILS AND GUARDRAILS, UNIFORM LOAD IN ANY DIRECTION HANDRAILS AND GUARDRAILS, CONCENTRATED LOAD IN ANY DIRECTION UNIF LOAD AND CONC LOAD NOT BE APPLIED SIMULTANEOUSLY

FOUNDATION NOTES

- 1. EXAMINE THE SITE RECORDS OF EXISTING UTILITIES AND CONSTRUCTION, RECORD OF TEST BORINGS, THE SUBSURFACE EXPLORATION REPORT, AND THE SOIL SAMPLES TO DETERMINE THE CONDITIONS UNDER WHICH THE WORK WILL BE PERFORMED.
2. THE FOLLOWING SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING ANALYSIS INCLUDING RECORDS OF TEST BORINGS IS FURNISHED FOR INFORMATION ONLY AND IS NOT GUARANTEED TO REPRESENT ALL CONDITIONS THAT WILL BE ENCOUNTERED. THE RECORDS OF EXISTING UTILITIES AND EXISTING CONSTRUCTION (INCLUDING UNDERGROUND CONSTRUCTION) REPRESENT ALL CONDITIONS KNOWN.
3. FOUNDATIONS FOR THE OFFICE BUILDING HAVE BEEN DESIGNED FOR AN ASSUMED NET ALLOWABLE BEARING PRESSURE OF 2000 PSF.
4. SEE SPECIFICATIONS AND REFERENCED GEOTECHNICAL REPORT FOR REQUIREMENTS FOR EXCAVATION AND PREPARATION OF THE FOUNDATION AND SLAB-ON-GRADE SUBGRADES, INCLUDING COMPACTION PROCEDURES. REQUIREMENTS CONTAINED IN THE GEOTECHNICAL REPORT ARE PART OF THIS WORK.
5. PROTECT ADJACENT STRUCTURES, UTILITIES, SIDEWALKS, PAVEMENTS, AND OTHER FACILITIES FROM DAMAGE CAUSED BY SETTLEMENT, LATERAL MOVEMENT, UNDERMINING, WASHOUT, AND OTHER HAZARDS CREATED BY EARTHWORK OPERATIONS OR CONSTRUCTION PROCEDURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, INSTALLATION, MONITORING, AND FINAL REMOVAL OF ANY REQUIRED NEEDLING, UNDERPINNING, SHORING, OR BRACING OF EXISTING OR NEW CONSTRUCTION.
6. OTHER CONSTRUCTION, OF WHICH NO RECORDS ARE AVAILABLE, MAY BE ENCOUNTERED. THE CONTRACTOR SHALL FORMULATE HIS OWN CONCLUSIONS AS TO THE EXTENT OF SUCH CONSTRUCTION AND SHALL REMOVE ALL MATERIAL OF ANY NATURE TO THE DESIGN SUBGRADES INDICATED. EXISTING UTILITIES KNOWN TO BE IN THE CONSTRUCTION AREA HAVE BEEN INDICATED. THE SIZE, LOCATION, AND DEPTH OF THE UTILITIES ARE NOT KNOWN EXACTLY AND MAY VARY SIGNIFICANTLY FROM THAT INDICATED. OTHER UNKNOWN UTILITIES MAY BE ENCOUNTERED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES, WHETHER INDICATED OR NOT, WHICH MAY AFFECT THE CONSTRUCTION PROCESS.
7. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL.
8. PROVIDE CONTINUOUS BENTONITE STRIP WATERSTOPS AT ALL VERTICAL AND HORIZONTAL CONSTRUCTION JOINTS IN ALL BELOW GRADE CONCRETE INCLUDING ELEVATOR PITS AND PIT WALLS.
9. ALL SHORING, SHEETING, AND DEWATERING SHALL BE THE TOTAL RESPONSIBILITY OF THE CONTRACTOR. SHEETING AND SHORING SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECTS JURISDICTION. ALL SUBMITTALS SHALL BEAR THE ENGINEER'S SEAL AND SIGNATURE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
10. ANY BACKFILL SHOULD BE PLACED AND COMPACTED IN EQUAL LAYERS TO EQUAL DEPTHS ON BOTH SIDES OF STRUCTURAL ELEMENTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE GEOTECHNICAL REPORT.
11. ALL ORGANIC AND/OR UNSUITABLE MATERIALS SHALL BE REMOVED FROM FOOTING AND SLAB SUBGRADES AND BACKFILLED WITH ACCEPTABLE GRANULAR AND/OR COMPACTED FILL IN CONFORMANCE WITH THE SPECIFICATIONS.
12. DO NOT PLACE MUD SLABS, FRAMED SLABS, SLABS-ON-GRADE, FOUNDATIONS, PIER CAPS, GRADE BEAMS, OR WALLS INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, OR ICE. SHOULD WATER OR FROST ENTER EXCAVATION AFTER SUBGRADE APPROVAL, THE SUBGRADE SHALL BE RE-INSPECTED BY THE OWNER'S INDEPENDENT TESTING AND INSPECTION AGENCY AFTER REMOVAL OF WATER OR FROST.
13. THE OWNER'S INDEPENDENT TESTING AND INSPECTION AGENCY SHALL CONTINUOUSLY INSPECT AND TEST THE SUBGRADE, FOUNDATION CONCRETE AND REINFORCEMENT WORK, AND SHALL REVIEW ALL FOUNDATIONS, INSTALLATION PROCEDURES AND SEQUENCES, AS SUBMITTED BY THE CONTRACTOR, AND SHALL CONTINUOUSLY, VISUALLY OBSERVE ALL WORK.
14. SEE CIVIL DRAWINGS FOR GRANULAR FILL MATERIALS.
15. SEE LANDSCAPE, MEP, AND ARCH. DRAWINGS FOR ALL PIPING AND/OR DRAINAGE SYSTEMS.
16. SEE ARCHITECTURAL DRAWINGS FOR ALL DAMPPROOFING AND WATERPROOFING DETAILS.
17. SEE SPECIFICATION SECTION 02466 FOR ADDITIONAL FOUNDATION REQUIREMENTS INCLUDING TESTING AND SPECIAL INSPECTIONS.

STRUCTURAL STEEL NOTES

- 1. ALL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO AISC SPECIFICATIONS AND CODES SHOWN IN THE REFERENCED STANDARDS SECTION OF THE GENERAL NOTES. CONNECTIONS SHALL BE BOLTED OR WELDED UNLESS NOTED OTHERWISE. ALL SHEAR, MOMENT, AND BRACING CONNECTIONS SHALL WITHSTAND LRFD (FACTORED) REACTIONS, LOADS, AND MOMENTS INDICATED ON THE CONTRACT DOCUMENTS AND WITH ANY OTHER INFORMATION AND RESTRICTIONS INDICATED.
2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING: WIDE FLANGE SHAPES CHANNELS, ANGLES, AND PLATES HOLLOW STRUCTURAL SECTIONS (HSS) SQUARE AND ROUND STEEL PIPE HIGH STRENGTH BOLTS ANCHOR BOLTS HEADED SHEAR STUDS COLUMN BASE PLATES
3. ALL SHOP AND FIELD CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS OR WELDS. ALL HIGH STRENGTH BOLTS AND NUTS SHALL BE CLEARLY MARKED AS REQUIRED BY AISC SPECIFICATIONS. ALL HIGH STRENGTH BOLTS SHALL BE A MINIMUM OF 3/4" IN DIAMETER, UNO, UNLESS NOTED OTHERWISE. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A 'SNUG-TIGHT' CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A PERSON USING AN ORDINARY SPUD WRENCH. THE SNUG TIGHT CONDITION MUST ENSURE THAT THE PLIES OF THE CONNECTED MATERIAL HAVE BEEN BROUGHT INTO SNUG CONTACT.
4. ALL HIGH STRENGTH BOLTS SUBJECT TO DIRECT TENSION OR DESIGNATED AS SLIP CRITICAL SHALL BE PRE-TENSIONED IN ACCORDANCE WITH ONE OF THE FOLLOWING METHODS AS DESCRIBED IN THE AISC MANUAL OF STEEL CONSTRUCTION: TURN-OF-NUT TIGHTENING, CALIBRATED WRENCH TIGHTENING, OR DIRECT TENSION INDICATOR TIGHTENING.
5. HIGH-STRENGTH BOLTS, NUTS AND WASHERS WHICH ARE NOT MELTED AND MANUFACTURED IN THE UNITED STATES, SHALL BE REQUIRED TO BE SAMPLED, TESTED AND APPROVED BY THE CITY OF SAN DIEGO, PRIOR TO THE INSTALLATION, IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH IN BUILDING NEWSLETTER 17-4.
6. THE CONTRACTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW, CHECKED SHOP DRAWINGS SHOWING ALL FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DRAWINGS FOR STRUCTURAL STEEL.
7. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND SHALL BE IN ACCORDANCE WITH AWS D1.1 'STRUCTURAL WELDING CODE - STEEL' OF THE AMERICAN WELDING SOCIETY. ALL WELDS SHALL BE MADE WITH SERIES E70XX ELECTRODES EXCEPT FOR METAL DECK PUDDLE WELDS WHICH MAY BE E60XX. MINIMUM FILLET WELD SIZE SHALL COMPLY WITH ALL AISC REQUIREMENTS BUT IN NO CASE SHALL THE WELD BE LESS THAN 1/4".
8. THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE FRAME FOR WIND AND CONSTRUCTION LOADS.
9. THE MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE TWO (2).
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL AND ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIAL, ESPECIALLY WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO CONCRETE WALLS, BEAMS, OR COLUMNS.
11. ALL BEAMS SHALL BE INSTALLED WITH THEIR NATURAL CAMBER UP. PROVIDE CAMBER AS INDICATED ON CONTRACT DRAWINGS. CAMBER INDICATED ON PLAN DRAWINGS IS THE REQUIRED CAMBER AT THE TIME OF ERECTION.
12. AFTER FABRICATION ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE, AND OTHER FOREIGN MATERIALS.
13. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE SER.
14. PROVIDE A 1/4" CAP PLATE CONTINUOUSLY WELDED AT ENDS OF HOLLOW STRUCTURAL SECTIONS AND STEEL PIPE.
15. CONTRACTOR SHALL PROVIDE ACCESS FOR INSPECTION OF ALL SHOP AND FIELD CONNECTIONS FOR PROPER MATERIALS AND WORKMANSHIP.
16. CERTIFIED COPIES OF MILL TEST REPORTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER.
17. THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE.
18. ALL COMPOSITE CONCRETE SLABS SHALL ACHIEVE 28-DAY DESIGN STRENGTH PRIOR TO THE INSTALLATION OF ANY SUPERIMPOSED LOADS SUCH AS MASONRY WALLS, CURTAIN WALLS, OR STAIRS.
19. ALL ADDITIONAL STEEL REQUIRED BY THE CONTRACTOR FOR ERECTION PURPOSES AND SITE ACCESS OF STOCKPILED MATERIALS SHALL BE PROVIDED AT NO COST TO THE OWNER. ALL SUCH ADDITIONAL STEEL SHALL BE REMOVED BY THE CONTRACTOR UNLESS APPROVED BY THE OWNER.
20. SEE ARCHITECTURAL DRAWINGS FOR PAINT AND FIREPROOFING REQUIREMENTS.
21. REFER TO SPECIFICATION SECTION 051200 FOR ADDITIONAL STRUCTURAL STEEL REQUIREMENTS AND SEE STATEMENT OF SPECIAL INSPECTIONS ON S-003 & S-004 FOR TESTING AND SPECIAL INSPECTIONS.

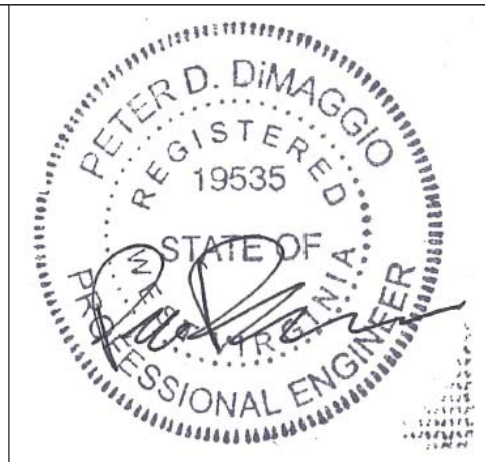
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FINAL CD ISSUE FULLY SPRINKLERED



REINFORCED MASONRY NOTES

- CONCRETE MASONRY WORK SHALL CONFORM TO THE BUILDING CODE PROVISIONS FOR MASONRY, AND THE SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY, NCMA.
- MORTAR SHALL BE TYPE M OR S MORTAR ONLY AND SHALL CONFORM TO THE SPECIFICATIONS.
- GROUT SHALL CONFORM TO SPECIFICATIONS WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI SHALL BE USED AS FILLING FOR VERTICAL CAVITIES, BOND BEAMS, LINTELS, AND HOLLOW MASONRY UNITS DESIGNATED AS SOLID GROUTED IN THE DESIGN DRAWINGS. GROUT SHALL HAVE A MINIMUM SLUMP OF 3 INCHES.
- CONCRETE MASONRY UNITS SHALL BE STANDARD HOLLOW LOAD BEARING CONCRETE MASONRY UNITS, WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 1,900 PSI ON NET SECTION DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF MASONRY OF 1500 PSI IN 28 DAYS (Fm) WHEN LAID IN TYPE S MORTAR.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF CONTROL JOINTS.
- USE ASTM A-615 GRADE 60 FOR ALL REINFORCING STEEL. PROVIDE LAP SPLICES OF NO LESS THAN 40 BAR DIAMETERS OR 24 INCHES FOR ALL REINFORCEMENT. REINFORCEMENT MUST BE CONTINUOUS AROUND ALL CORNERS AND AT INTERSECTIONS.
- VERTICAL BARS SHALL BE EMBEDDED AND EPOXYED 2 1/2" MIN INTO EXISTING CONC SLAB.
- HORIZONTAL REINFORCEMENT BARS SHALL BE PLACED IN THE CONTINUOUS MASONRY COURSES CONSISTING OF BOND BEAM OR TROUGH BLOCK UNITS, AND SHALL BE SOLIDLY GROUTED IN PLACE.
- ALL LOAD BEARING WALLS, EXTERIOR BUILDING WALLS, AND WALLS AROUND STAIRS AND ELEVATOR SHAFTS SHALL HAVE VERTICAL REINFORCING IN GROUTED CELLS AS FOLLOWS IF NOT OTHERWISE NOTED ON THE CONTRACT DRAWINGS:
 - FOR SPANS NOT GREATER THAN 14 FEET BETWEEN LATERAL SUPPORTS #5@32"o/c
 - FOR SPANS BETWEEN 14 FT AND 25 FT BETWEEN LATERAL SUPPORTS #5@16"o/c
 - VERTICAL REINFORCING SHALL BE ANCHORED INTO SUPPORTING SLAB OR BEAM BELOW WITH TENSION EMBEDMENT LENGTH.
 - ALL MASONRY IS TO BE GROUTED SOLID.
- ALL OTHER INTERIOR NON LOAD BEARING WALLS SHALL BE DETAILED AND REINFORCED AS PER TYPICAL DETAILS ON THE CMU DETAILED SHEET. VERTICAL REINFORCING SHALL BE PROVIDED CONFORMING TO THE SPECIFICATIONS AND THE CONTRACT DOCUMENTS. THE FIRST CELL AT CORNERS AND ENDS OF WALLS SHALL BE REINFORCED WITH 1-#5 AND GROUTED. 1-#5 (EXTENDING 2'-0" BEYOND CORNERS) SHALL BE PLACED ON ALL SIDES OF WALL OPENINGS.
- REINFORCE AND GROUT CMU WALLS IN LIFTS NOT TO EXCEED 4 FEET IN HEIGHT.
- SEE SPECIFICATIONS FOR TESTING AND INSPECTION REQUIREMENTS.
- SEE TYPICAL DETAILS FOR BOND BEAM INFORMATION AND LINTEL SCHEDULE.
- ALL MASONRY IS TO BE GROUTED SOLID.

STEEL DECK NOTES

- METAL DECK SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH SDI 'STEEL DECK INSTITUTE DESIGN MANUAL FOR FLOOR DECKS AND ROOF DECKS,' EXCEPT AS MODIFIED BY THE PROJECT SPECIFICATIONS.
- SUBMIT ENGINEERING CALCULATIONS, AND LOAD TEST DATA VERIFYING THE SPECIFIED DECK REQUIREMENTS TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW.
- PROVIDE ENGINEERED AND CHECKED SHOP DRAWINGS INDICATING LOCATION, GAUGE, AND SIZE OF EACH PIECE OF DECKING. THE DRAWINGS SHALL CLEARLY SHOW WELDING DETAILS TO STRUCTURAL FRAMING, AND SIDE LAP CONNECTION DETAILS AND REQUIRED SUPPLEMENTARY SUPPORT STEEL.
- METAL DECK SECTION PROPERTIES SHALL BE COMPUTED IN ACCORDANCE WITH AISI 'SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS.'
- ALL METAL DECKING SHALL BE GALVANIZED AND FABRICATED FROM STEEL TYPE ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 40000 PSI.
- PROVIDE STEEL DECK WITH THE FOLLOWING MINIMUM SECTION PROPERTIES. ALL DECK PROPERTIES ARE BASED ON PRODUCTS MANUFACTURED BY UNITED STEEL DECK, INC. DECKS BY OTHER MANUFACTURERS MAY BE SUPPLIED PROVIDED SECTION PROPERTIES ARE EQUIVALENT TO THOSE SHOWN AND SUBJECT TO THE APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER. THE MINIMUM THICKNESS OF METAL DECK SHALL BE 20 GAUGE.
 $I_p = 0.313 \text{ IN}^4$, $S_p = 0.378 \text{ IN}^3$, $S_x = 0.376 \text{ IN}^3$
- ALL ROOF DECK SHALL BE FORMED WITH TELESKOPEDED ENDS. LAP ENDS OF SHEET A MINIMUM OF TWO (2) INCHES.
- ALL ROOF DECK SHALL BE DESIGNED BY THE CONTRACTOR FOR THE CONDITIONS SHOWN ON THE DRAWINGS AND IN THE METAL DECK SCHEDULE.
- METAL DECK SHALL BE CONTINUOUS OVER THREE (3) SPANS WHENEVER POSSIBLE. SINGLE AND DOUBLE SPANS IF REQUIRED SHALL SATISFY LOAD AND DEFLECTION REQUIREMENTS. LAPS SHALL BE PLACED OVER SUPPORTS.
- ALL DECKING SHALL BE WELDED TO STRUCTURAL STEEL BY QUALIFIED WELDERS USING PREQUALIFIED PROCEDURES. THE ERECTOR SHALL ESTABLISH A PROCEDURE FOR THE PLUG WELDING OF METAL DECK TO STRUCTURAL STEEL FOR EACH GAUGE USED. PRIOR TO THE START OF ERECTION OF THE STEEL DECK, EACH WELDER SHALL BE QUALIFIED IN THESE PROCEDURES AS WITNESS BY THE OWNER'S INDEPENDENT TESTING AND INSPECTION AGENCY.
- ALL ROOF DECK SHALL BE WELDED TO THE SUPPORTING STEEL WITH 3/4" DIAMETER WELDS AT 8" ON CENTER MAXIMUM (MINIMUM 4 WELDS ACROSS A 24" WIDE SHEET). SIDE SEAMS SHALL BE FASTENED AT 12" ON CENTER MAXIMUM. ALL COMPOSITE FLOOR DECK SHALL BE WELDED TO THE SUPPORTING STEEL WITH 3/4" DIAMETER WELDS AT 12" ON CENTER MAXIMUM (MINIMUM 4 WELDS ACROSS 36" WIDE SHEET). SIDE SEAMS SHALL BE FASTENED AT 30" ON CENTER MAXIMUM.
- PROVIDE SUPPLEMENTAL FRAMING AT OPENINGS AS REQUIRED FOR SUPPORT OF THE METAL DECKING. ALL OPENINGS SHALL BE COORDINATED WITH ARCHITECTURAL AND MEP DRAWINGS.
- PROVIDE CONTINUOUS SHEET METAL CLOSURES AT ALL SLAB OPENINGS AND SLAB EDGES AND CONTINUOUS DECK CLOSURE AT ALL DECK ENDS.
- DO NOT HANG DUCTWORK, PIPING, ETC. FROM ROOF DECK. ALL HANGING LOAD DETAILS SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
- SEE SPECIFICATION SECTION 053100 FOR ADDITIONAL METAL DECK REQUIREMENTS INCLUDING TESTING AND SPECIAL INSPECTIONS.
- CONTRACTOR IS ADVISED THAT THE STRUCTURAL STEEL BEAMS SUPPORTING CONCRETE OVER METAL DECKING ARE EXPECTED TO DEFLECT PLUS OR MINUS 3/4" DURING CONSTRUCTION OPERATIONS AT INTERMEDIATE BEAMS AND A LESSER AMOUNT AT BEAMS ON GRID LINES. AS FLOOR SLABS ARE REQUIRED TO BE LEVEL WITHIN THE OFFICE TOWER AND HAVE SPECIFIC SLOPES ELSEWHERE. CONTRACTOR SHALL USE EITHER OR BOTH OF THE BELOW METHODS TO ASSURE PROPER ELEVATIONS OF CONCRETE.
 - PROVIDE ADDITIONAL LIGHTWEIGHT CONCRETE REQUIRED TO OBTAIN A LEVEL SURFACE OR;
 - SHORE STRUCTURAL BEAMS DURING CONSTRUCTION. SHORING SHALL REMAIN IN PLACE FOR SEVEN DAYS MINIMUM AFTER CONCRETE IS POURED. UNSHORED BEAMS SHALL NOT BE USED TO SUPPORT SHORING BEAMS ABOVE UNLESS THE CONCRETE HAS BEEN IN PLACE FOR FOURTEEN DAYS MINIMUM. SHORING DESIGN SHALL BE BY CONTRACTOR. THE TERM "SHORING" SHALL INCLUDE ALL TEMPORARY BRACING, CONNECTIONS, ETC.

CAST-IN-PLACE CONCRETE NOTES

- ALL CONCRETE, MATERIALS, DESIGN, AND WORK SHALL CONFORM WITH THE REQUIREMENTS OF THE ACI SPECIFICATIONS SHOWN IN THE REFERENCED STANDARDS SECTION OF THE GENERAL NOTES.
- ALL CAST-IN-PLACE CONCRETE SHALL BE NORMAL WEIGHT (150 PCF) EXCEPT COMPOSITE FILL SLAB CONCRETE WHICH SHALL BE LIGHTWEIGHT (115 PCF). ALL CONCRETE SHALL HAVE THE FOLLOWING MINIMUM 28-DAY CYLINDER COMPRESSIVE STRENGTHS:

PIERS, FOOTINGS, GRADE BEAMS	f _c = 4000 PSI
SLABS ON GRADE	f _c = 4000 PSI
COMPOSITE FILL SLABS	f _c = 3000 PSI
SITE STRUCTURES	f _c = 5000 PSI
CAST-IN-PLACE CONCRETE BEAMS AND WALLS	f _c = 5000 PSI
- ALL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO THE STANDARDS OF ASTM A615, GRADE 60. EPOXY COATED REINFORCING BARS SHALL CONFORM TO ASTM A775. ALL REINFORCING STEEL IN THE BOUNDARY ZONES OF SPECIAL CONCRETE SHEAR WALLS SHALL CONFORM TO ASTM A706.
- ALL WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE STANDARDS OF ASTM A185. ALL WELDED WIRE REINFORCEMENT SUBJECT TO VEHICULAR TRAFFIC SHALL BE GALVANIZED.
- MIX DESIGNS FOR ALL CONCRETE COMPONENTS SHALL BE SIGNED AND SEALED BY A CA LICENSED PE AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW PER THE SPECIFICATIONS. ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING, PLASTICIZING ADMIXTURE. ALL CONCRETE PERMANENTLY EXPOSED TO THE WEATHER SHALL CONTAIN AN APPROVED AIR-ENTRAINING ADMIXTURE. CONCRETE FOR ALL FRAMED SLABS, BEAMS, AND GIRDERS SUBJECT TO VEHICULAR TRAFFIC SHALL CONTAIN AN APPROVED CORROSION INHIBITING ADMIXTURE AND HAVE A MAXIMUM WATER TO CEMENT RATIO OF 0.50.
- ALL PORTLAND CEMENT SHALL CONFORM TO ASTM C150, SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE IBC, ACI-318, ACI-301, AND THE MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES, ACI 315-99.
- UNLESS NOTED OTHERWISE, THE CONCRETE COVER OF ALL REINFORCING BARS SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH FORMED CONCRETE SURFACES EXPOSED TO EARTH OR WEATHER	3"
#6 BARS AND LARGER	2"
#5 BARS AND SMALLER	1 1/2"
FORMED CONCRETE SURFACES NOT EXPOSED TO EARTH OR WEATHER	
SLABS AND WALLS	3/4"
BEAMS AND COLUMNS (CLEAR COVER TO TIES OR STIRRUPS)	1 1/2"
FRAMED CONCRETE SURFACES EXPOSED TO VEHICULAR TRAFFIC	
SLABS (TOP)	2"
SLABS (BOTTOM)	1"
BEAMS (TOP, CLEAR COVER TO TIES OR STIRRUPS)	2"
- THE CONTRACTOR SHALL SUBMIT DETAILED COORDINATED SHOP DRAWINGS AT EACH LEVEL SHOWING REINFORCING DETAILS (INCLUDING BAR SIZES, SPACING, AND PLACEMENT), THE LOCATIONS OF ALL CONSTRUCTION JOINTS, CURBS, DEPRESSIONS, OPENINGS AND SLEEVES REQUIRED BY ALL TRADES TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
- ALL REINFORCING STEEL SPLICES SHALL BE FULL CLASS 'B' TENSION SPLICES UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- ALL WELDED WIRE REINFORCEMENT SHALL BE LAPPED A MINIMUM OF TWO (2) FULL MESH PANELS AND TIED SECURELY UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
- BEAMS, GIRDERS, AND COLUMNS SHALL NOT BE SLEEVED OR OTHERWISE INTERRUPTED UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS.
- ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED AND ALL BALCONIES EXPOSED TO WEATHER SHALL HAVE EPOXY COATED REINFORCING STEEL.
- ALL WALLS AND FRAMED STRUCTURAL SLABS SHALL BE REINFORCED WITH A MINIMUM OF #4@12" O.C. EACH WAY, EACH FACE OR THE MINIMUM TEMPERATURE REINFORCEMENT REQUIRED BY ACI-318 WHICHEVER REQUIREMENT IS MORE STRINGENT, UNLESS NOTED OTHERWISE.
- PROVIDE ONE (1) LAYER OF 6x6-W2.9xW2.9 CONTINUOUS, IN ALL CONCRETE FILLS ABOVE FRAMED STRUCTURAL SLABS. CONCRETE FILLS AND TOPPING SHALL HAVE THEIR CONSTRUCTION AND EXPANSION JOINTS AT THE SAME LOCATION AS THE CONSTRUCTION AND EXPANSION JOINTS IN THE SUPPORTING CONCRETE.
- CONCRETE WALLS SHALL BE CAST IN ALTERNATE PANELS NOT EXCEEDING 60 FEET IN LENGTH. VERTICAL CONSTRUCTION JOINTS SHALL BE LOCATED AT LEAST 20 FEET FROM CORNERS OR INTERSECTIONS AND AT LEAST 4'-0" FROM ANY WALL OPENINGS, PENETRATIONS, OR OTHER INTERRUPTIONS.
- WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING.
- OPENINGS IN SLABS OR WALLS LESS THAN 12" SQUARE OR 12" IN DIAMETER ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- PROVIDE POCKETS IN WALLS TO RECEIVE SLABS OR BEAMS WHERE REQUIRED.
- ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR OPENINGS. SEE TYPICAL DETAILS.
- ALL FORMWORK, SHORING, AND RESHORING SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMISSIONS SHALL BEAR HIS STAMP AND SIGNATURE.
- CORE DRILLING OF FOUNDATION, BEAMS, JOISTS, SLABS, OR COLUMNS SHALL NOT BE PERMITTED UNLESS AUTHORIZED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.
- ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE WHENEVER FEASIBLE. POST-INSTALLED FASTENERS WILL BE PERMITTED WHEN PROVEN TO THE SATISFACTION OF THE STRUCTURAL ENGINEER OF RECORD THAT THE FASTENERS WILL NOT SPALL THE CONCRETE AND HAVE THE SAME CAPACITY AS CAST-IN-PLACE INSERTS.
- WHEN INSTALLING EXPANSION BOLTS OR ADHESIVE ANCHORS, THE CONTRACTOR SHALL TAKE MEASURES TO AVOID DRILLING OR CUTTING ANY OF THE EXISTING REINFORCEMENT.
- ALL REINFORCEMENT MARKED CONTINUOUS SHALL HAVE FULL CLASS B TENSION LAP SPLICES AT ALL SPLICE LOCATIONS AND CORNERS, UNLESS NOTED OTHERWISE ON DRAWINGS. LAP SPLICES FOR TOP REINFORCEMENT SHALL BE LOCATED AT THE CENTER OF SPANS, AND LAP SPLICES FOR BOTTOM REINFORCEMENT SHALL BE LOCATED AT SUPPORTS.
- WELDING OF REINFORCEMENT IS NOT PERMITTED. MECHANICAL SPLICES SHALL DEVELOP 125% OF THE YIELD STRENGTH OF THE BARS BEING SPLICED AND SHALL DEVELOP 100% OF THE TENSILE STRENGTH OF BARS AT BOUNDARY ELEMENTS, THEIR USE IS SUBJECT TO THE APPROVAL IN WRITING OF THE STRUCTURAL ENGINEER OF RECORD.
- NO CONDUIT OR DUCTS, UNDERFLOOR OR ANY OTHER TYPE ARE PERMITTED IN ANY BEAM, GIRDER, OR SLAB WITHOUT THE WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER OF RECORD UNLESS SPECIFICALLY SHOWN IN THE DRAWINGS. WHEN CONDUIT IS INSTALLED IN SLABS, ALL REQUIREMENTS OF ACI 318, CHAPTER 6 SHALL BE MET.
- CONCRETE SLABS SHALL BE FINISHED FLAT AND LEVEL TO THE ELEVATION SHOWN ON THE DRAWINGS. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.

- CONSTRUCTION JOINTS FOR REINFORCED CONCRETE SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF ANY SPAN. PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SHOWN ON A SHOP DRAWING AND SUBMITTED TO THE ARCHITECT FOR REVIEW. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS, UNLESS NOTED OTHERWISE. ALL REINFORCING SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS. CONSTRUCTION JOINTS IN BEAMS CARRYING COLUMN LOADS OR OTHER CONCENTRATED LOADS ARE NOT PERMITTED. CONSTRUCTION JOINTS IN SLABS ON METAL DECK SHALL BE LOCATED HALFWAY BETWEEN BEAMS WHERE THE JOINT IS PARALLEL TO THE SPAN AND SHALL BE LOCATED IN THE MIDDLE THIRD OF THE SPAN WHERE THE JOINT IS PERPENDICULAR TO THE SPAN.
- FOUNDATIONS, PILE CAPS, DRILLED PIERS, SLABS, BEAMS, GIRDERS, AND JOISTS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE SECURELY HELD IN PLACE WHILE POURING CONCRETE. IF REQUIRED ADDITIONAL BARS AND STIRRUPS SHALL BE PROVIDED BY THE CONTRACTOR TO FURNISH SUPPORT TO THE REINFORCING STEEL. NO CONCRETE SHALL BE PLACED UNTIL ALL REINFORCEMENT HAS BEEN INSPECTED AND APPROVED BY THE INDEPENDENT INSPECTION AND TESTING AGENCY.
- SEE ARCHITECTURAL DRAWINGS FOR THE TYPE AND LOCATION OF ALL FLOOR FINISHES, FLOOR DEPRESSIONS, PADS, FILLS, AND CURBS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING AND DAMPPROOFING DETAILS.
- SEE SPECIFICATION SECTION 03300 FOR ADDITIONAL CONCRETE REQUIREMENTS INCLUDING TESTING AND SPECIAL INSPECTIONS.

DEFERRED SUBMITTALS

- THE CONTRACTOR SHALL SUBMIT, FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL OF THE FOLLOWING ASSEMBLIES. THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S SPECIALTY ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS SHALL BEAR THE ENGINEER'S SEAL AND SIGNATURE. REVIEW SHALL BE FOR GENERAL COMPLIANCE WITH THE PROJECT PARAMETERS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES AND FOR LOADS IMPOSED ON THE STRUCTURE.
 - NON PRIMARY LOAD-BEARING METAL STUD WALL, MASONRY, WINDOW WALL, CURTAIN WALL, AND EXTERIOR CLADDING SYSTEMS AND RELATED CONNECTIONS. DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES.
 - METAL STAIRS AND METAL RAILINGS. DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. WHERE HEADERS OR OTHER TYPES OF STRUCTURAL MEMBERS HAVE BEEN DESIGNATED BY THE STRUCTURAL ENGINEER OF RECORD TO SUPPORT THE STAIRS, THE CONNECTIONS FROM THE STAIRS SHALL BE DESIGNED SUCH THAT NO ECCENTRIC ORRSONAL FORCES ARE INDUCED IN THESE STRUCTURAL MEMBERS. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING EMBEDS AND HARDWARE AS REQUIRED BY THE STAIR DESIGN.
 - CONCRETE DESIGN MIXES.
 - ALL STRUCTURAL WORK RELATED TO MEANS AND METHODS OF CONSTRUCTION INCLUDING FORMWORK, SHORING, RESHORING, SUPPORT OF EXCAVATION, UNDERPINNING, AND SUPPORT OF ALL CONSTRUCTION EQUIPMENT INCLUDING CRANES AND HOISTS.
 - STEEL JOISTS.
- PLANS FOR THE DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED IN A TIMELY MANNER THAT ALLOWS A MINIMUM OF 30 WORKING DAYS FOR INITIAL PLAN REVIEW BY THE BUILDING OFFICIAL. ALL COMMENTS RELATED TO THE DEFERRED SUBMITTAL MUST BE ADDRESSED TO THE SATISFACTION OF THE PLAN CHECK DIVISION PRIOR TO APPROVAL OF THE SUBMITTAL ITEMS.
- DEFERRED SUBMITTAL ITEMS SHALL NOT BE FABRICATED PRIOR TO APPROVAL BY THE BUILDING OFFICIAL OF THE CALCULATIONS AND DRAWINGS.

FINAL CD ISSUE
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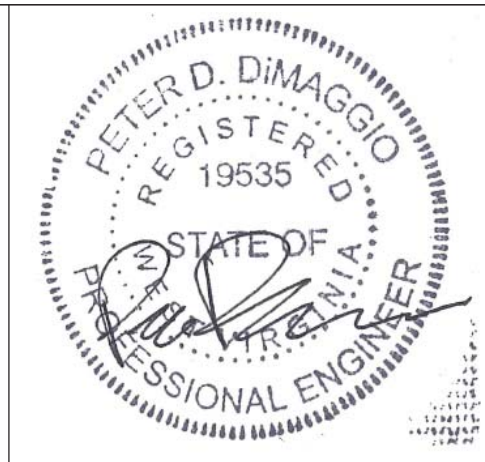
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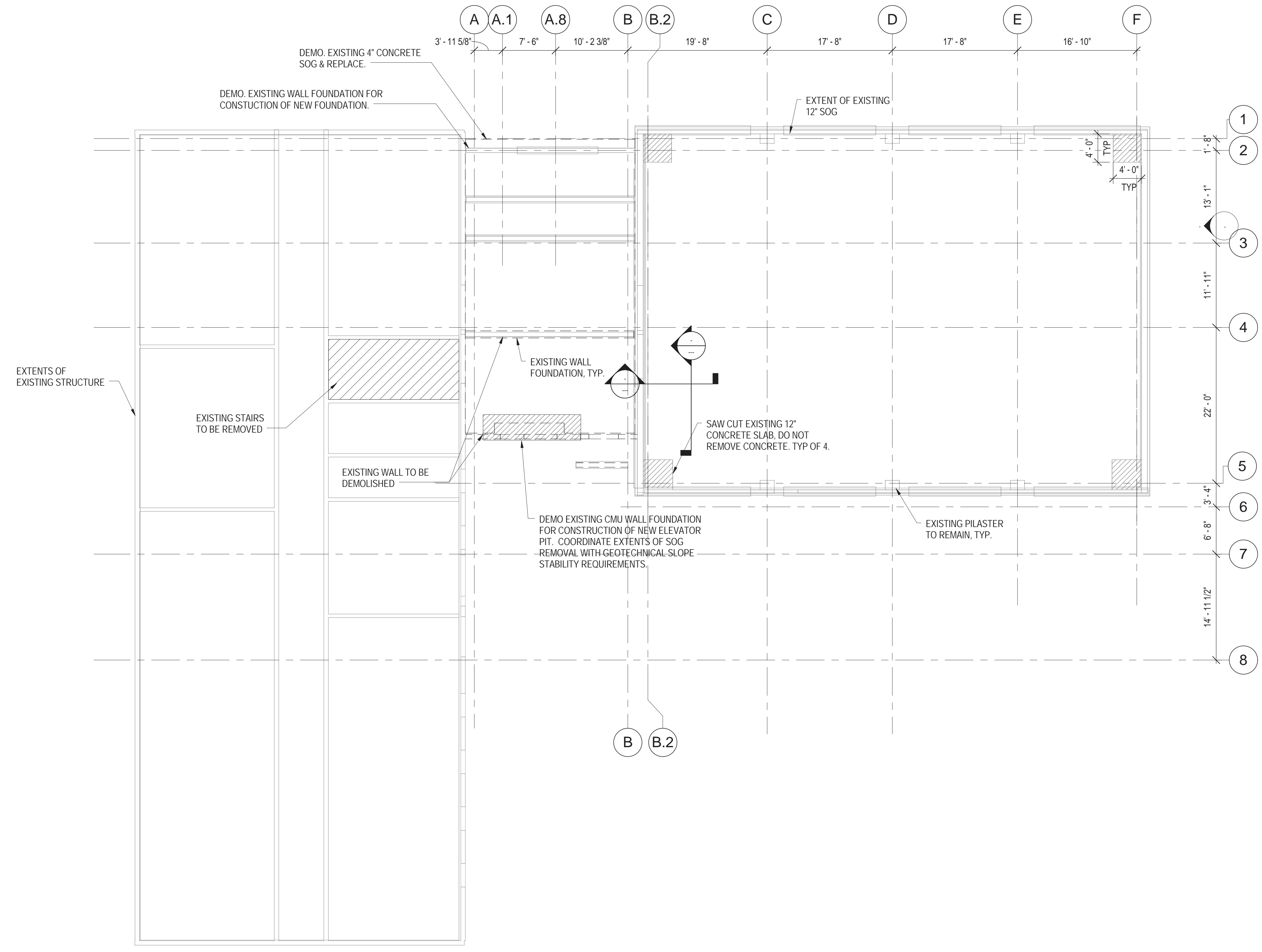


Drawing Title GENERAL NOTES II		Project Title Renovations to the Former BRAC Property		Date 09.03.2013	
Building No. 20		Project Architect: PF&A		S0.02	
Location Huntington, WV		Checked by: MP		Drawn by: VC	
Architects Proj. No. 2099.11		Architects Proj. No. 2099.11		SHEET 0.17 OF 115	



NOTES:

- PLEASE SEE GENERAL NOTES ON S0.1.
- THESE DRAWINGS REPRESENT THE COMPLETED PROJECT, WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS AND FOR THE SUPERIMPOSED LOADS INDICATED IN THE DESIGN LOADS SECTION OF THE GENERAL NOTES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS, INCLUDING EXISTING UNREINFORCED CMU PERIMETER WALLS, AND TO PROVIDE THE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGINGS, BRACING SHEETING AND SHORING, ETC. THE CONTRACTOR MUST COMPLY WITH ALL DEPARTMENT OF BUILDINGS OR STATE BUILDING CODE RULES REGARDING STRUCTURAL STABILITY INTEGRITY DURING CONSTRUCTION OPERATIONS.



1 FOUNDATION DEMOLITION PLAN
1/8" = 1'-0"

Vertical scale bars on the left side of the drawing, each representing a different scale:

- three inches = one foot
- one and one-half inches = one foot
- one inch = one foot
- three-quarters inch = one foot
- one-half inch = one foot
- three-eighths inch = one foot
- one-quarter inch = one foot
- one-eighth inch = one foot

FINAL CD ISSUE
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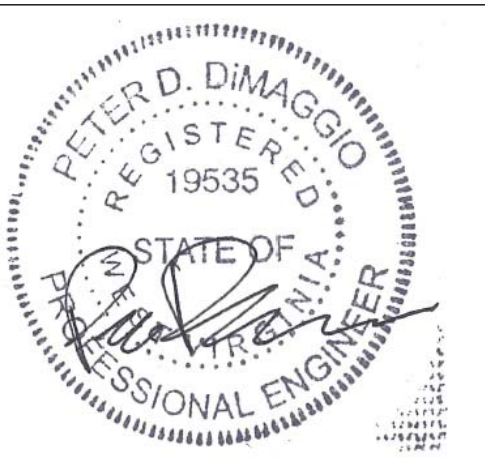
Revisions	Date

Miller-Remick LLC
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A Service Disabled Veteran Owned Small Business

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Drawing Title 1ST FLOOR DEMOLITION PLAN
Building No. 20
Location Huntington, WV

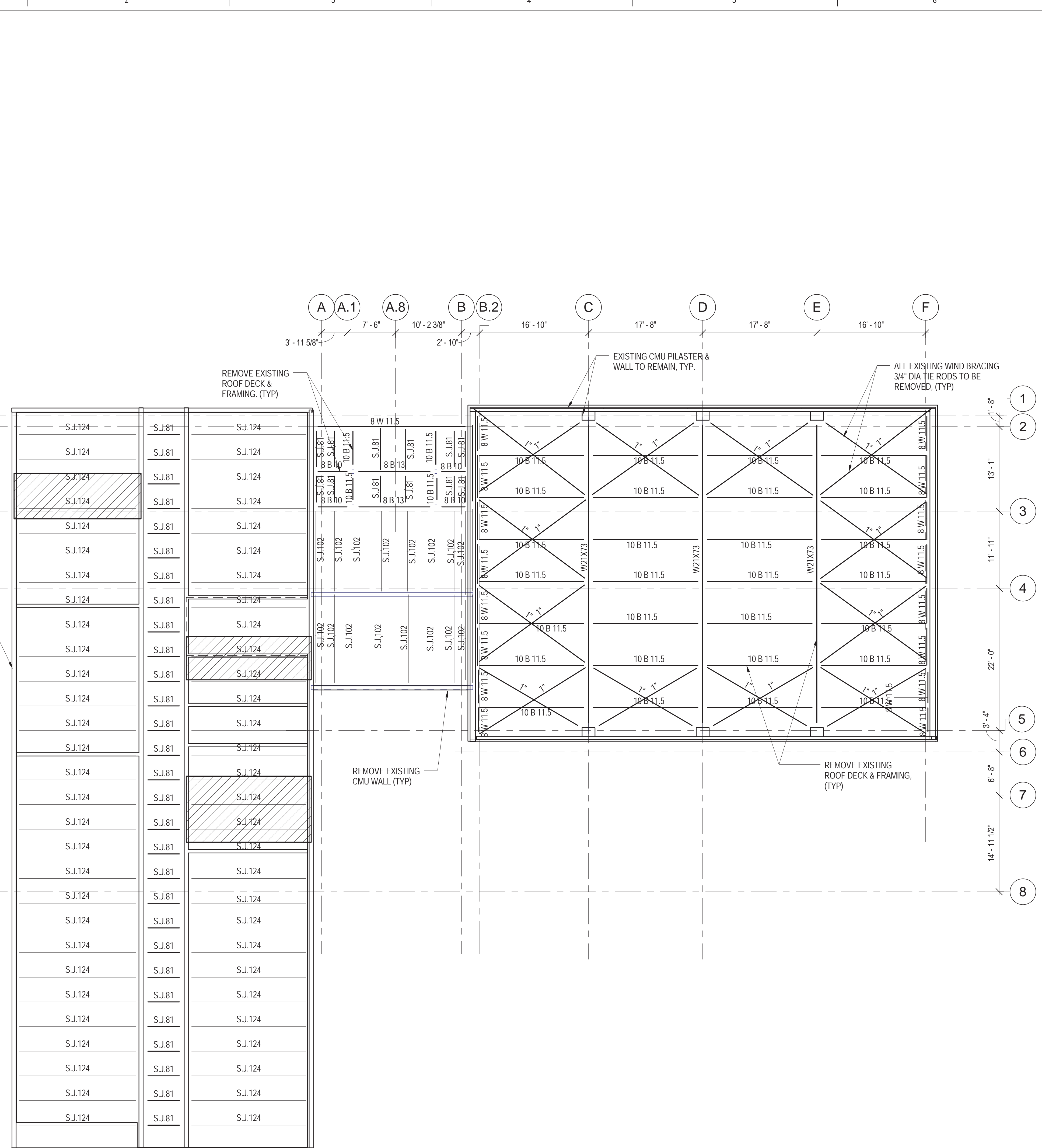
Project Title Renovations to the Former BRAC Property	Checked by: MP	Drawn by: VC
Architects Proj. No. 2099.11	Date 09.03.2013	

Project No. 581-12-101
SD1.01
SHEET 019 OF 115



three inches = one foot
 one and one-half inches = one foot
 one inch = one foot
 three-quarters inch = one foot
 one-half inch = one foot
 three-eighths inch = one foot
 one-quarter inch = one foot
 one-eighth inch = one foot

1
2
3
4
5
6
7
8
9



1 ROOF DEMOLITION PLAN
 1/8" = 1'-0"

- NOTES:
- PLEASE SEE GENERAL NOTES ON S0.1.
 - THESE DRAWINGS REPRESENT THE COMPLETED PROJECT, WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS AND FOR THE SUPERIMPOSED LOADS INDICATED IN THE DESIGN LOADS SECTION OF THE GENERAL NOTES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS, INCLUDING EXISTING UNREINFORCED CMU PERIMETER WALLS, AND TO PROVIDE THE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGINGS, BRACING SHEETING AND SHORING, ETC. THE CONTRACTOR MUST COMPLY WITH ALL DEPARTMENT OF BUILDINGS OR STATE BUILDING CODE RULES REGARDING STRUCTURAL STABILITY INTEGRITY DURING CONSTRUCTION OPERATIONS.
 - CONTRACTOR SHALL REMOVE/REPLACE EXISTING ROOF UNDER NEW MECHANICAL UNITS TO INSTALL JOISTS AS SHOWN ON ROOF FRAMING PLAN.
 - CONTRACTOR SHALL COORDINATE LOCATIONS OF ROOF MECHANICAL UNITS WITH MEPH/VAC DRAWINGS.

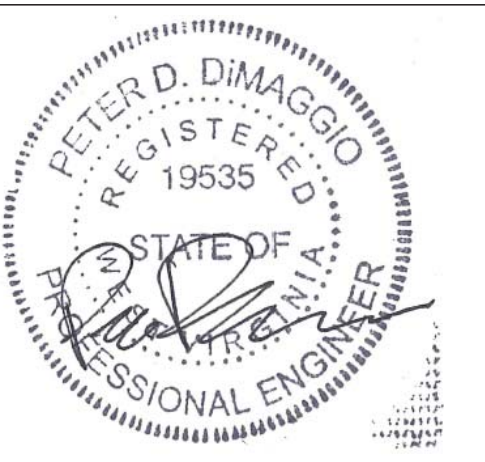
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Drawing Title ROOF DEMOLITION PLAN	
Building No. 20	Location Huntington, WV

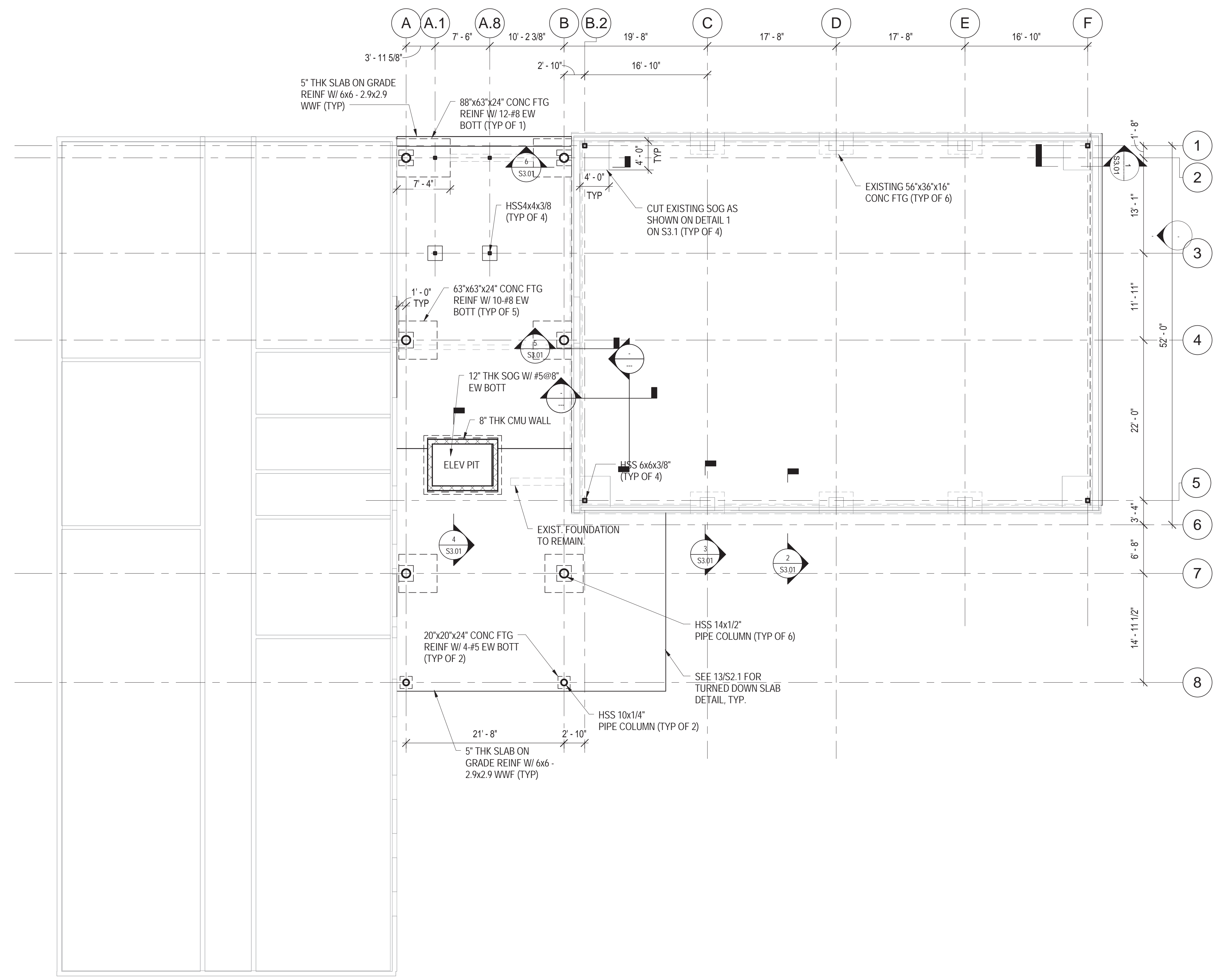
Project Title Renovations to the Former BRAC Property		
Project Architect: PF&A	Checked by: MP	Drawn by: VC
Architects Proj. No. 2099.11		Date 09.03.2013

Project No. 581-12-101	Date 09.03.2013
SD1.02	SHEET 020 OF 115



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three inches = one foot
 one and one-half inches = one foot
 one inch = one foot
 three-quarters inch = one foot
 one-half inch = one foot
 three-eighths inch = one foot
 one-quarter inch = one foot
 one-eighth inch = one foot



1 FIRST FLOOR
 1/8" = 1'-0"

- NOTES:**
- FOUNDATION DESIGN BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 3000 PSF.
 - ALL BOTTOM OF NEW FOOTINGS TO BE ALIGNED W/ BOTTOM OF EXISTING FOOTINGS, UNO.
 - CENTERLINE OF FOOTINGS SHALL COINCIDE WITH CENTERLINE OF COLUMNS UNLESS NOTED OTHERWISE.
 - SEE SHEET S4.1 FOR COLUMN FOOTING SCHEDULES AND DETAILS.
 - SEE SHEET S4.1 FOR COLUMN SCHEDULE.
 - SEE SHEETS S0.1, S0.2 AND S0.3 FOR GENERAL NOTES.
 - INDICATES CMU WALL, SEE REINFORCED MASONRY NOTE 9 FOR REINFORCING.

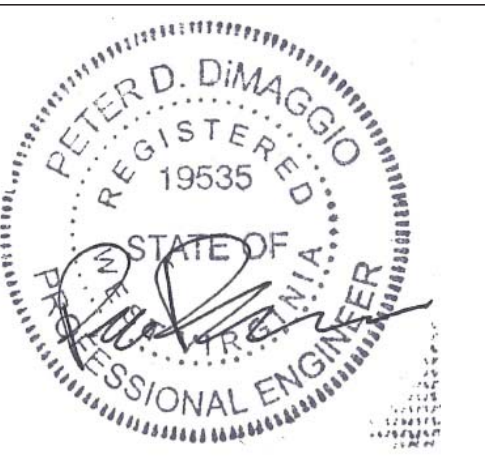
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Drawing Title 1ST FLOOR PLAN	
Building No. 20	Location Huntington, WV

Project Title Renovations to the Former BRAC Property		
Project Architect: PF&A	Checked by: MP	Drawn by: VC
Architects Proj. No. 2099.11		Date 09.03.2013

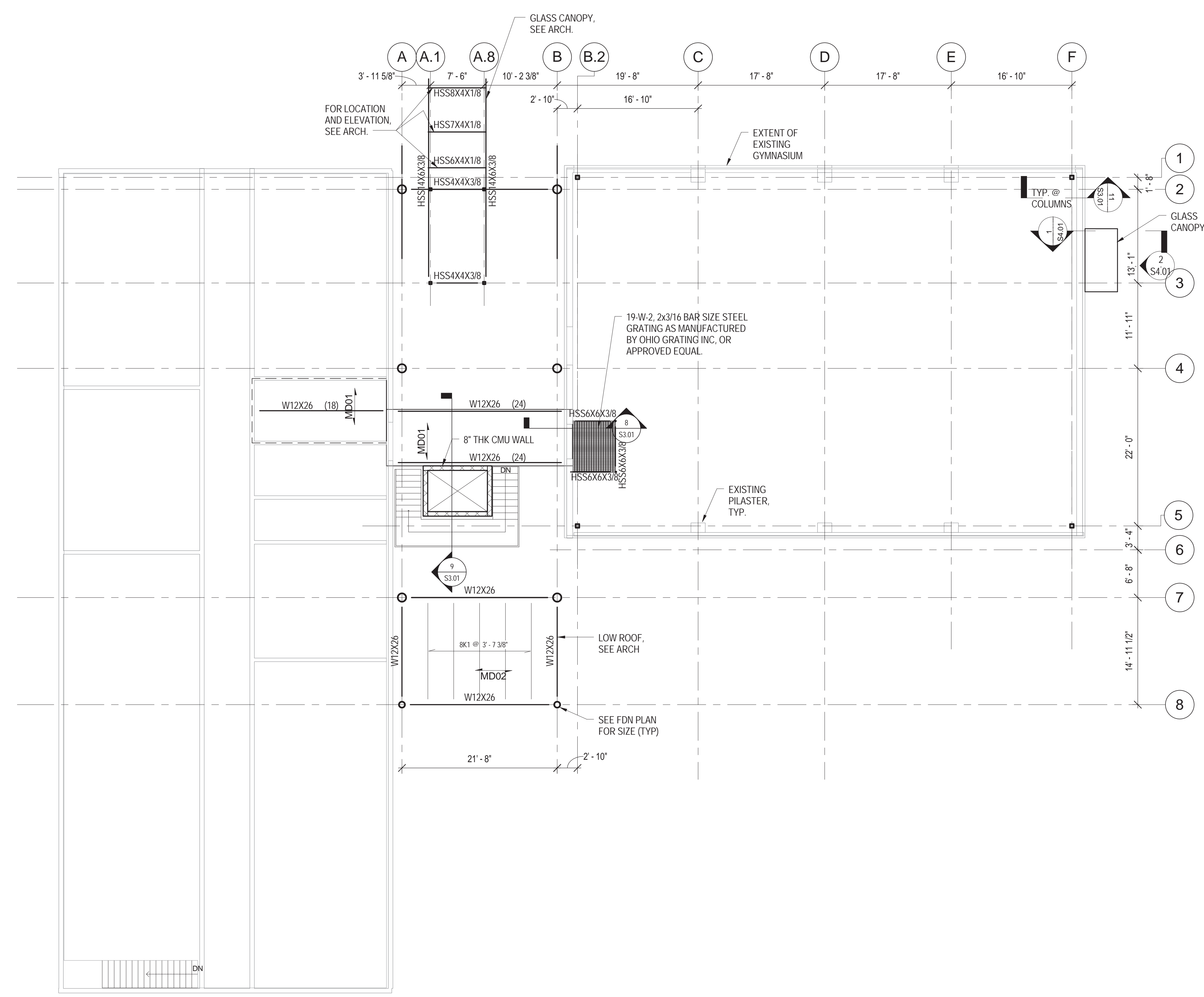
Project No. 581-12-101	Date 09.03.2013
S1.01	SHEET 021 OF 115

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three inches = one foot
 one and one-half inches = one foot
 one inch = one foot
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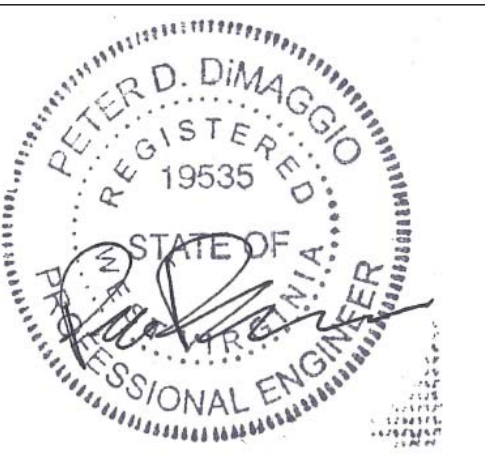


1 SECOND LEVEL FRAMING PLAN
 1/8" = 1'-0"

NOTES:

- TOP OF SLAB ELEVATION SHALL BE 10'-0" UNLESS NOTED OTHERWISE THUS ±____ INDICATING DISTANCE ABOVE OR BELOW TOP OF SLAB.
- MD01 INDICATES DIRECTION OF COMPOSITE METAL DECK PLUS LIGHTWEIGHT CONCRETE TOPPING. SEE SHEET S2.2 FOR METAL DECK SCHEDULE AND TYPICAL SLAB DETAILS.
- MD02 INDICATES DIRECTION OF NON-COMPOSITE METAL ROOF DECK. SEE SHEET S2.2 FOR METAL DECK SCHEDULE AND TYPICAL DETAILS.
- TOP OF STRUCTURAL STEEL ELEVATION SHALL BE 4 3/4' (3 1/4' LW TOPPING + 1 1/2' DECK BELOW THE TOP OF SLAB ELEVATION UNLESS NOTED OTHERWISE THUS ±____) INDICATING DISTANCE ABOVE OR BELOW TOP OF SLAB. SEE SHEETS S2.1 FOR TYPICAL BEAM DETAILS.
- STRUCTURAL STEEL BEAMS SHALL BE EQUALLY SPACED WITHIN BAYS UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB LOCATIONS AT SLAB OPENINGS AND BUILDING PERIMETER.
- SEE DRAWING S4.1 FOR COLUMN SCHEDULE.
- COORDINATE ALL OPENINGS, SLAB DEPRESSIONS, CURBS, AND SLOPES WITH ARCH. AND MEP DRAWINGS.
- UNLESS PROVIDED IN PLAN, SEE GENERAL NOTE SHEET S-001 FOR VALUE OF BEAM END REACTIONS AND ADDITIONAL INFORMATION REGARDING BEAM CONNECTION DESIGN.
- SEE TYPICAL DETAIL SHEETS FOR ELEVATOR SILL AND ELEVATOR GUIDE RAIL SUPPORT INFO.
- SEE DETAILS 8 TO 12 ON S3.1 FOR CONNECTIONS FROM EXISTING MASONRY TO NEW FRAMING.
- ▨▨▨▨▨ INDICATES CMU WALL, SEE REINFORCED MASONRY NOTE 9 FOR REINFORCING.

Revisions	Date

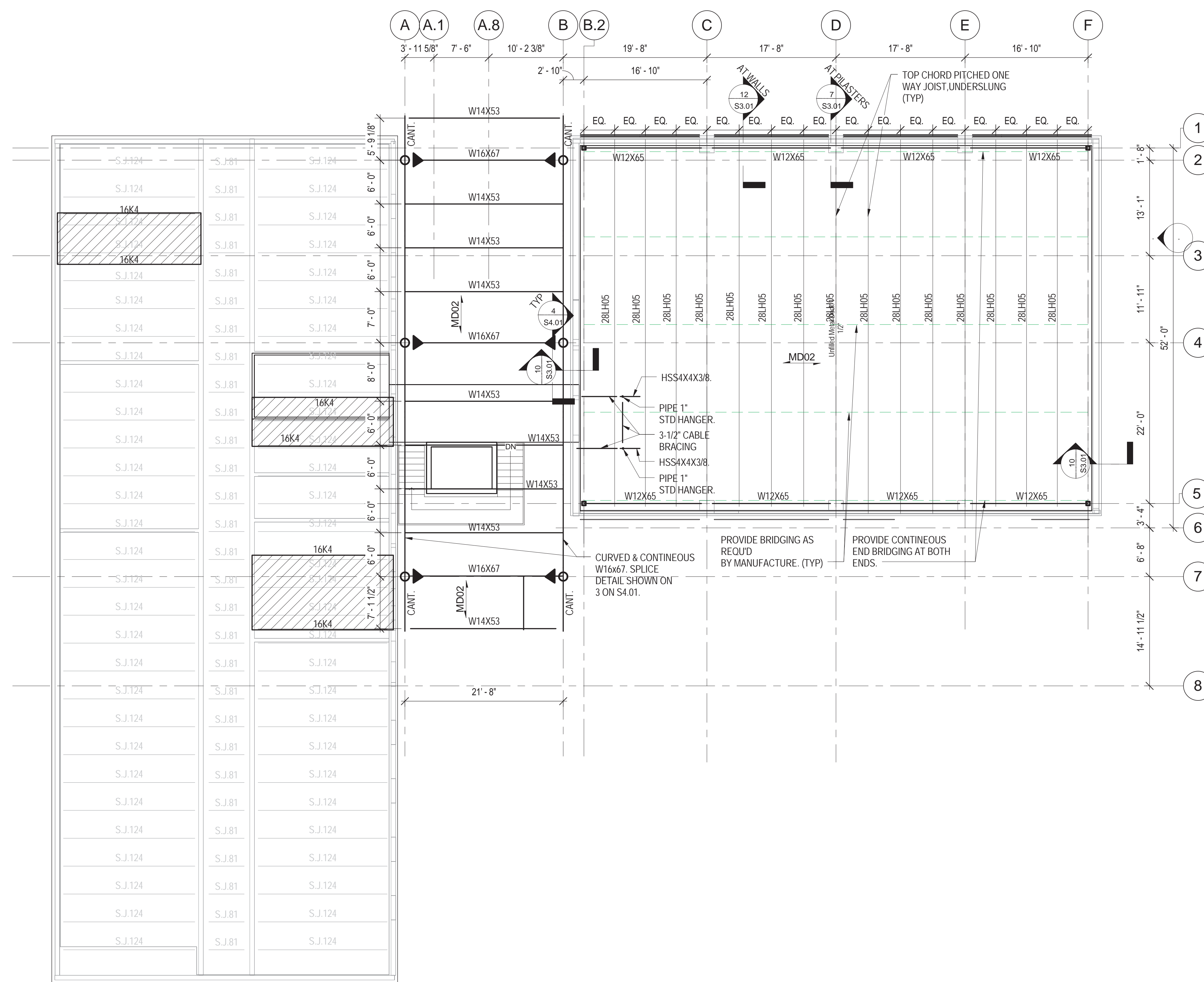
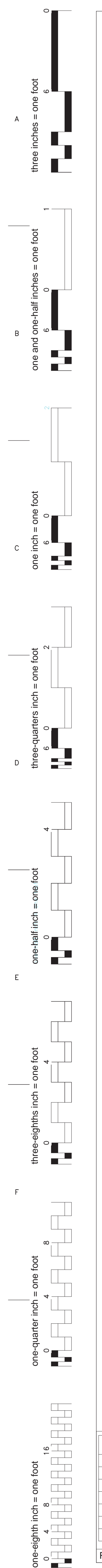



Drawing Title SECOND FLOOR FRAMING PLAN		Project Title Renovations to the Former BRAC Property		Date 09.03.2013	
Building No. 20		Project Architect: PF&A		Checked by: MP	
Location Huntington, WV		Architects Proj. No. 2099.11		Drawn by: VC	
SHEET 022 OF 115				S1.02	

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- NOTES:**
1. TOP OF STEEL ELEVATION VARIES.
 2. MDO2 INDICATES DIRECTION OF NON-COMPOSITE METAL ROOF DECK. SEE SHEET S2.2 FOR METAL DECK SCHEDULE AND TYPICAL DETAILS.
 3. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB LOCATIONS AT SLAB OPENINGS AND BUILDING PERIMETER.
 4. SEE DRAWING S4.1 FOR COLUMN SCHEDULE.
 5. COORDINATE ALL OPENINGS, SLAB DEPRESSIONS, CURBS, AND SLOPES WITH ARCH. AND MEP DRAWINGS. SEE S2.2 FOR BEAM SIZES.
 6. COORDINATE LOCATION OF HOIST BEAMS WITH LOCATION OF ELEVATOR CAB LOCATION.
 7. REFER TO ELEVATOR MANUFACTURER FOR LOCATION OF POSTS.
 8. SEE DETAILS 8 TO 12 ON S3.1 FOR CONNECTIONS OF NEW FRAMING TO EXISTING MASONRY.
 9. CONTRACTOR SHALL COORDINATE LOCATIONS OF ROOF MECHANICAL UNITS WITH MEPI/HVAC DRAWINGS.
 10.  CONTRACTOR SHALL REMOVE/REPLACE EXISTING ROOF UNDER NEW MECHANICAL UNITS TO INSTALL JOISTS AS SHOWN ON PLAN.

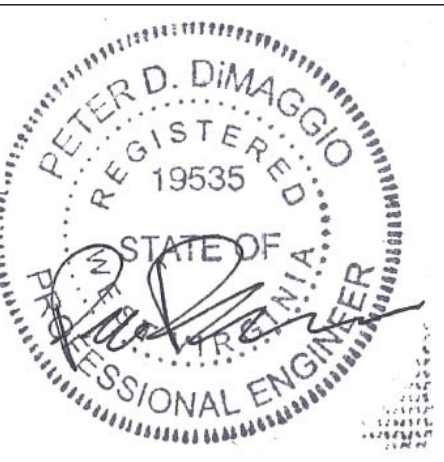
1 ROOF LEVEL FRAMING PLAN
1/8" = 1'-0"

Revisions	Date

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M.E.P. & Structural Engineering
A Service Disabled Veteran Owned Small Business

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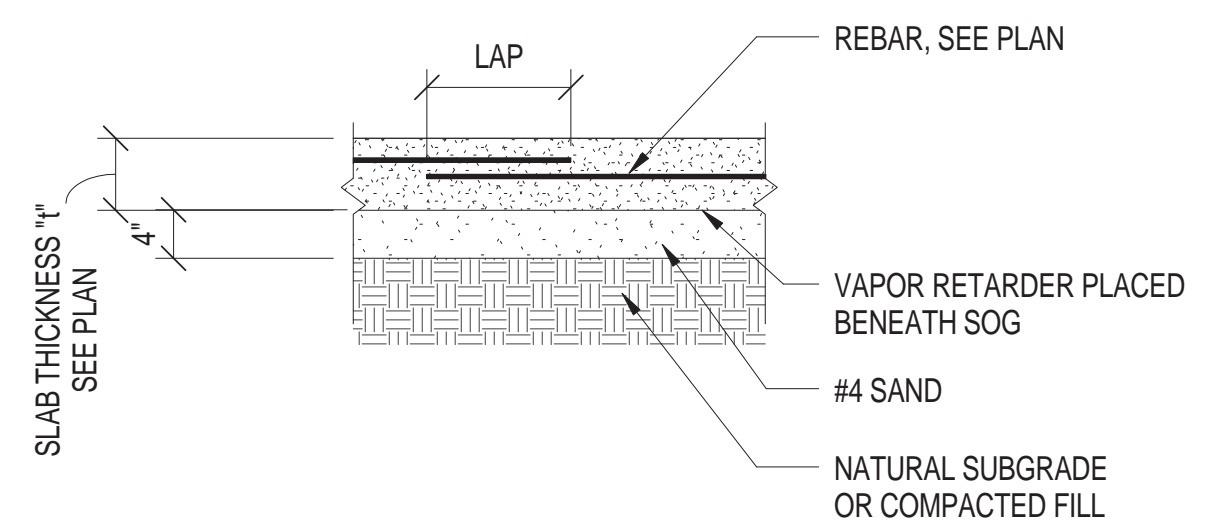
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40 Wall Street, NY, NY 10005



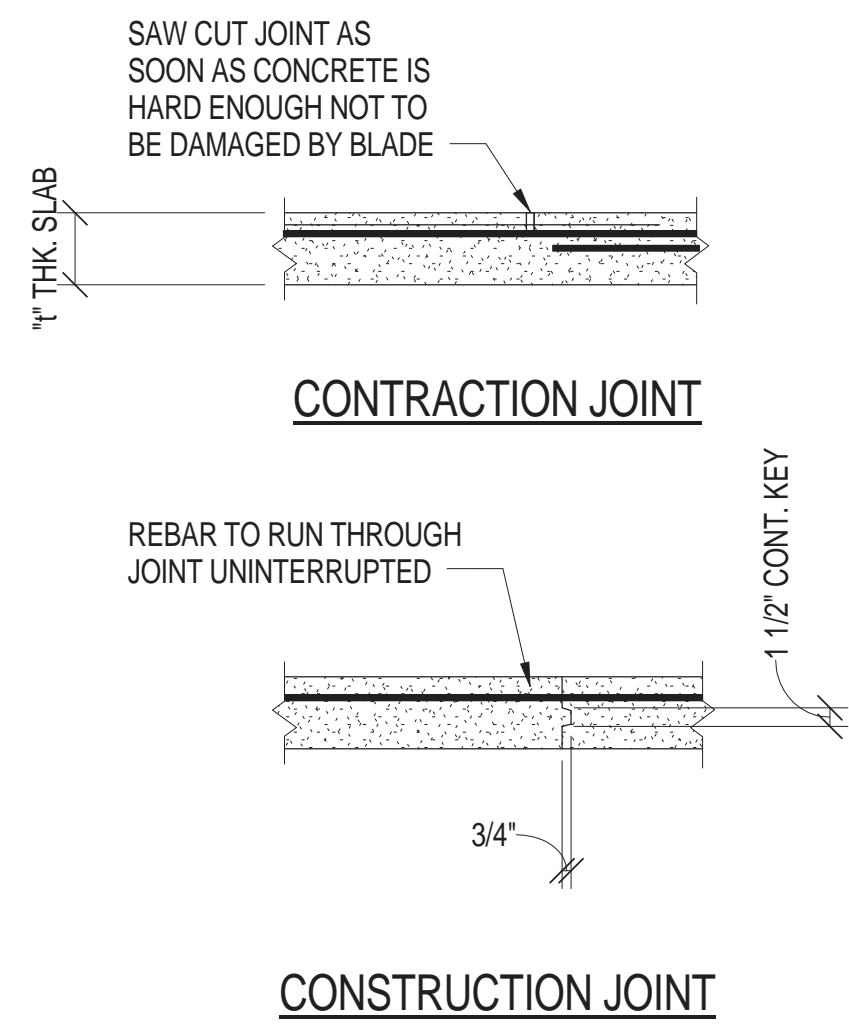
Drawing Title ROOF FRAMING PLAN	Project Title Renovations to the Former BRAC Property	Date 09.03.2013
Building No. 20	Project Architect: PF&A	Checked by: MP
Location Huntington, WV	Architects Proj. No. 2099.11	Drawn by: VC
		S1.03
		SHEET 023 OF 115

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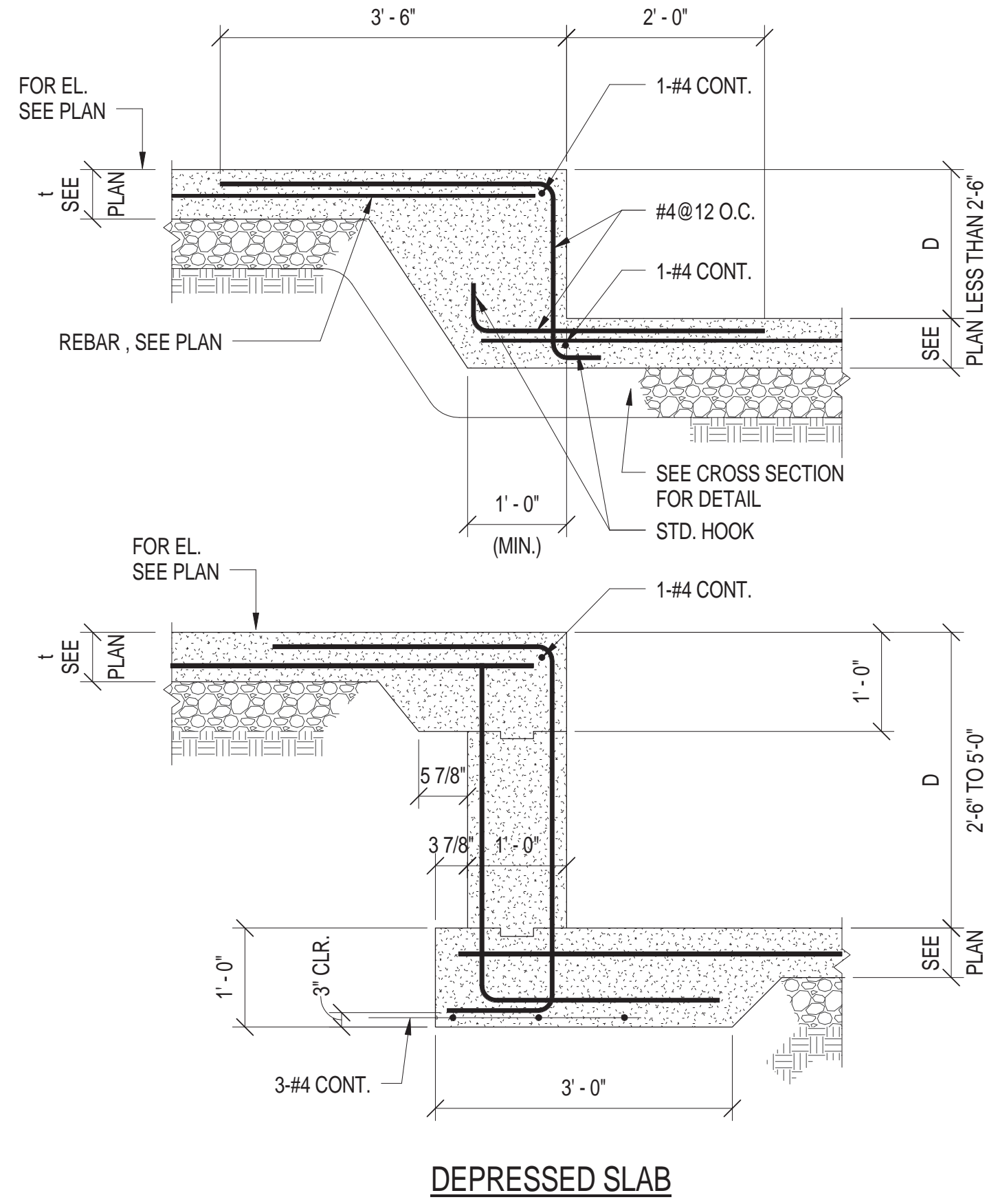




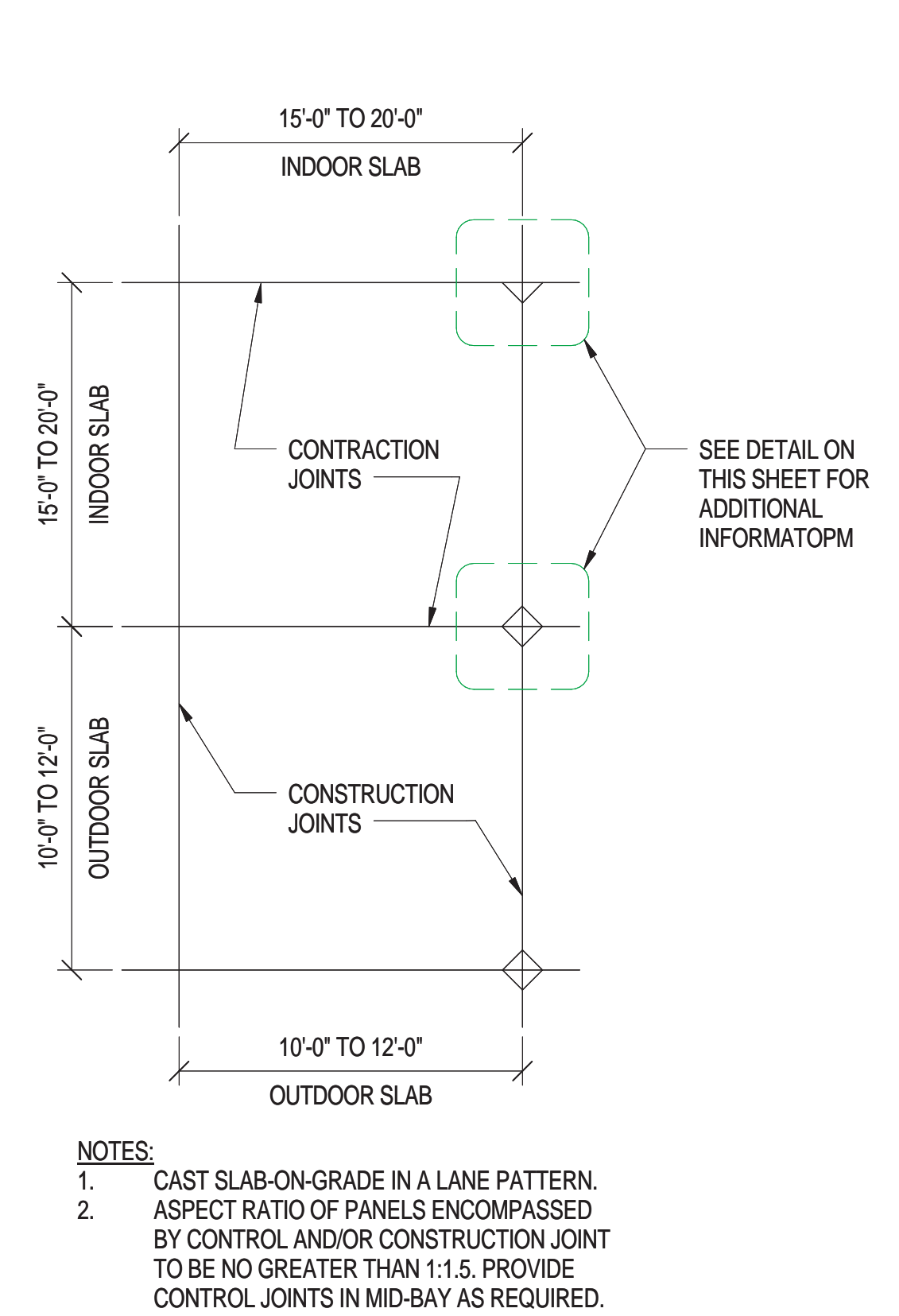
1 TYPICAL SLAB ON GRADE CROSS SECTION
 NTS



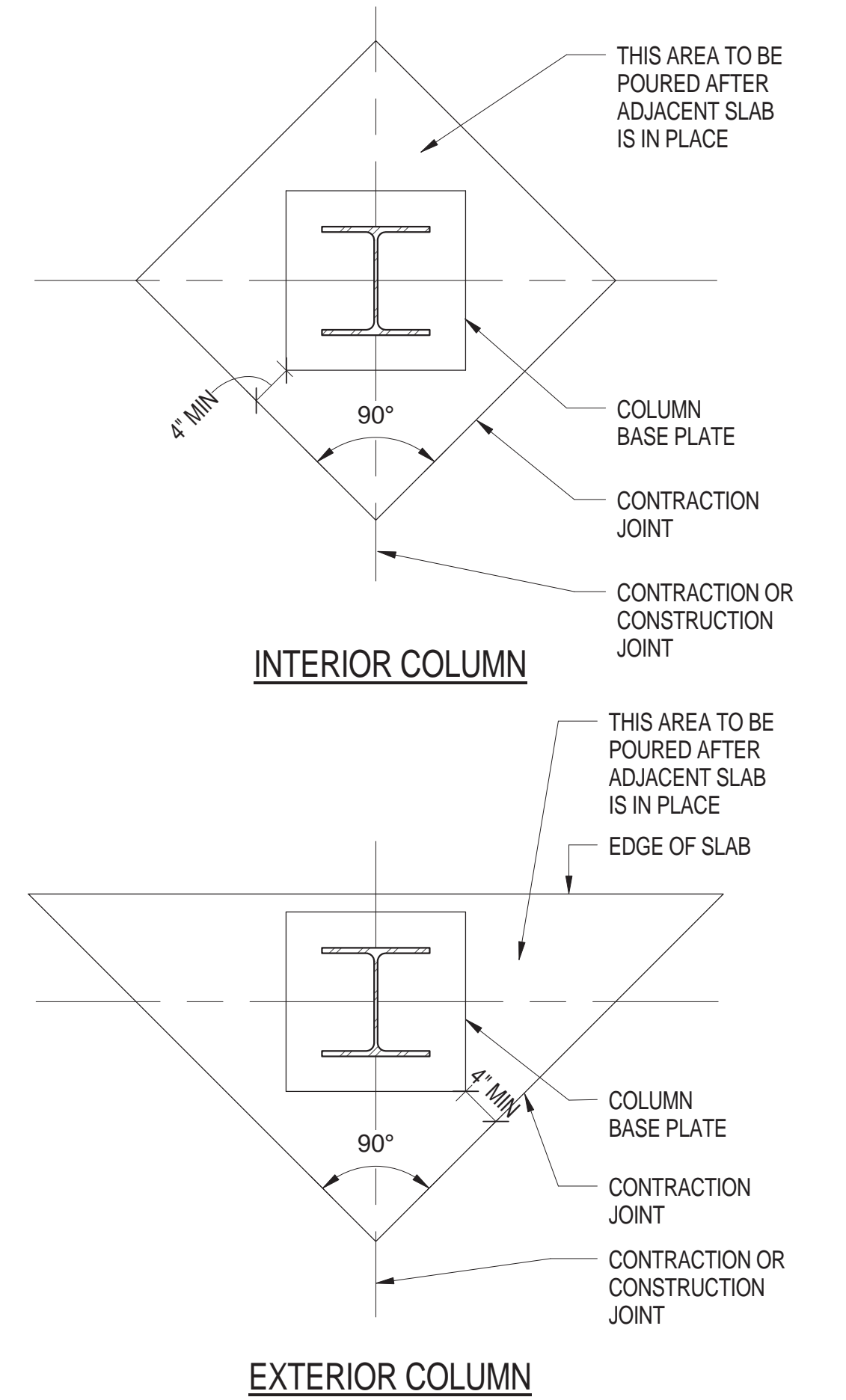
2 TYPICAL SLAB ON GRADE JOINT DETAILS
 NTS



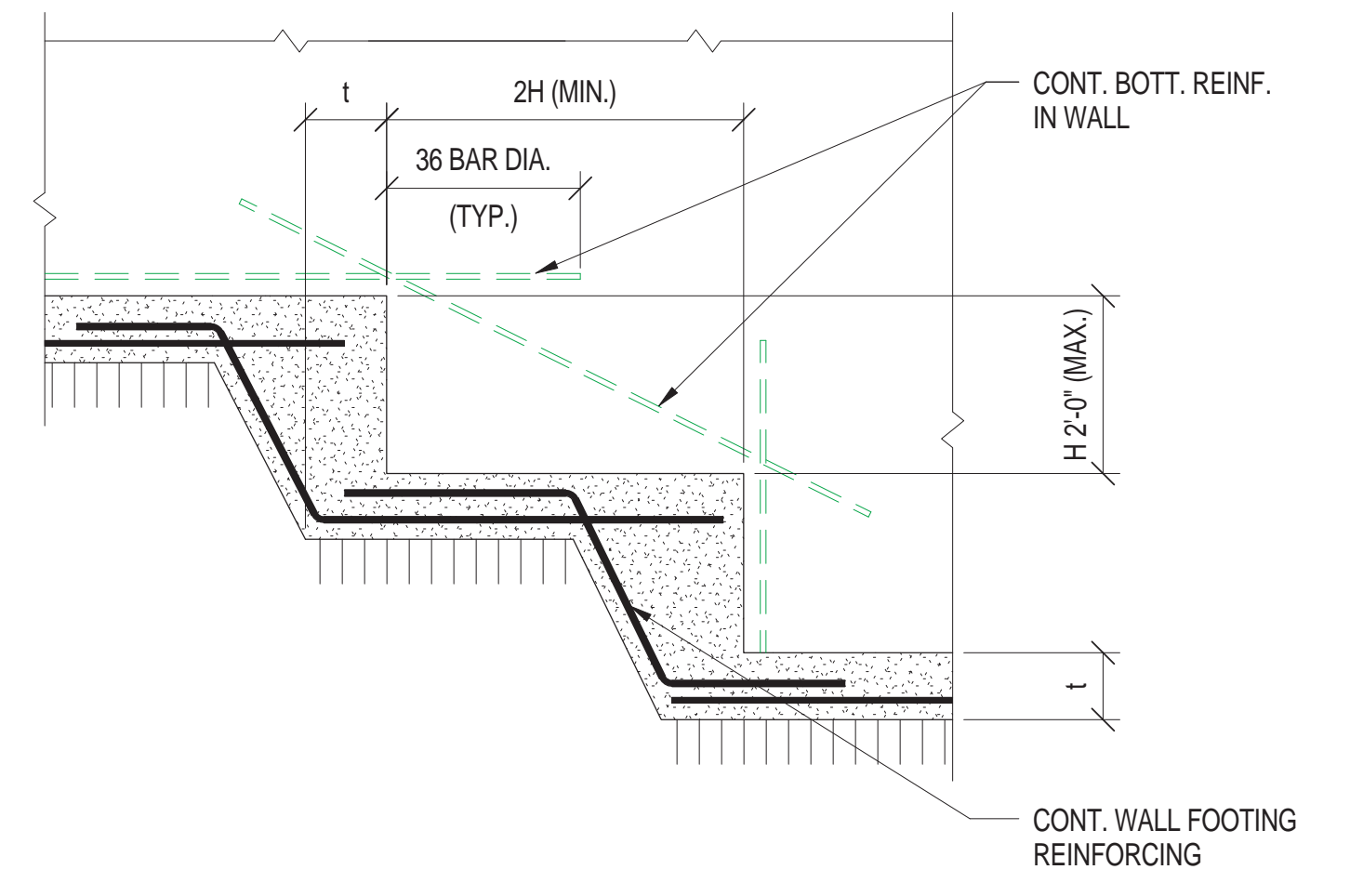
3 SLAB ON GRADE DETAILS
 NTS



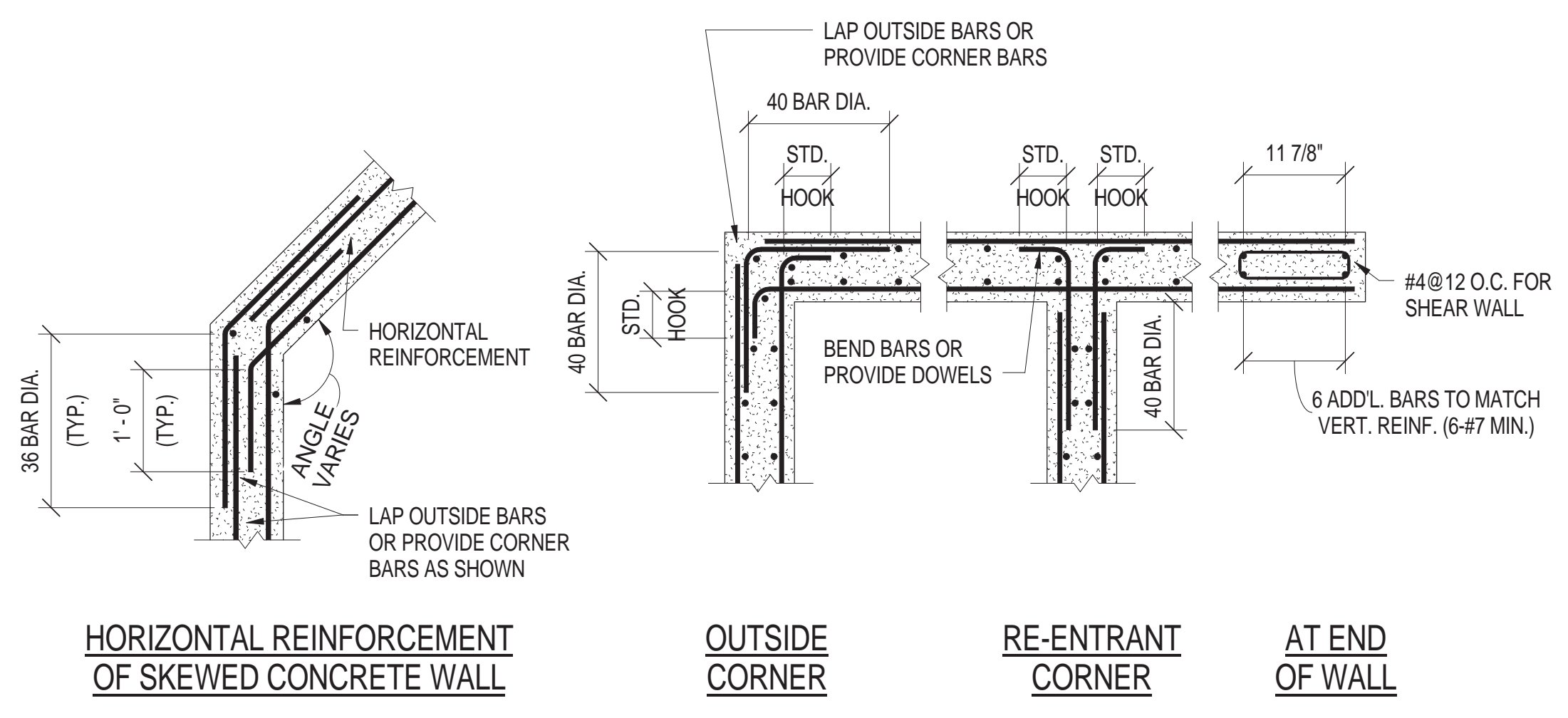
4 TYPICAL SLAB ON GRADE JOINT LAYOUT
 NTS



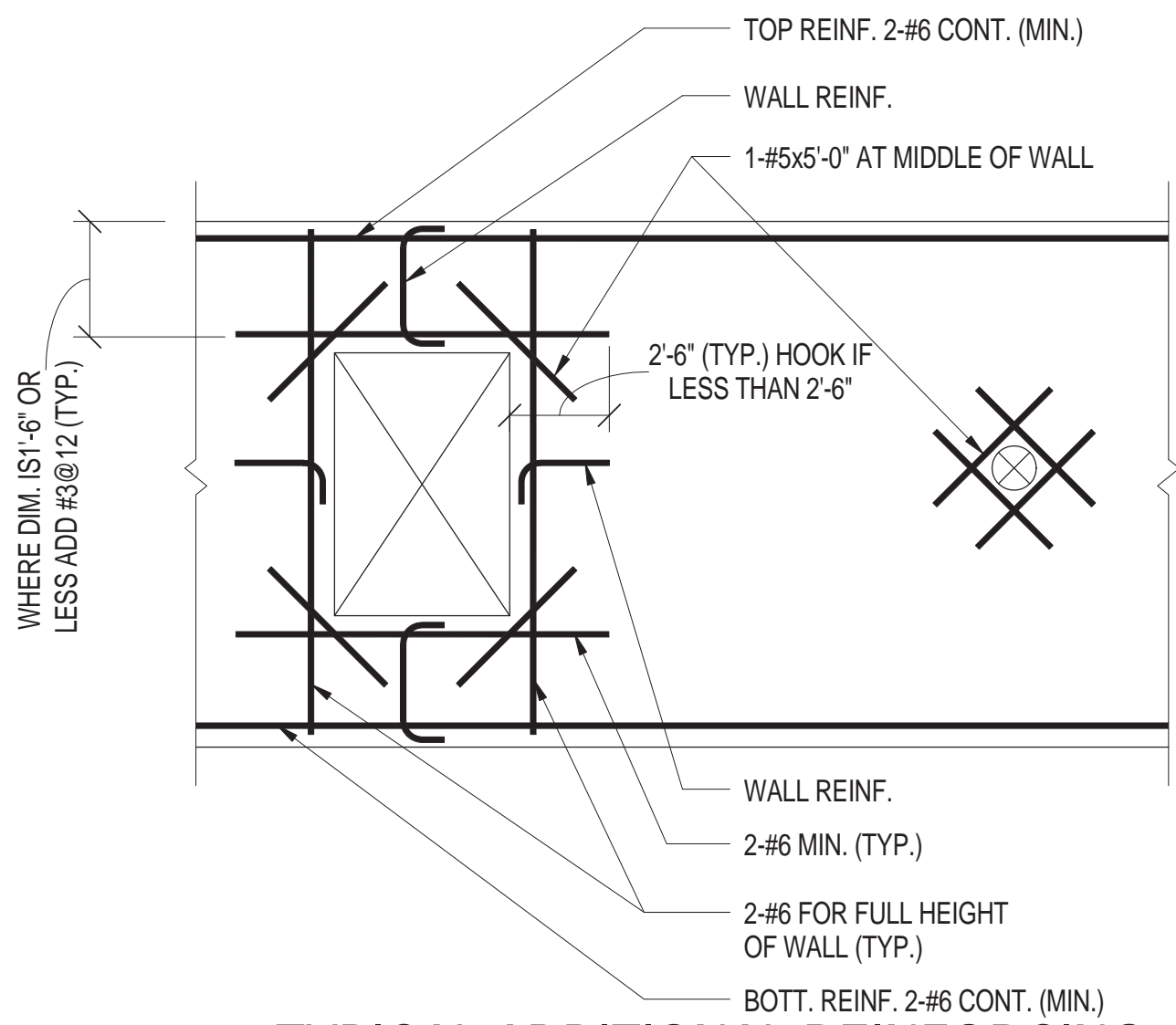
5 SLAB ON GRADE ISOLATION JOINTS
 NTS



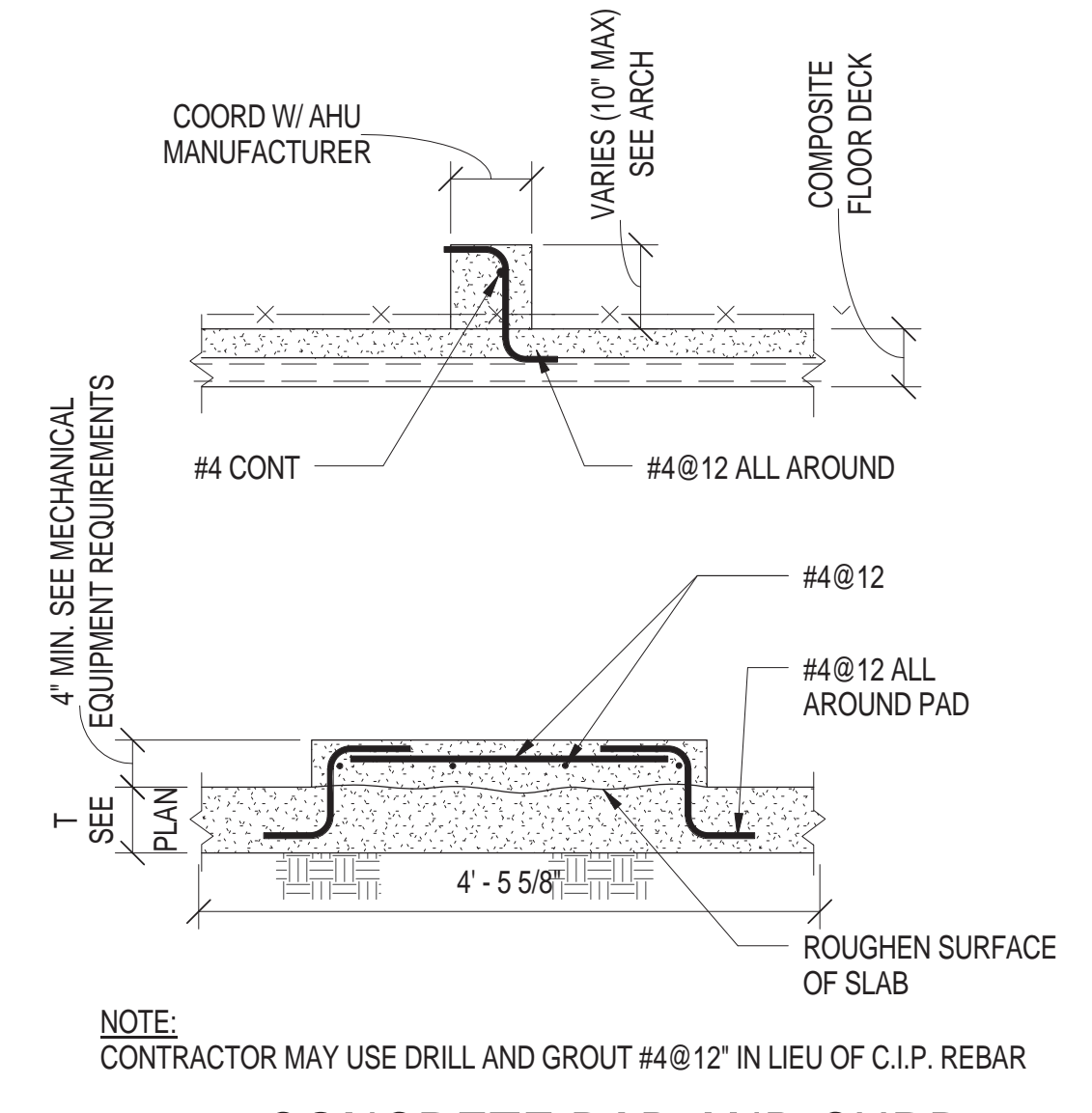
6 STEPPED WALL FOOTING AT SOIL
 NTS



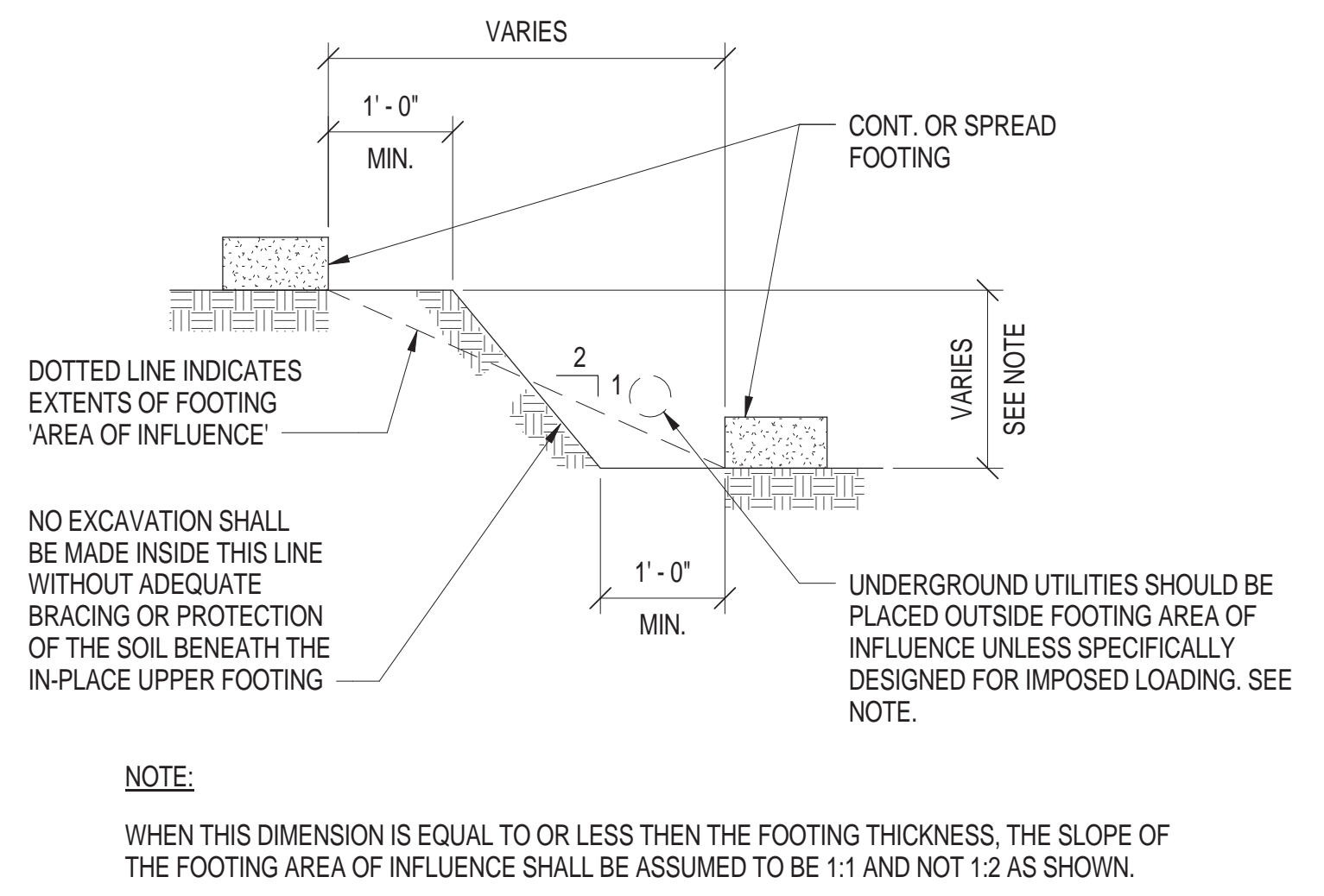
7 TYPICAL CONCRETE WALL REINFORCING
 NTS



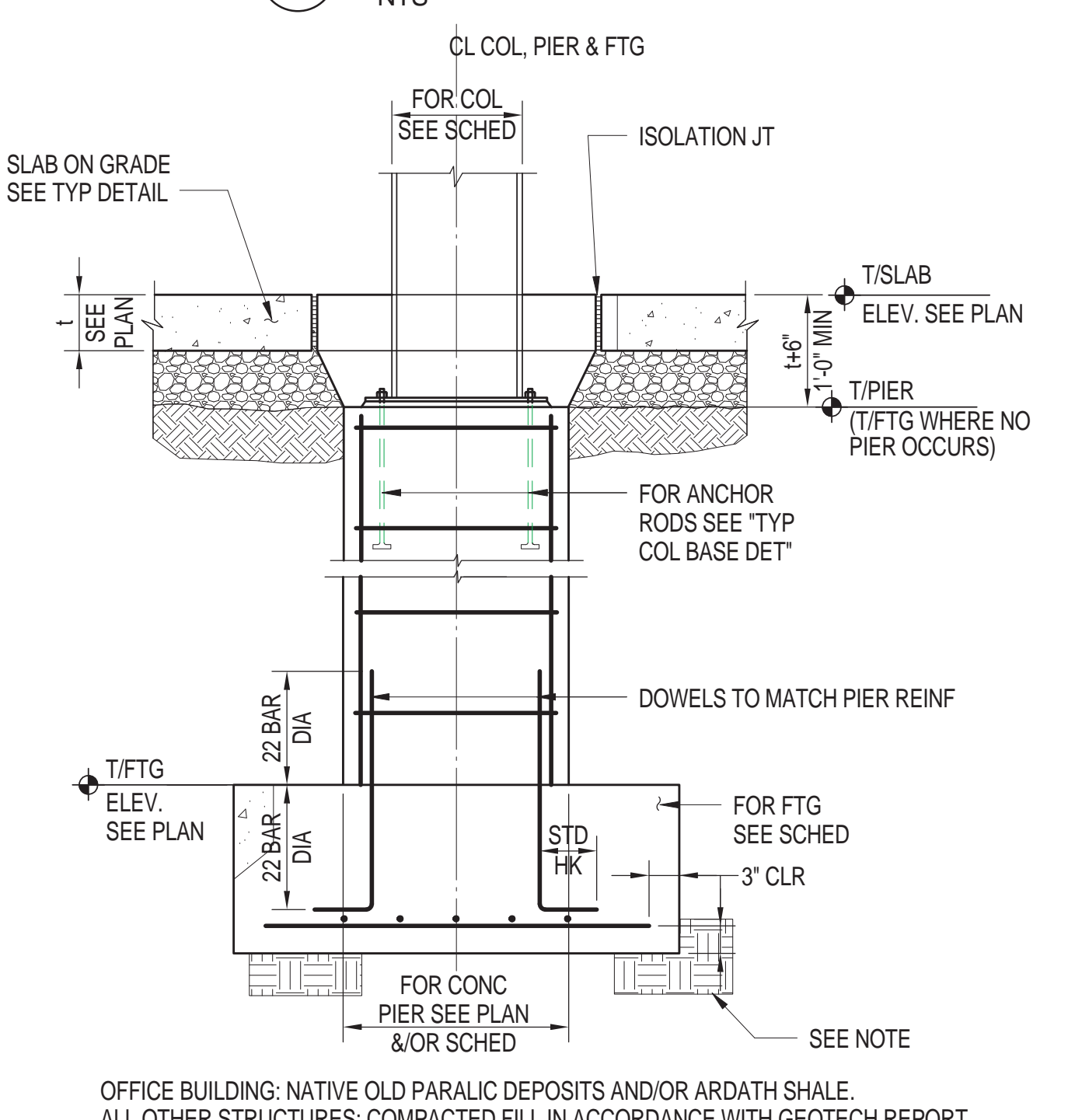
8 TYPICAL ADDITIONAL REINFORCING AT OPENINGS IN CONCRETE WALL
 NTS



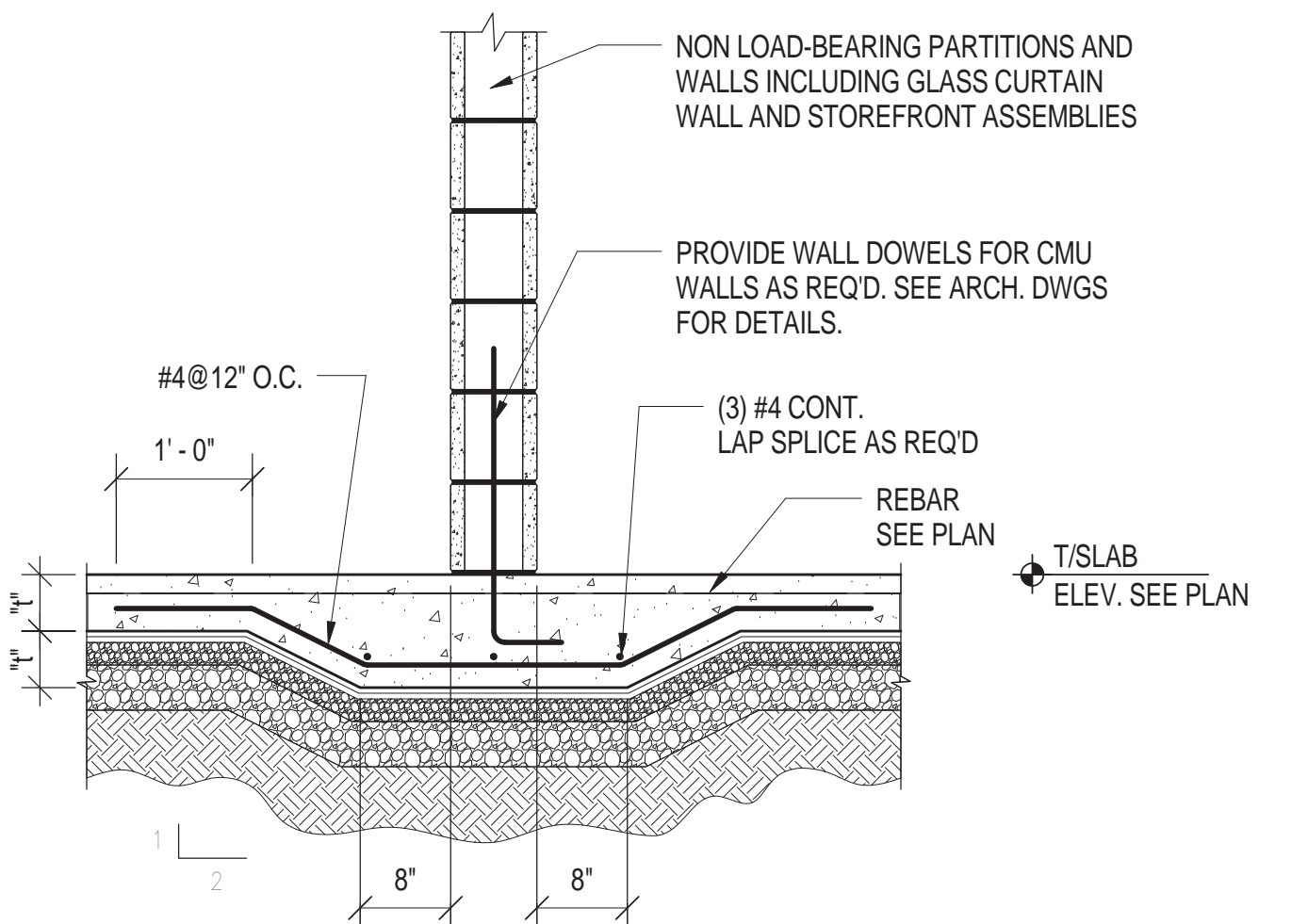
9 CONCRETE PAD AND CURB FOR MECHANICAL EQUIPMENT
 NTS



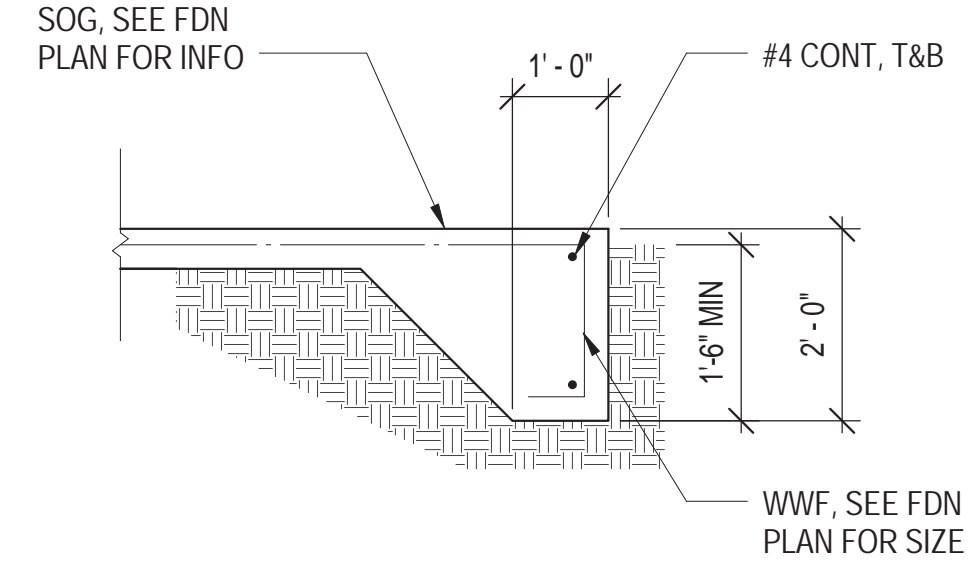
10 SLOPE BETWEEN FOOTING AND ADJACENT CONSTRUCTION
 NTS



11 TYPICAL COLUMN FOOTING DETAIL
 NTS



12 THICKENED SLAB-ON-GRADE AT NON LOAD BEARING PARTITIONS AND WALLS
 NTS



13 SOG EDGE DETAIL
 NTS

Revisions	Date

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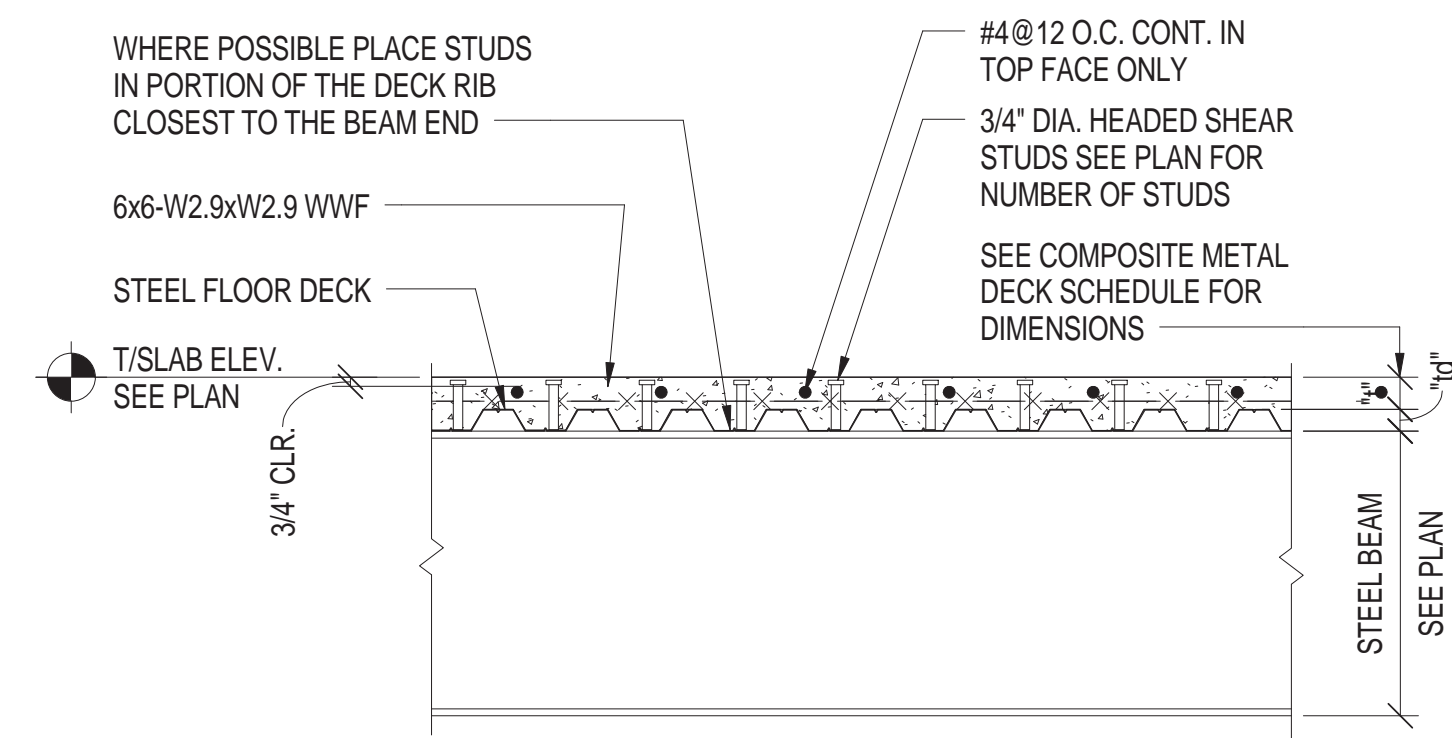
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 40 Wall Street, NY, NY 10005

PETER D. DIMAGGIO
 REGISTERED PROFESSIONAL ENGINEER
 19535
 STATE OF VIRGINIA

Drawing Title TYPICAL DETAILS I	Project Title Renovations to the Former BRAC Property	Date 09.03.2013
Building No. 20	Project Architect: PF&A	Project No. 581-12-101
Location Huntington, WV	Checked by: MP	Drawn by: VC
Architects Proj. No. 2099.11	S2.01 SHEET 024 OF 115	

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Veterans Affairs

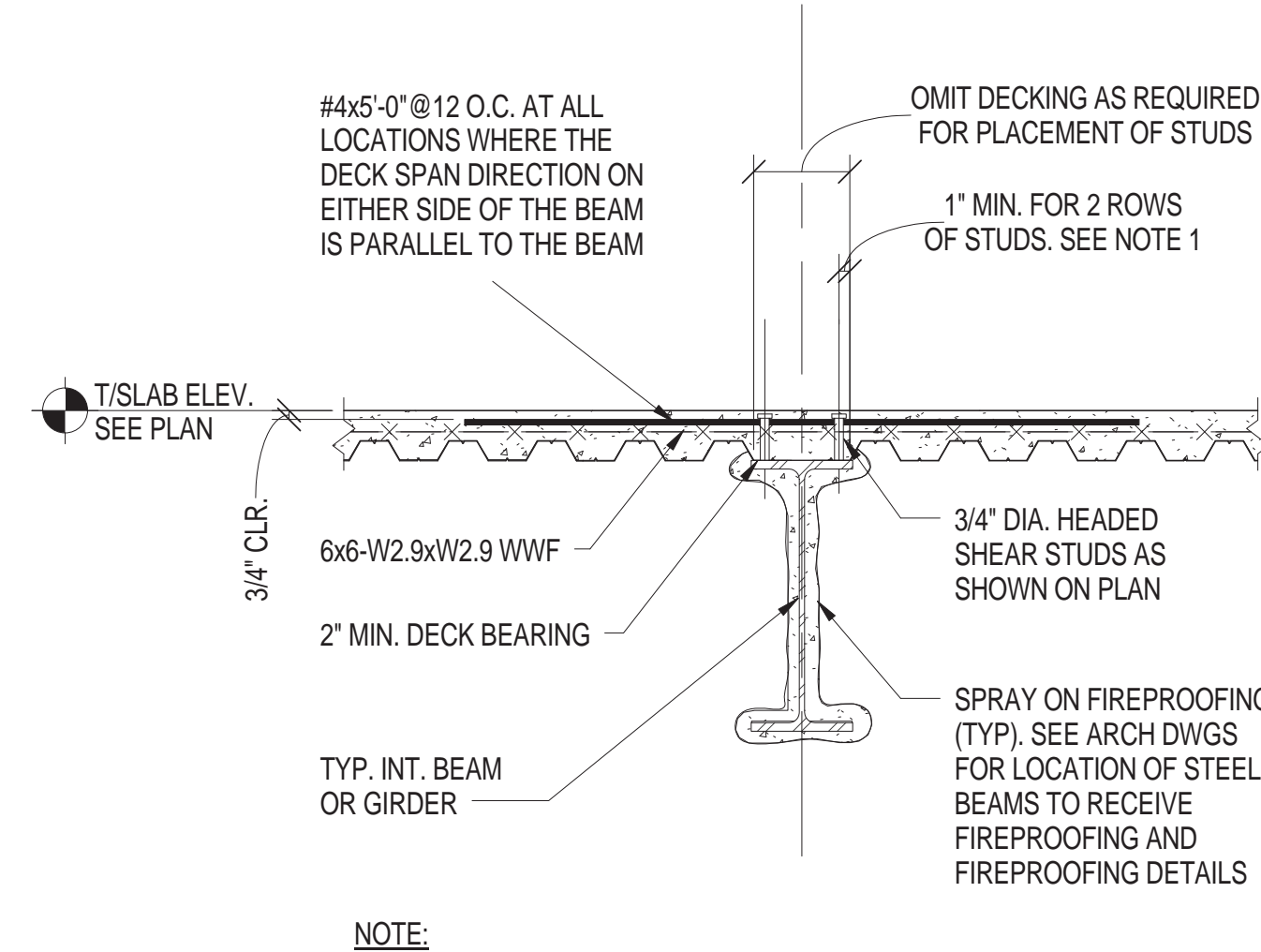


METAL DECK SCHEDULE					
MARK	DECK HEIGHT 1/4"	SLAB THICKNESS 1"	WWF	3/4" SHEAR STUD LENGTH	REMARKS
MDD1	1 1/2"	3 1/4"	6x6-W2.9xW2.9	4"	SEE NOTES 1 & 2
MDD2	1 1/2"	-	-	-	SEE NOTE 3

- NOTES:**
- SHEAR STUDS SHALL BE UNIFORMLY DISTRIBUTED ALONG BEAM UNLESS MORE THAN ONE GROUP IS SHOWN ON PLAN.
 - WHEN MORE STUDS ARE REQUIRED THAN CAN BE PLACED IN ONE ROW, PLACE DOUBLE STUDS AT EACH END OF BEAM TOWARDS MIDSPAN.
 - MAXIMUM SPACING OF STUDS = 36" O.C. (STEEL FLOOR DECK REQUIRES A PLUG WELD BETWEEN STUDS IF STUD SPACING EXCEEDS 12" O.C.) ANCHORAGE SHALL NOT EXCEED 18 O.C.
 - MINIMUM SPACING OF STUDS ALONG LONGITUDINAL AXIS OF BEAM OR GIRDER = 4 1/2".

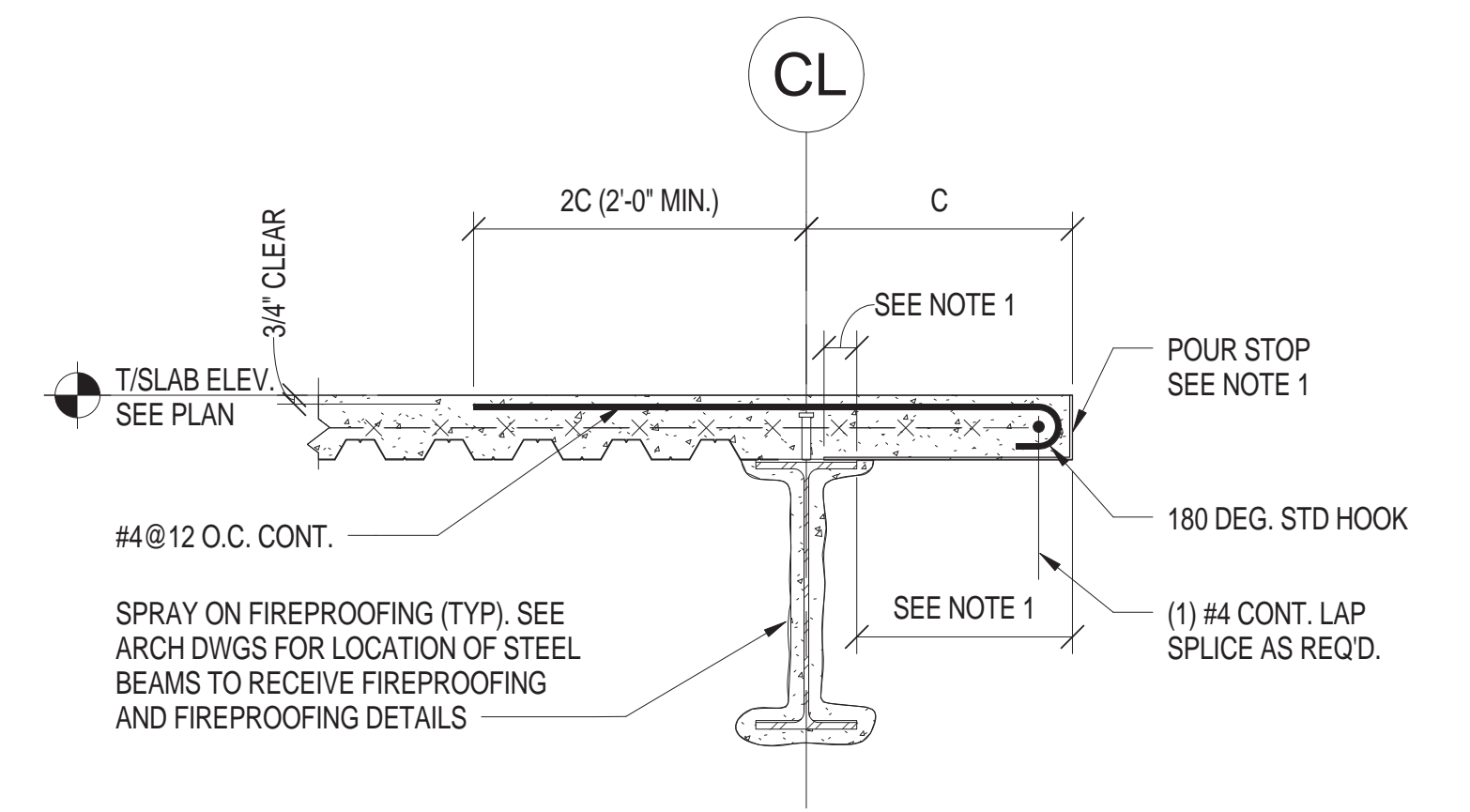
- NOTES:**
- ALL CONCRETE FILL PLACED IN COMPOSITE METAL DECK FLOOR SYSTEMS SHALL BE LIGHTWEIGHT (115 PCF) AND SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH = 3000 PSI.
 - COMPOSITE METAL DECK SHALL BE 18 GAGE MINIMUM LOK-FLOOR BY UNITED STEEL DECK OR APPROVED EQUAL. SEE METAL DECK NOTES ON S-001 FOR DECK PROPERTIES.
 - METAL ROOF DECK SHALL BE 18 GAGE MINIMUM TYPE N DEEP RIB ROOF DECK BY UNITED STEEL DECK OR APPROVED EQUAL. SEE METAL DECK NOTES ON S-002 FOR DECK PROPERTIES.

1 TYPICAL COMPOSITE FLOOR DECK SECTION AND METAL DECK SCHEDULE
NTS



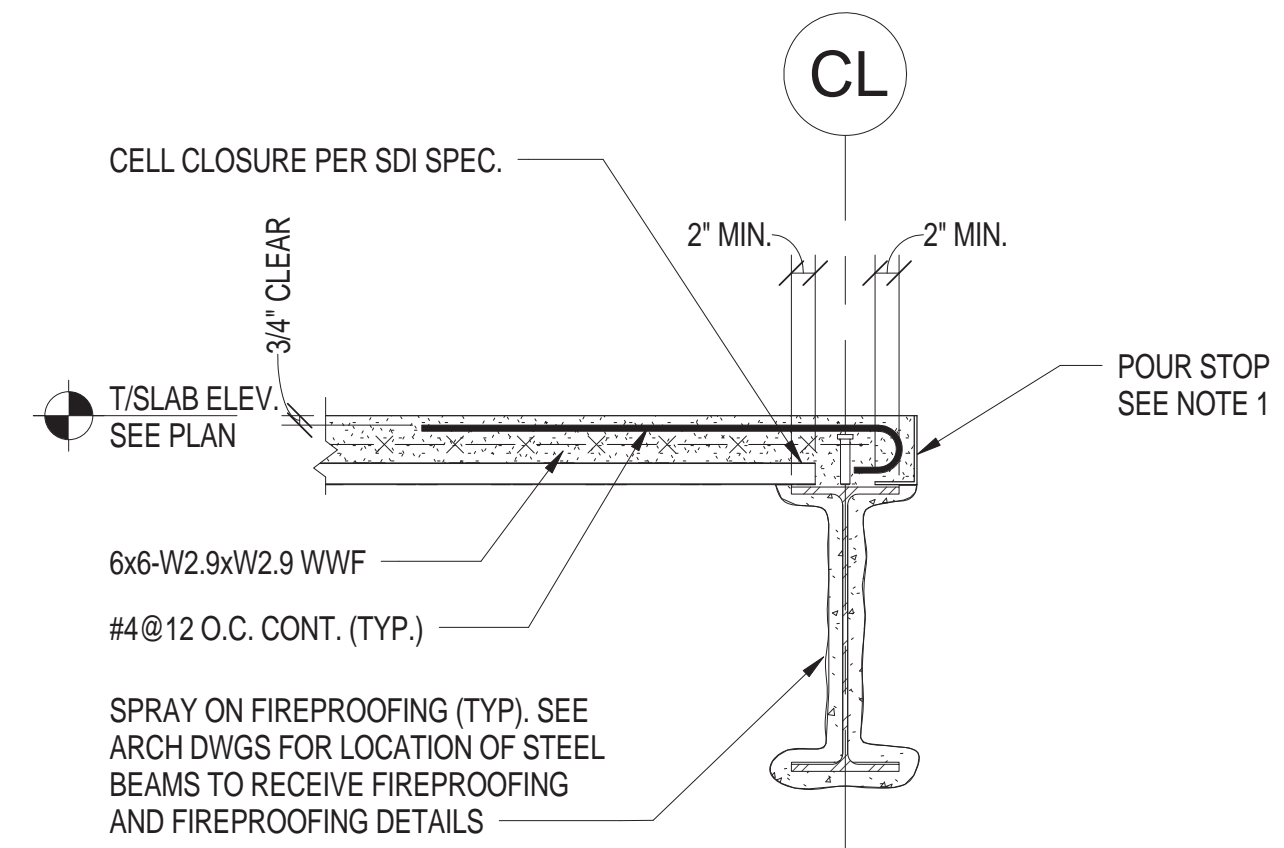
- NOTE:**
- DISTANCE BETWEEN STUDS (PERPENDICULAR TO BEAM OR GIRDER) FOR TWO ROWS SHALL BE 3" MINIMUM.

2 TYPICAL COMPOSITE FLOOR DECK PARALLEL TO BEAM OR GIRDER
NTS



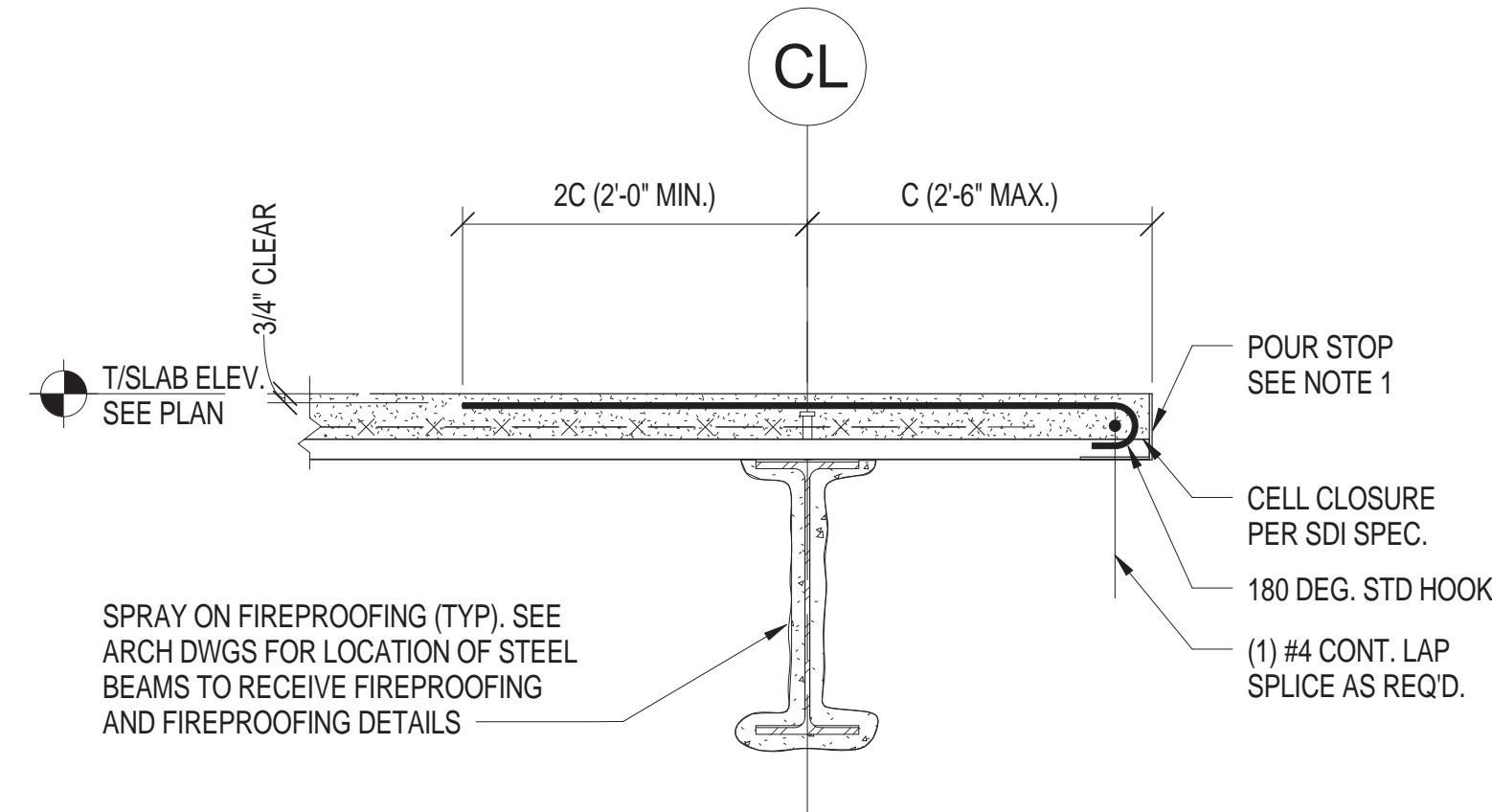
- NOTE:**
- OVERHANG SHALL NOT EXCEED 8". PROVIDE CONTINUOUS POUR STOP PER STEEL DECK INSTITUTE (SDI) SPECIFICATIONS. MINIMUM BEARING OF POUR STOP ON BEAM TOP FLANGE = 2" WITH WELD PER SDI. IF OVERHANG EXCEEDS 10.5" POUR STOP SHALL BE 3/8" THICK BENT PLATE. OVERHANG SHALL NEVER EXCEED 1'-6".

3 TYPICAL COMPOSITE FLOOR DECK PARALLEL EDGE BEAM
NTS



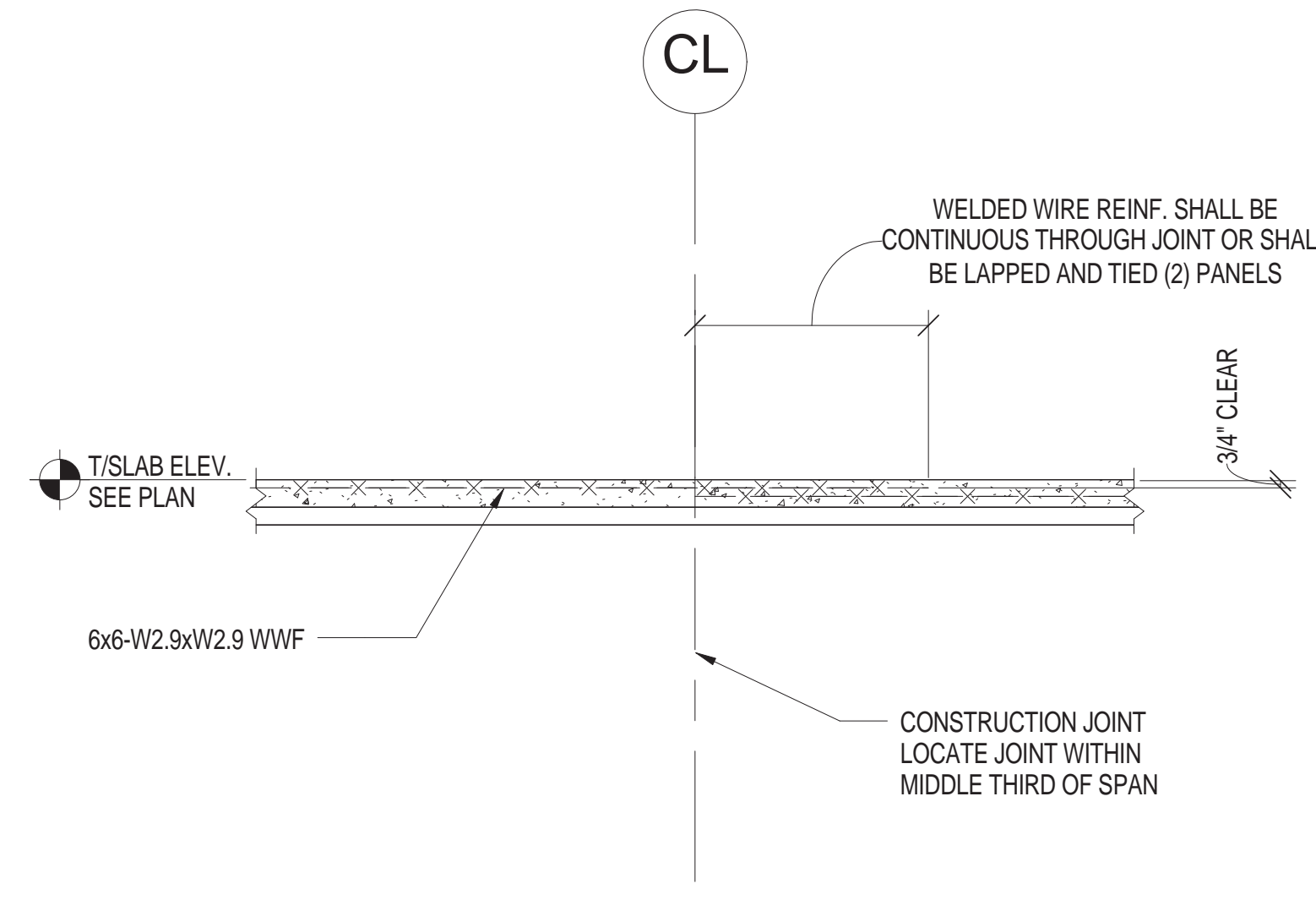
- NOTE:**
- PROVIDE CONTINUOUS POUR STOP PER STEEL DECK INSTITUTE (SDI) SPECIFICATIONS. MINIMUM BEARING OF POUR STOP ON BEAM TOP FLANGE = 2" WITH WELD PER SDI.
 - IF OVERHANG EXCEEDS 1" SEE DETAIL 3 ON THIS SHEET.

4 TYPICAL COMPOSITE FLOOR DECK PERPENDICULAR TO EDGE BEAM
NTS

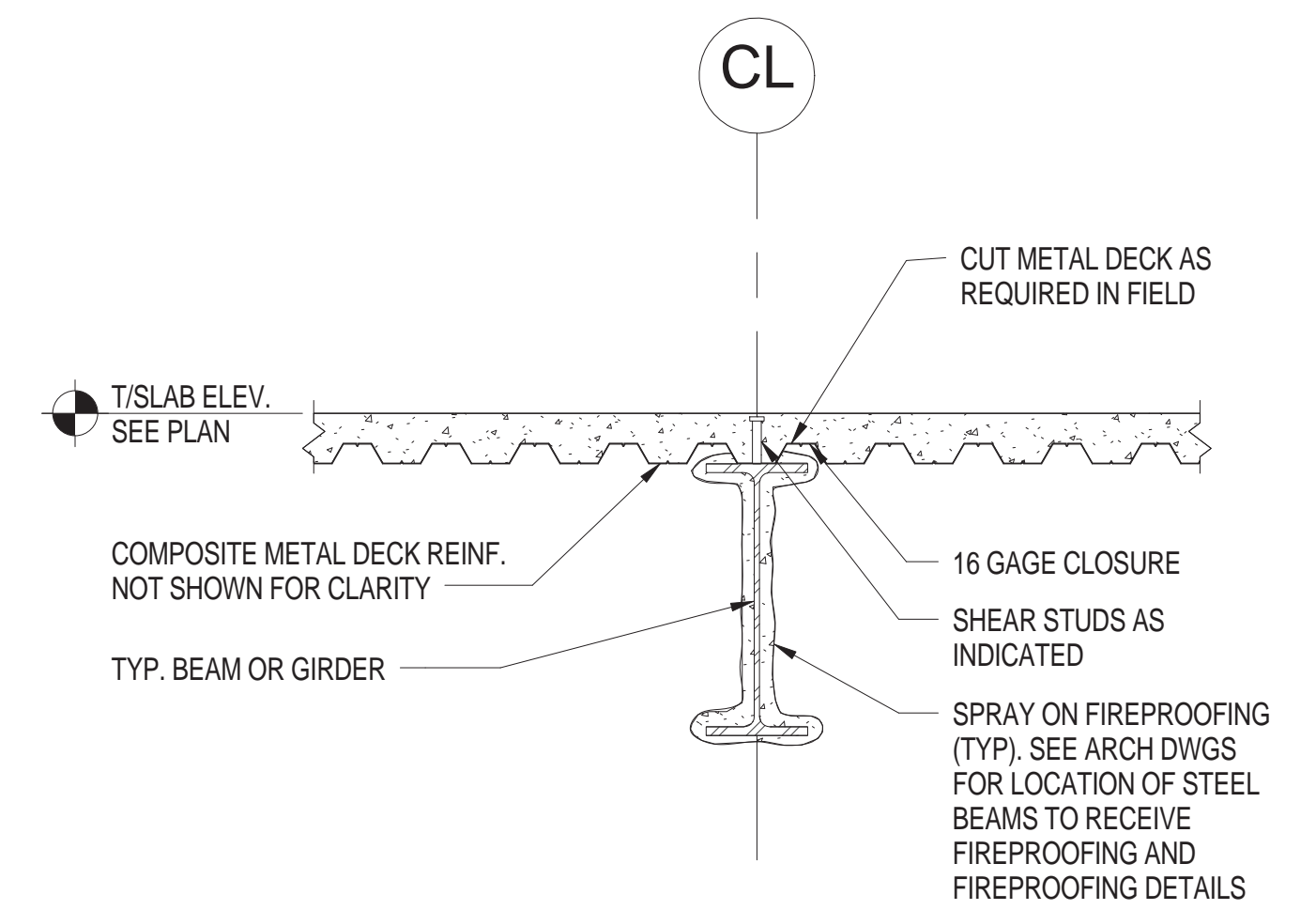


- NOTE:**
- PROVIDE CONTINUOUS POUR PER STEEL DECK INSTITUTE (SDI) SPECIFICATIONS. CONTRACTOR'S OPTION TO STOP DECK @ BEAM AND PROVIDE POUR STOP IN ACCORDANCE WITH 3 & 4 ON THIS SHEET.

5 CANTILEVERED COMPOSITE FLOOR DECK PERPENDICULAR TO EDGE BEAM
NTS

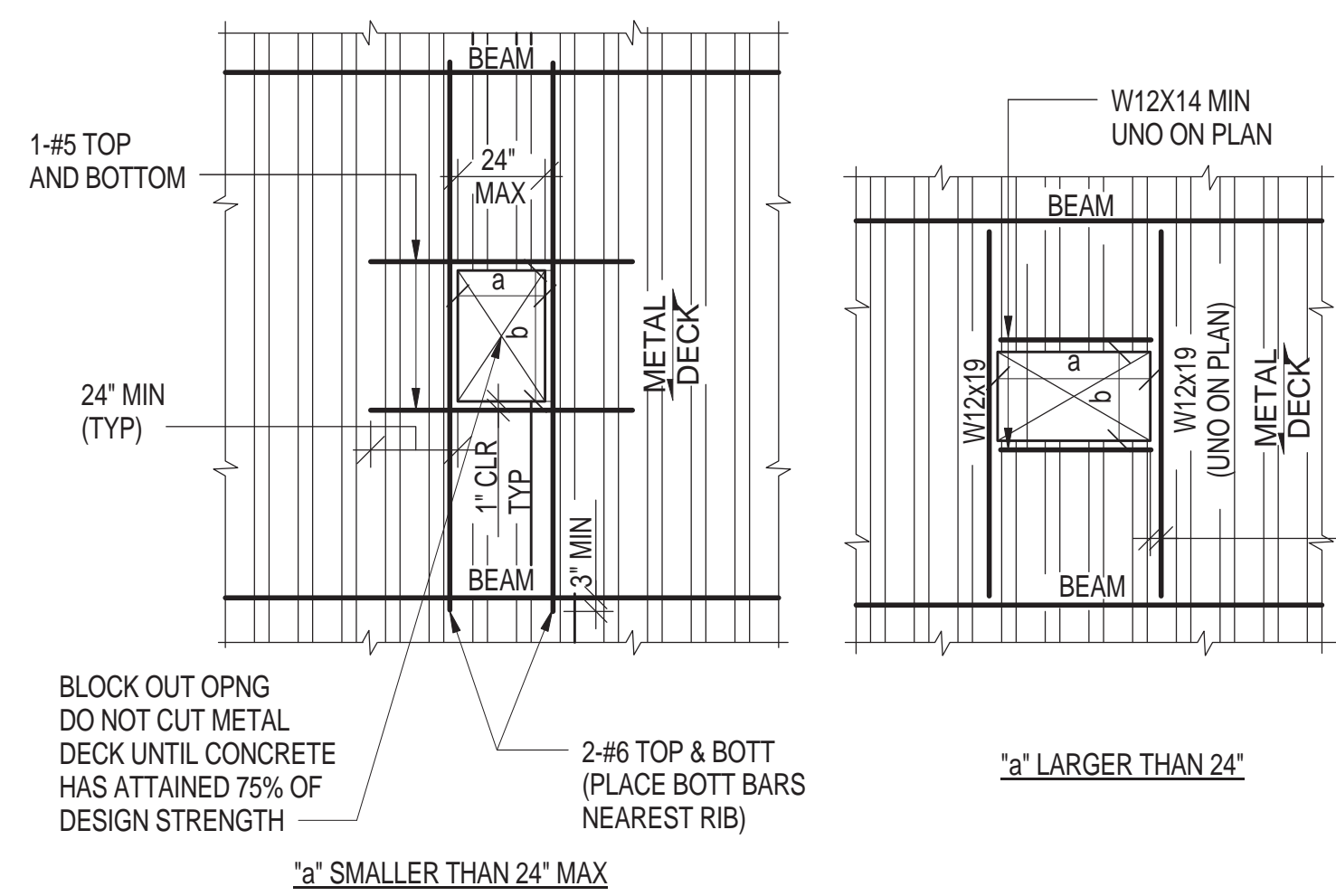


6 TYPICAL COMPOSITE FLOOR DECK SLAB CONSTRUCTION JOINT
NTS

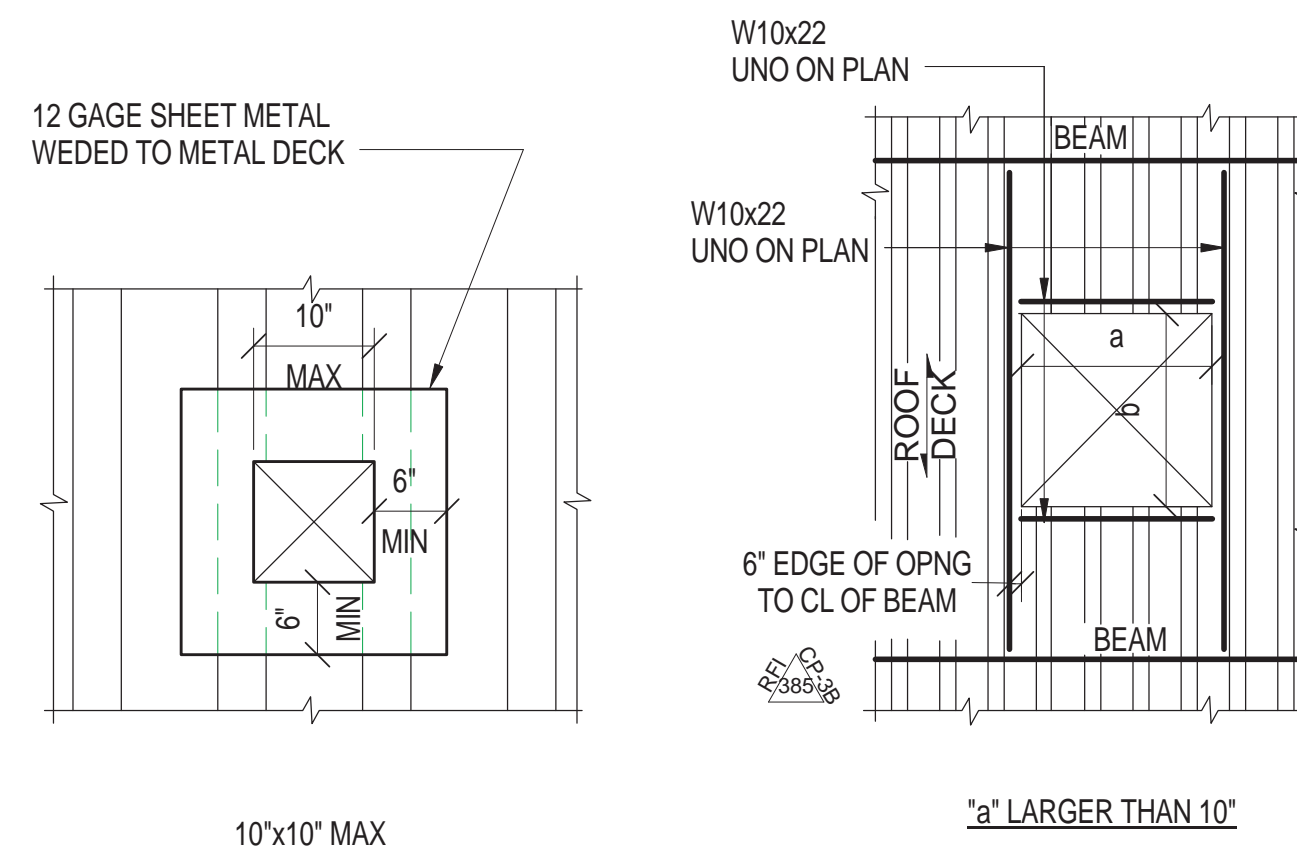


- NOTE:**
- DECK FLUTE MUST BE IN CONTACT WITH BEAM FLANGE TO INSURE THAT FLANGE DOES NOT FALL UNDER HIGH CELL RESULTING IN UNBRACED BEAM

7 TYPICAL COMPOSITE FLOOR DECK PARALLEL TO BEAM
NTS



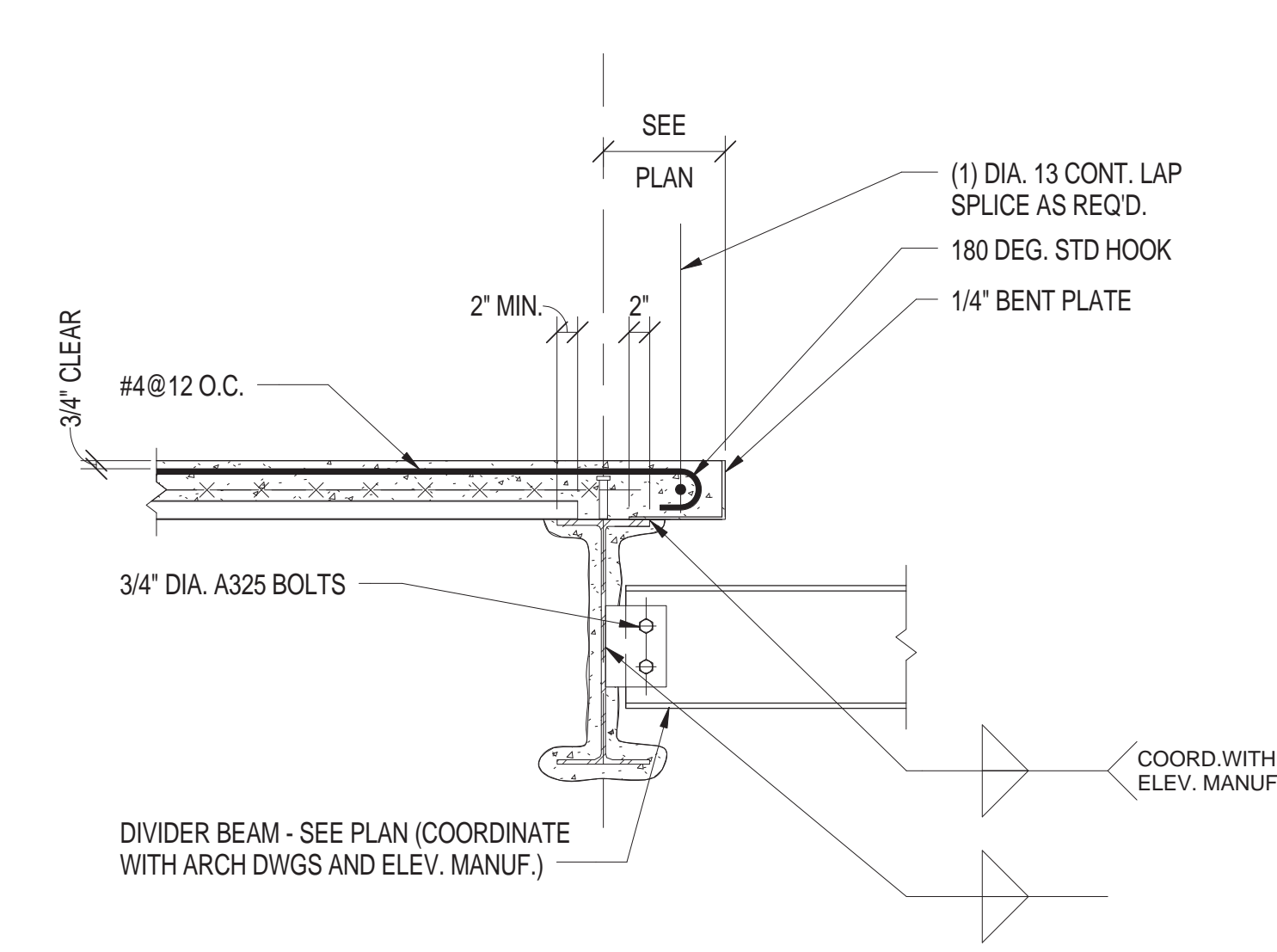
AT FLOOR METAL DECK



AT ROOF DECK

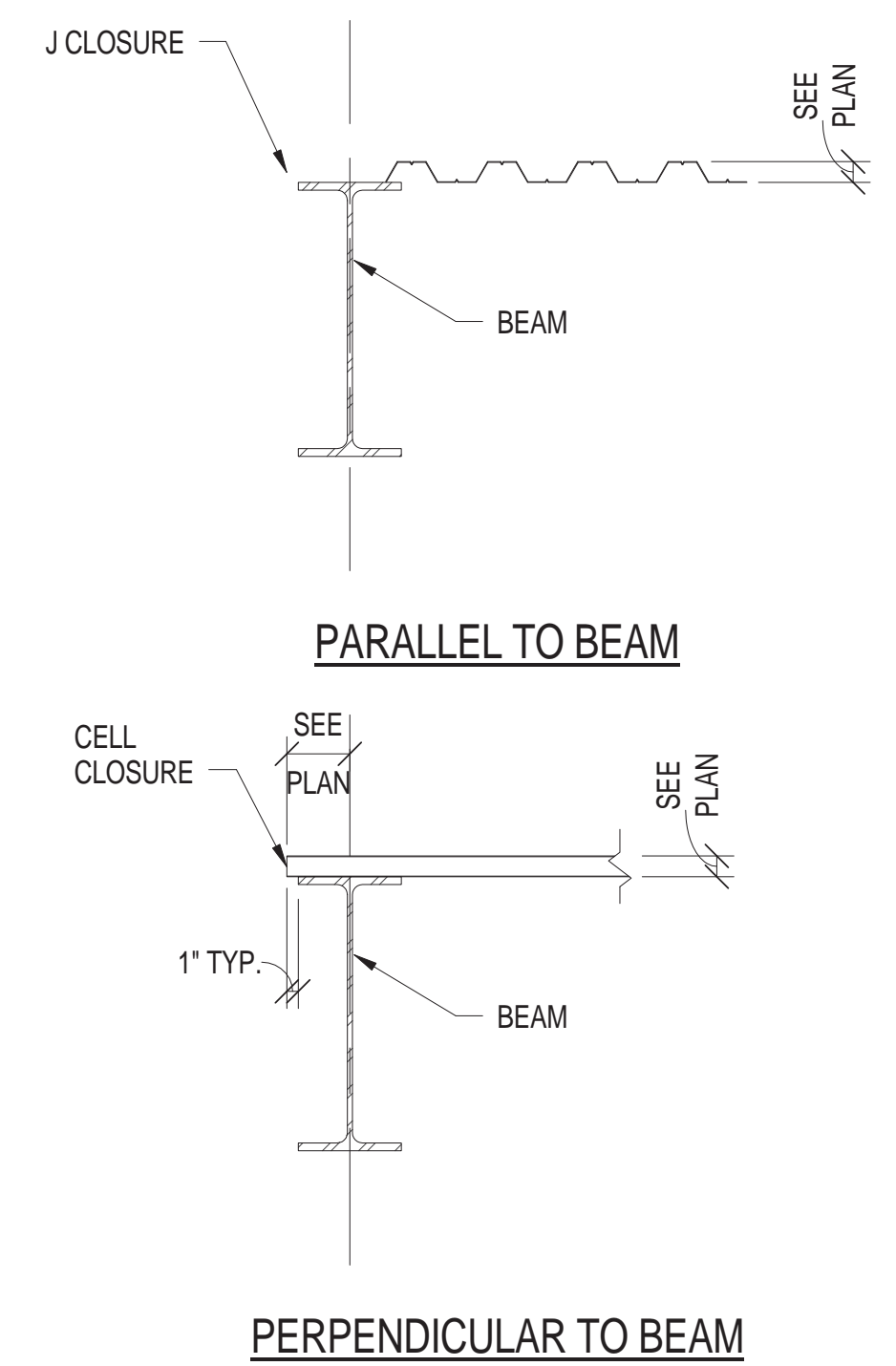
- NOTES:**
- NO REINFORCEMENT IS REQUIRED FOR OPENINGS WITH "a" SMALLER THAN 6".
 - OPENING SHOWN AS "a"x"b" IN PLAN. ("a" MEASURED PERPENDICULAR TO METAL DECK SPAN).

8 TYPICAL METAL DECK OPENING DETAILS WITH OR WITHOUT CONCRETE SLAB
NTS



- NOTES:**
- ALL ADDITIONAL STEEL ANGLES AND DIVIDER BEAMS SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS AND THE ELEVATOR MANUFACTURER.

9 TYPICAL COMPOSITE FLOOR DECK EDGE DETAIL ELEVATOR SILL
NTS



PERPENDICULAR TO BEAM

10 ROOF METAL DECK DETAIL
NTS

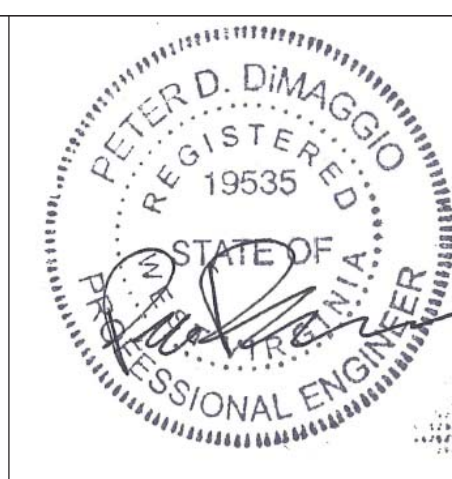
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Revisions	Date



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Drawing Title
TYPICAL DETAILS II

Building No. 20
Location Huntington, WV

Project Title
Renovations to the Former BRAC Property

Project Architect: PF&A
Checked by: MP
Drawn by: VC
Architects Proj. No. 2099.11

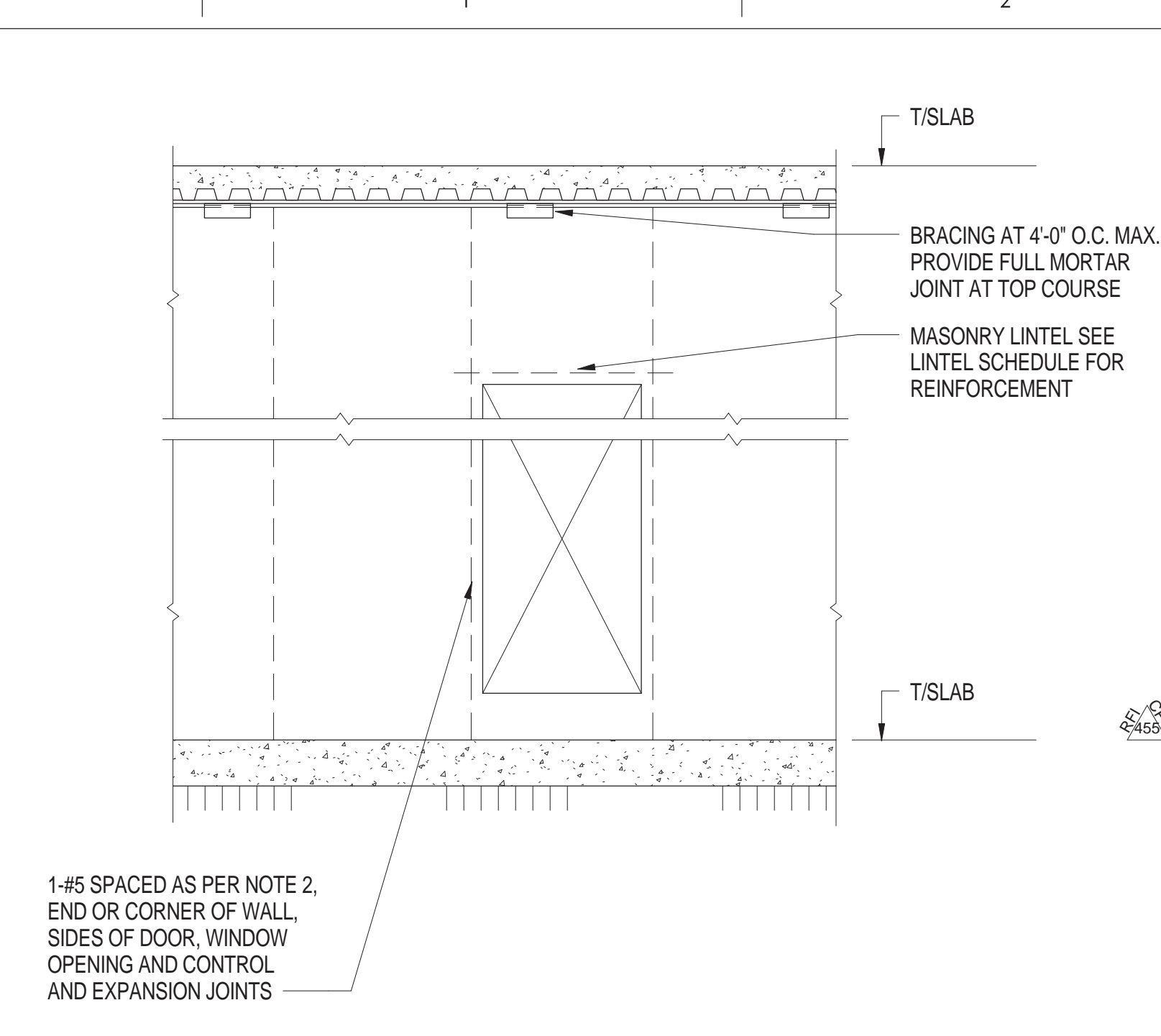
Date
09.03.2013

Project No.
581-12-101

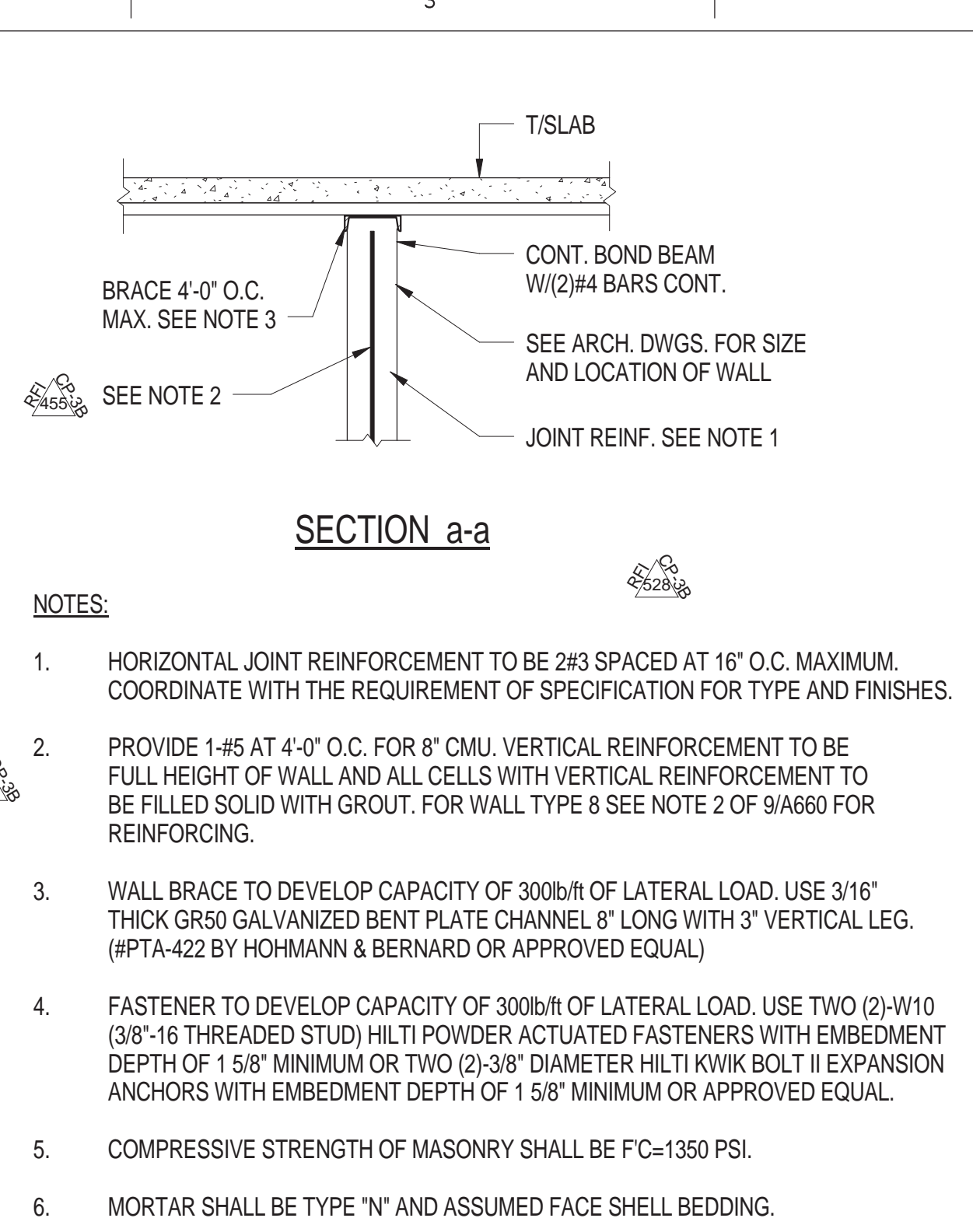
S2.02
SHEET 025 OF 115



three inches = one foot
 one and one-half inches = one foot
 one inch = one foot
 three-quarters inch = one foot
 one-half inch = one foot
 three-eighths inch = one foot
 one-quarter inch = one foot
 one-eighth inch = one foot



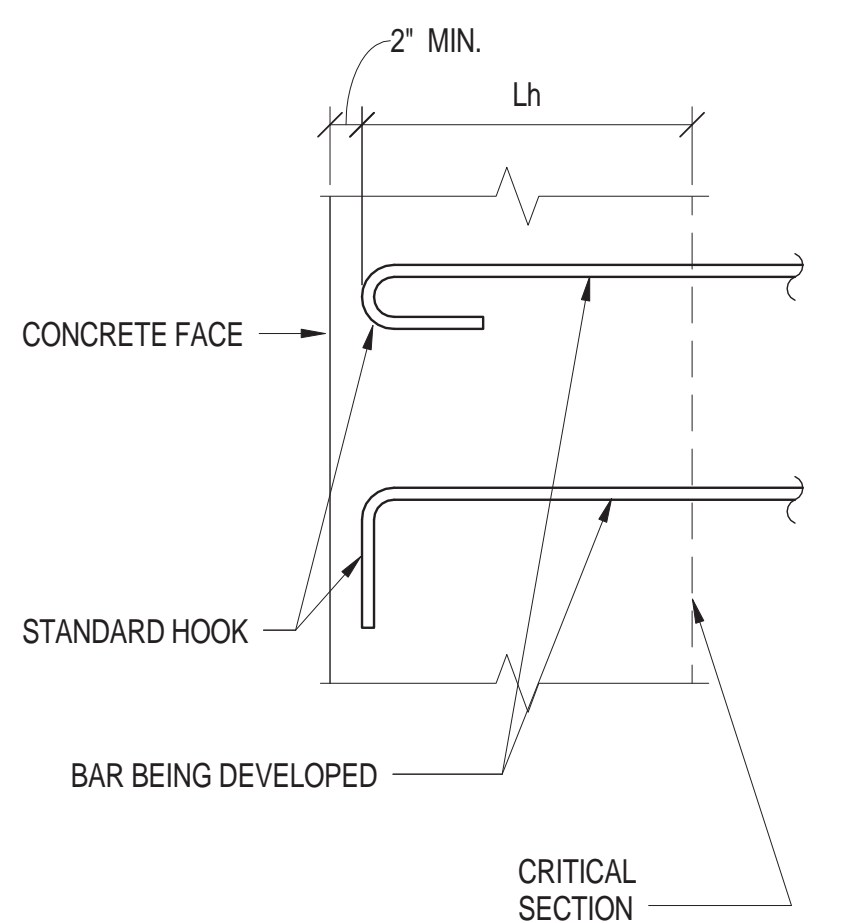
WALL ELEVATION



1 CMU NON LOAD-BEARING WALL REINFORCING DETAIL
 NTS

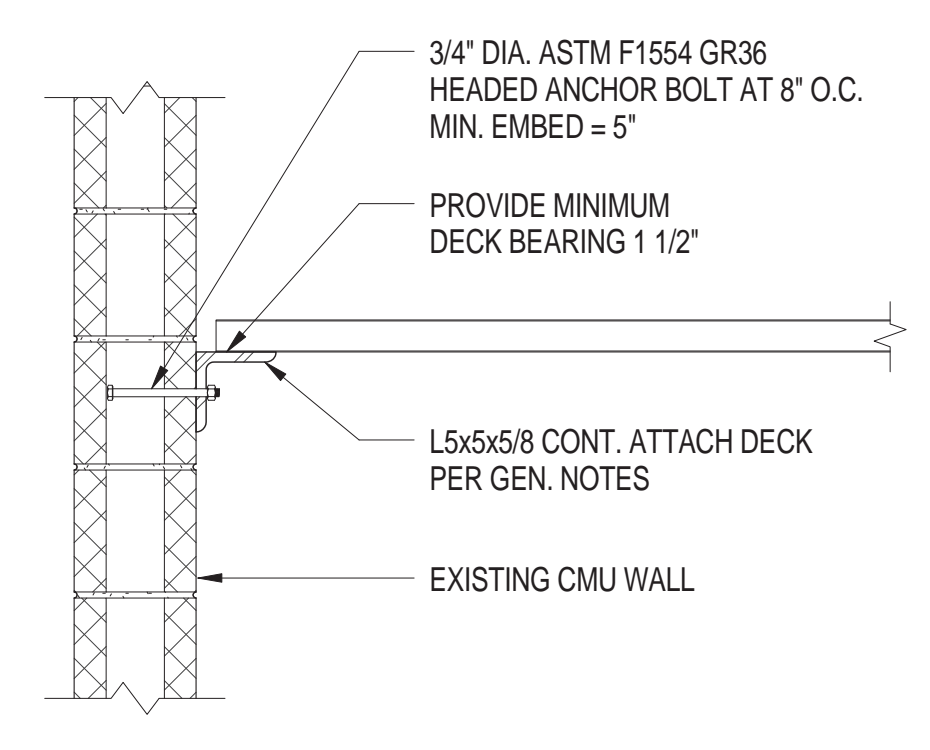
HOOKED BAR DEVELOPMENT LENGTHS, L_h

BAR	f _c	4000 PSI	5000 PSI
#3	0'-8"	0'-7"	
#4	0'-10"	0'-9"	
#5	1'-0"	0'-11"	
#6	1'-3"	1'-1"	
#7	1'-5"	1'-3"	
#8	1'-7"	1'-5"	
#9	1'-10"	1'-8"	
#10	2'-1"	1'-10"	
#11	2'-3"	2'-0"	

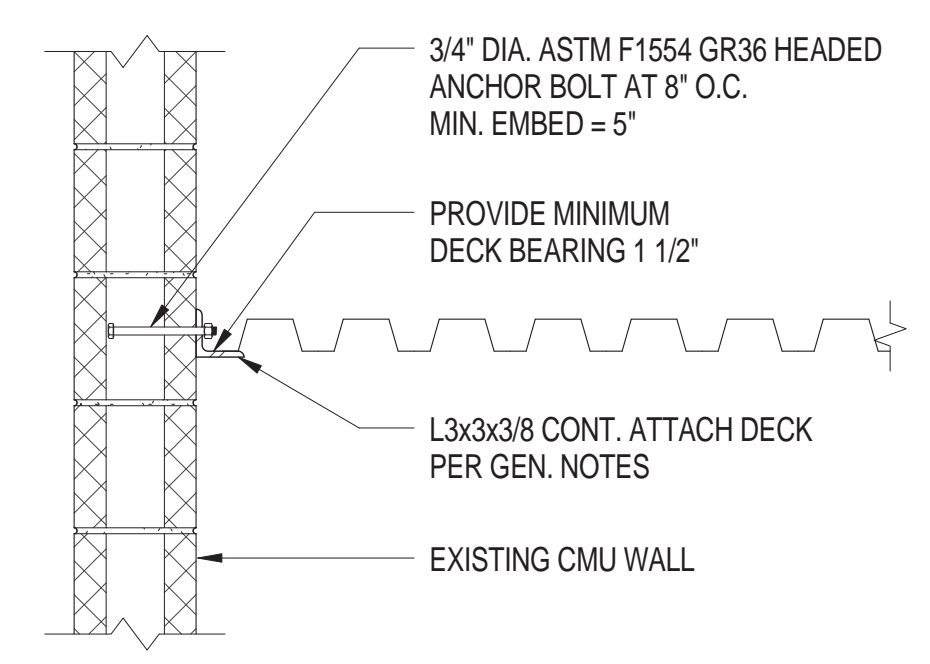


NOTES:
 1. LENGTHS APPLY TO GRADE 60 REBAR IN NORMAL WEIGHT CONCRETE
 2. VALUES INDICATED DO NOT INCLUDE 0.7 FACTOR FOR 2" MIN END COVER INDICATED IN SKETCH

4 HOOKED BAR DEVELOPMENT LENGTHS
 NTS



DECK SPAN PERPENDICULAR TO WALL



DECK SPAN PARALLEL TO WALL

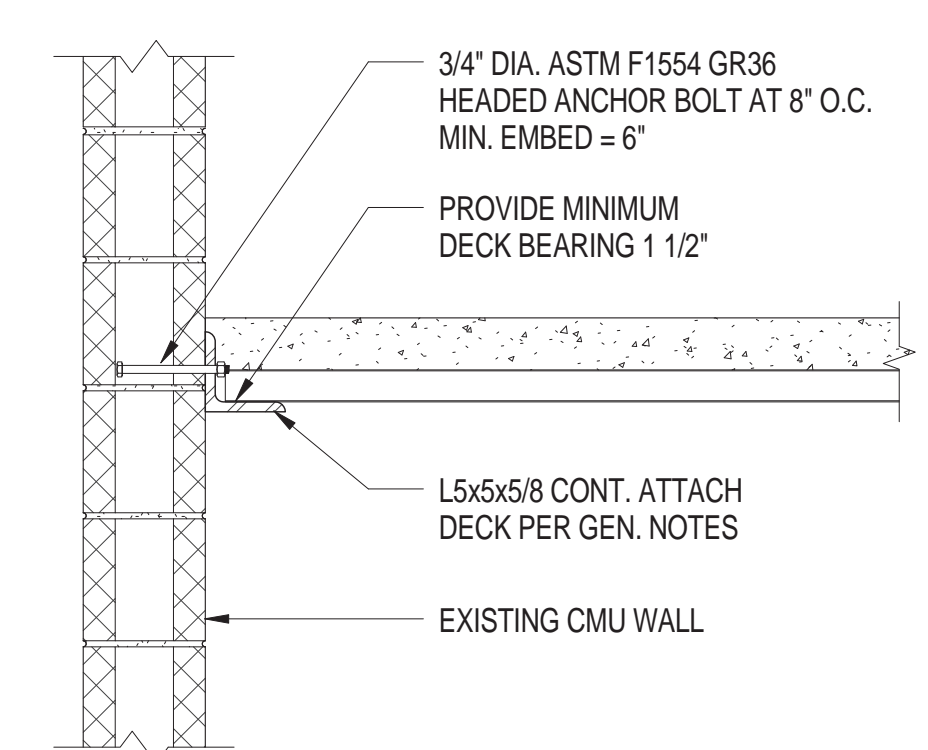
7 TYPICAL METAL DECK SUPPORT DETAIL AT CMU WALL
 NTS

STANDARD HOOK DIMENSIONS

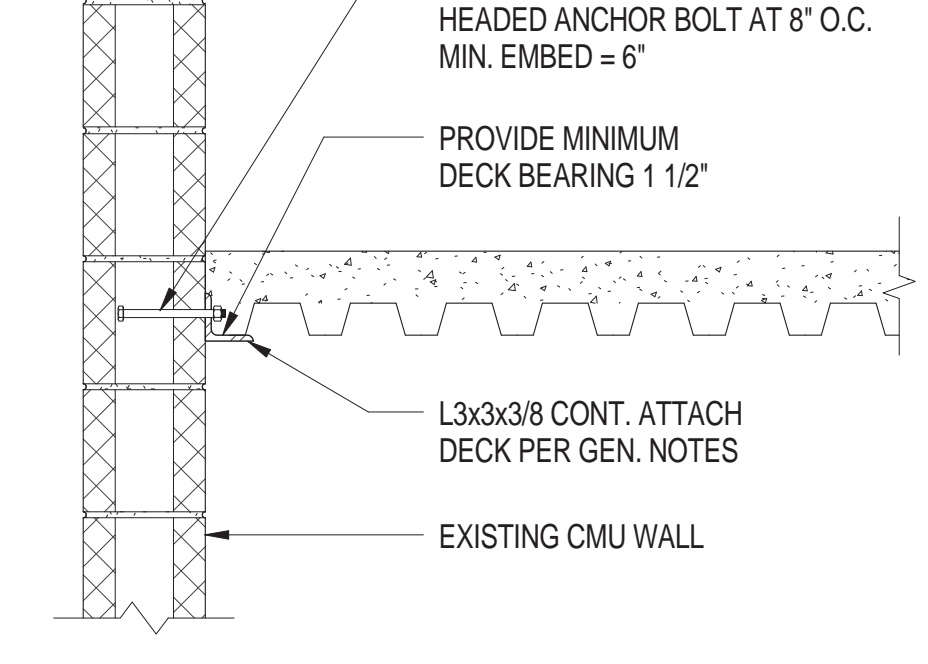
DIM.	D	90 DEG HOOK, A	180 DEG HOOK, A	180 DEG HOOK, J
#3	0'-2 1/4"	0'-6"	0'-5"	0'-3"
#4	0'-3"	0'-8"	0'-6"	0'-4"
#5	0'-3 3/4"	0'-10"	0'-7"	0'-5"
#6	0'-4 1/2"	1'-0"	0'-8"	0'-6"
#7	0'-5 1/4"	1'-2"	0'-10"	0'-7"
#8	0'-6"	1'-4"	0'-11"	0'-8"
#9	0'-9 1/2"	1'-7"	1'-3"	0'-11 3/4"
#10	0'-10 3/4"	1'-10"	1'-5"	1'-1 1/4"
#11	1'-0"	2'-0"	1'-7"	1'-2 3/4"

NOTES:
 1. J DIMENSION IS APPROXIMATE
 2. D IS BEND DIAMETER
 3. d IS BAR DIAMETER

5 STANDARD HOOK DIMENSIONS
 NTS

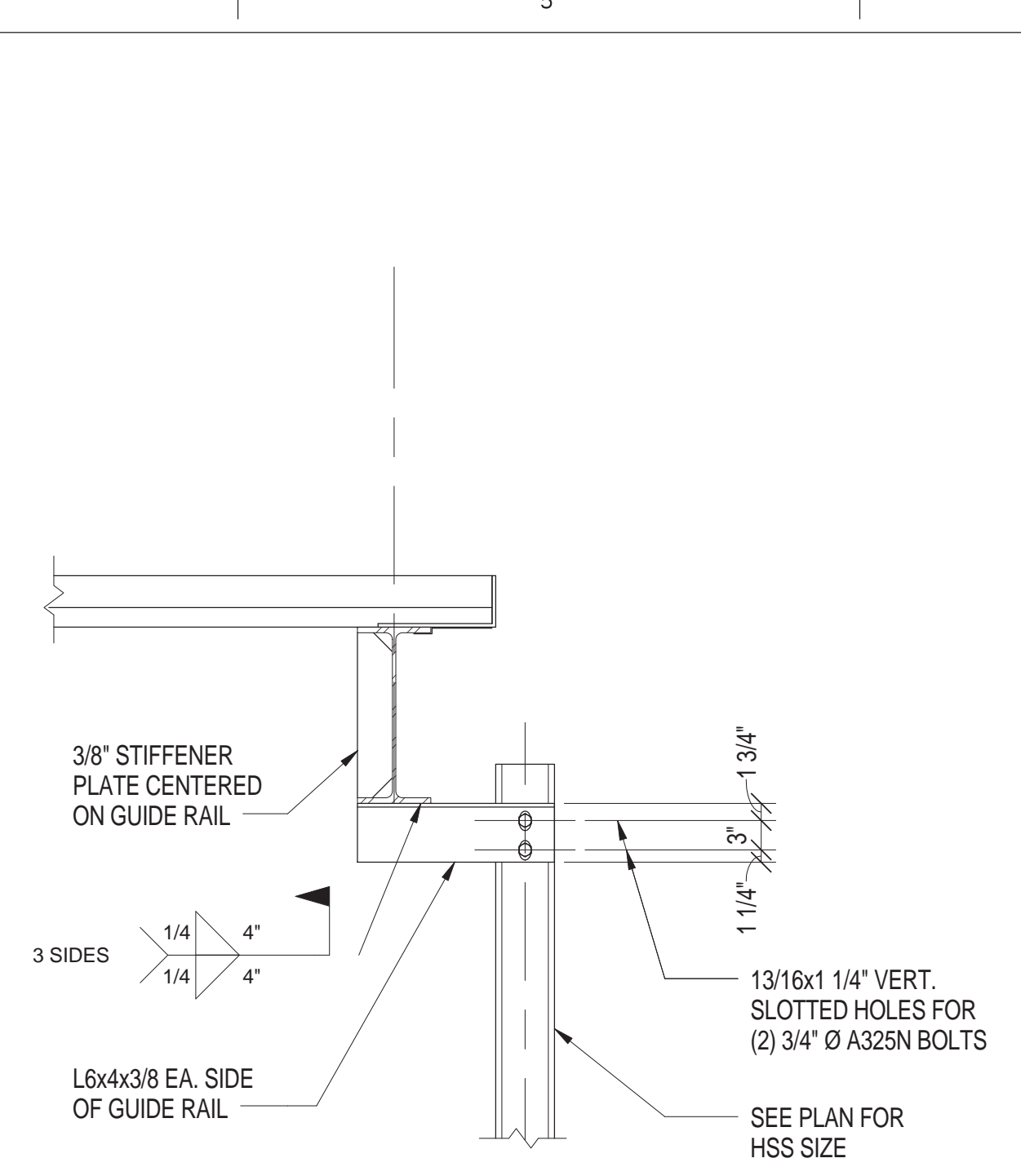


DECK SPAN PERPENDICULAR TO WALL

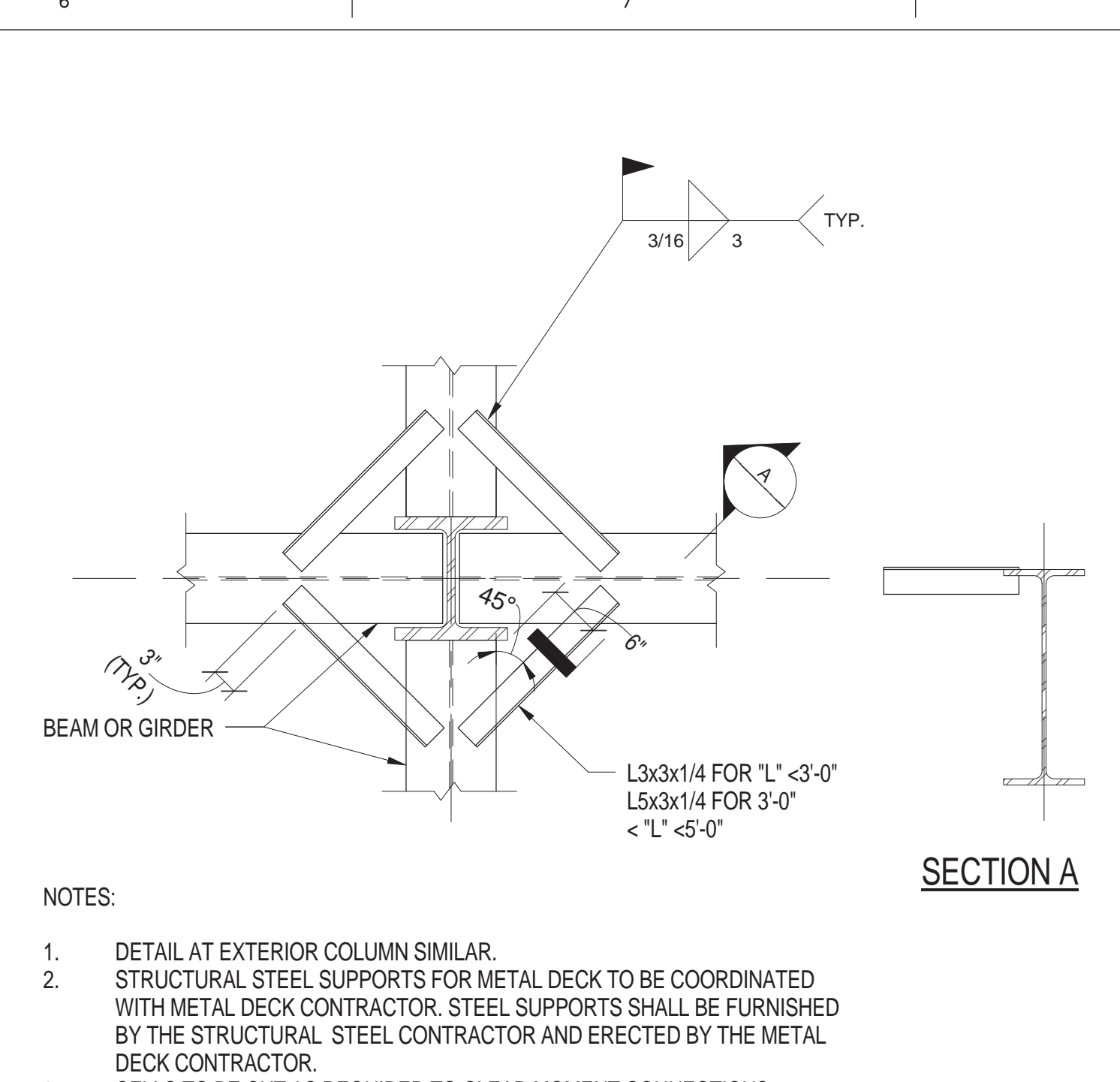


DECK SPAN PARALLEL TO WALL

8 TYPICAL COMPOSITE SLAB SUPPORT DETAIL AT CMU WALL
 NTS

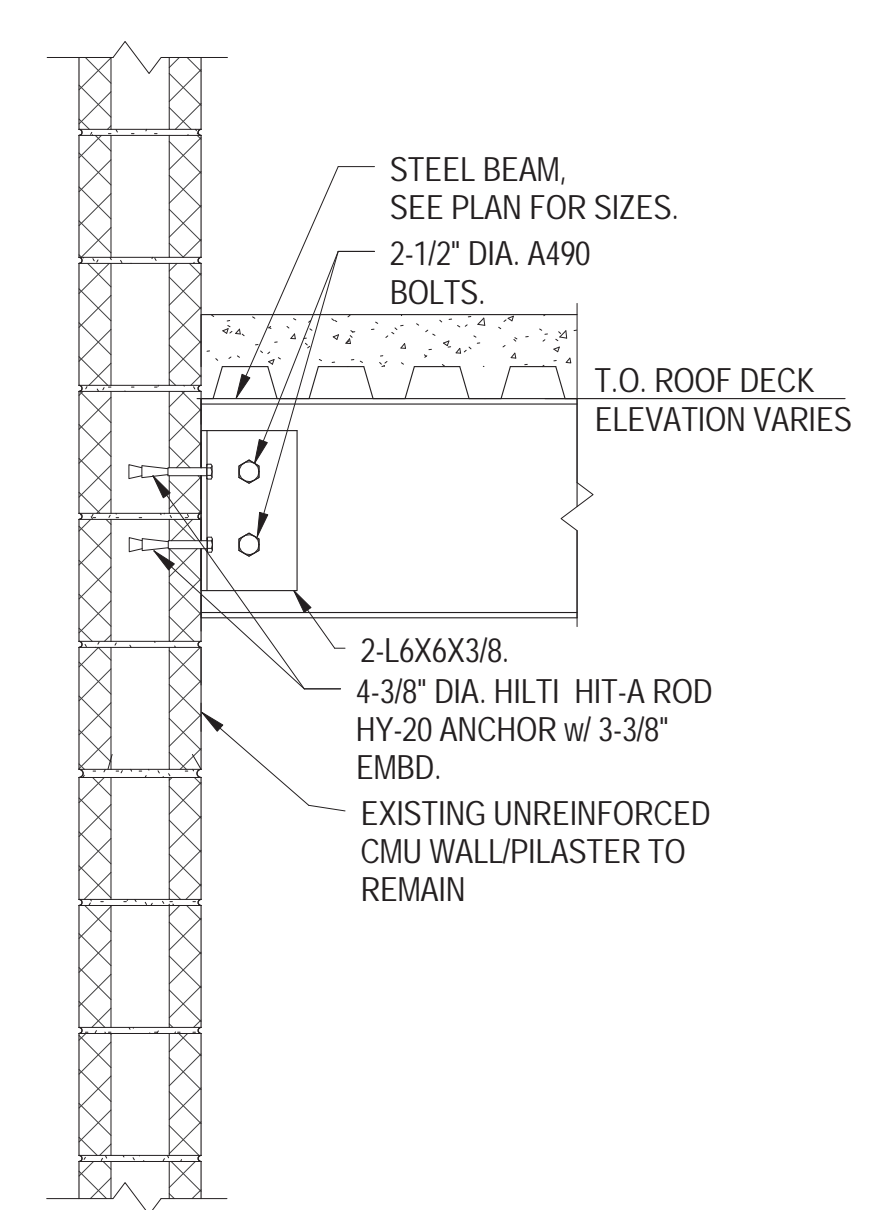


2 ELEVATOR GUIDE RAIL SUPPORT DETAIL
 NTS

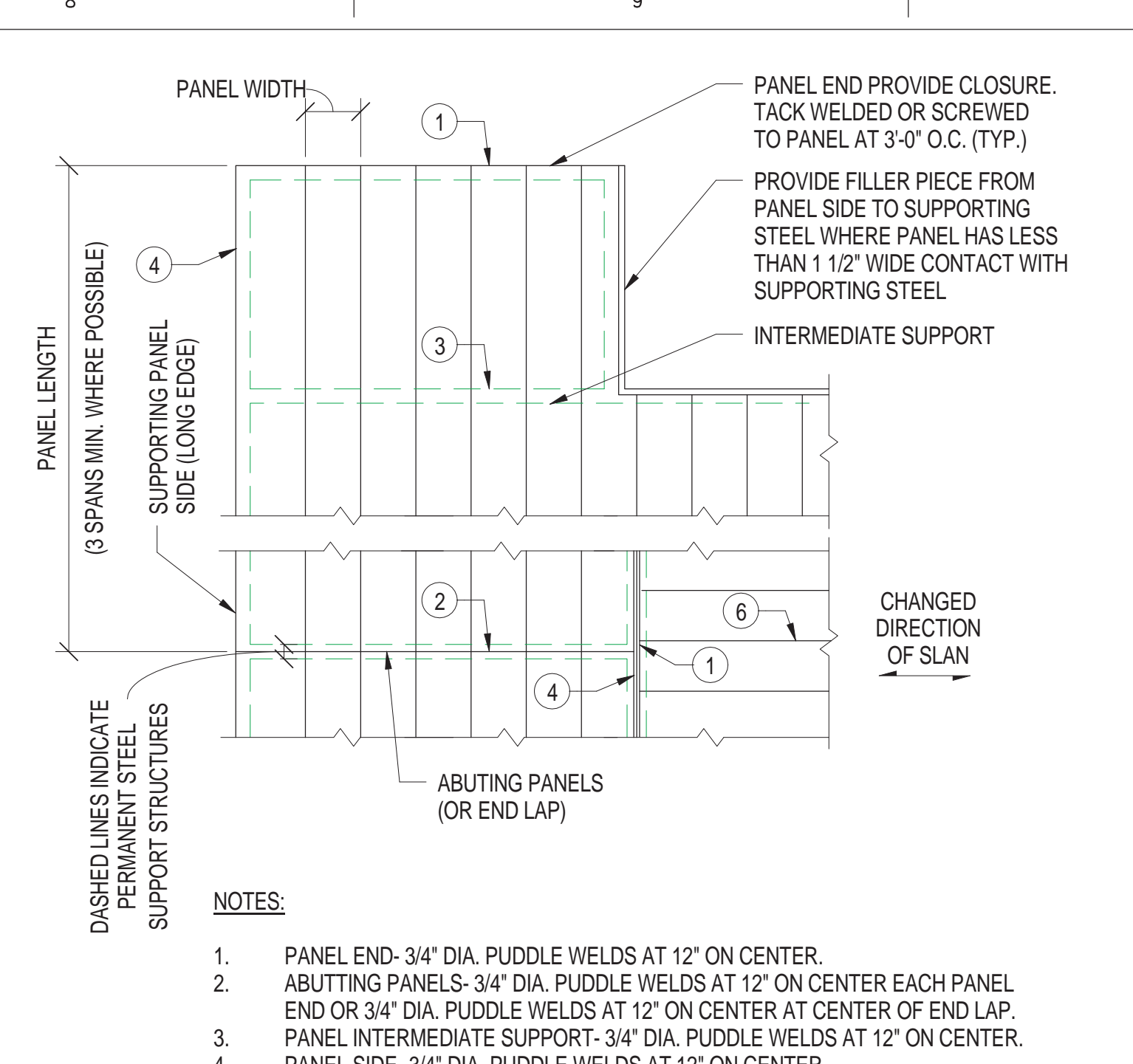


NOTES:
 1. DETAIL AT EXTERIOR COLUMN SIMILAR.
 2. STRUCTURAL STEEL SUPPORTS FOR METAL DECK TO BE COORDINATED WITH METAL DECK CONTRACTOR. STEEL SUPPORTS SHALL BE FURNISHED BY THE STRUCTURAL STEEL CONTRACTOR AND ERECTED BY THE METAL DECK CONTRACTOR.
 3. CELLS TO BE CUT AS REQUIRED TO CLEAR MOMENT CONNECTIONS, VERTICAL PIPING, CONDUIT, AND DUCT WORK ETC. AS REQUIRED BY OTHER TRADES.
 4. PROVIDE SUPPORT WHERE MORE THAN TWO CELLS ARE UNSUPPORTED.

3 METAL DECK SUPPORT DETAIL AT COLUMN
 NTS

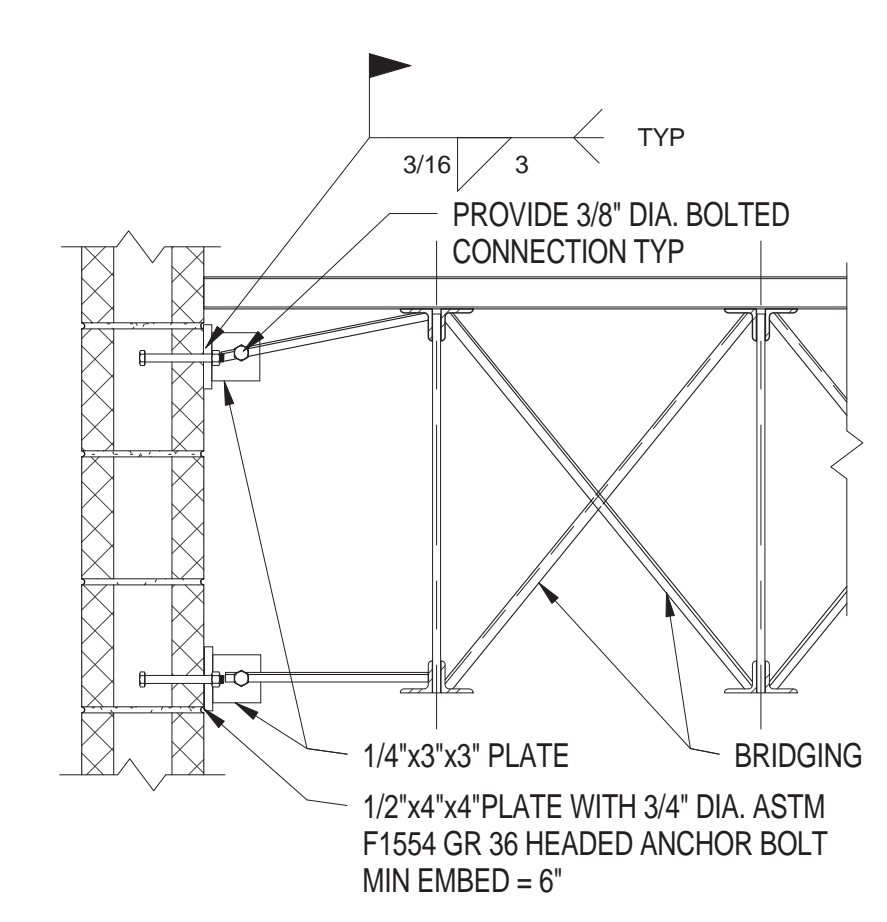


6 TYPICAL BEAM SEAT DETAIL AT CMU WALL
 NTS



NOTES:
 1. PANEL END- 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER.
 2. ABUTTING PANELS- 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER EACH PANEL END OR 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER AT CENTER OF END LAP.
 3. PANEL INTERMEDIATE SUPPORT- 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER.
 4. PANEL SIDE- 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER.
 5. PANEL SIDE WITH FILLER PIECE- 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER FILLER TO PANEL AND FILLER TO SUPPORTING STEEL.
 6. PANEL SIDE LAP- 1" LONG SEAM WELDS AT 12" ON CENTER.

9 TYPICAL METAL DECK WELDING PLAN
 NTS



10 TYPICAL OPEN WEB JOIST DIAGONAL BRIDGING DETAIL AT CMU WALL
 1" = 1'-0"

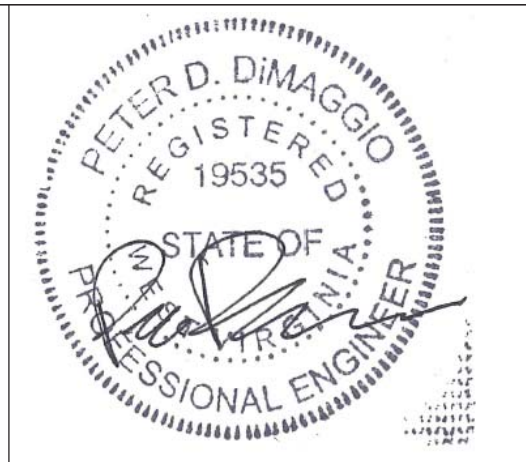
Revisions

Revisions	Date



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 40 Wall Street, NY, NY 10005



Drawing Title
TYPICAL DETAILS III

Building No. 20
 Location Huntington, WV

Project Title
Renovations to the Former BRAC Property

Project Architect: PF&A
 Architects Proj. No. 2099.11

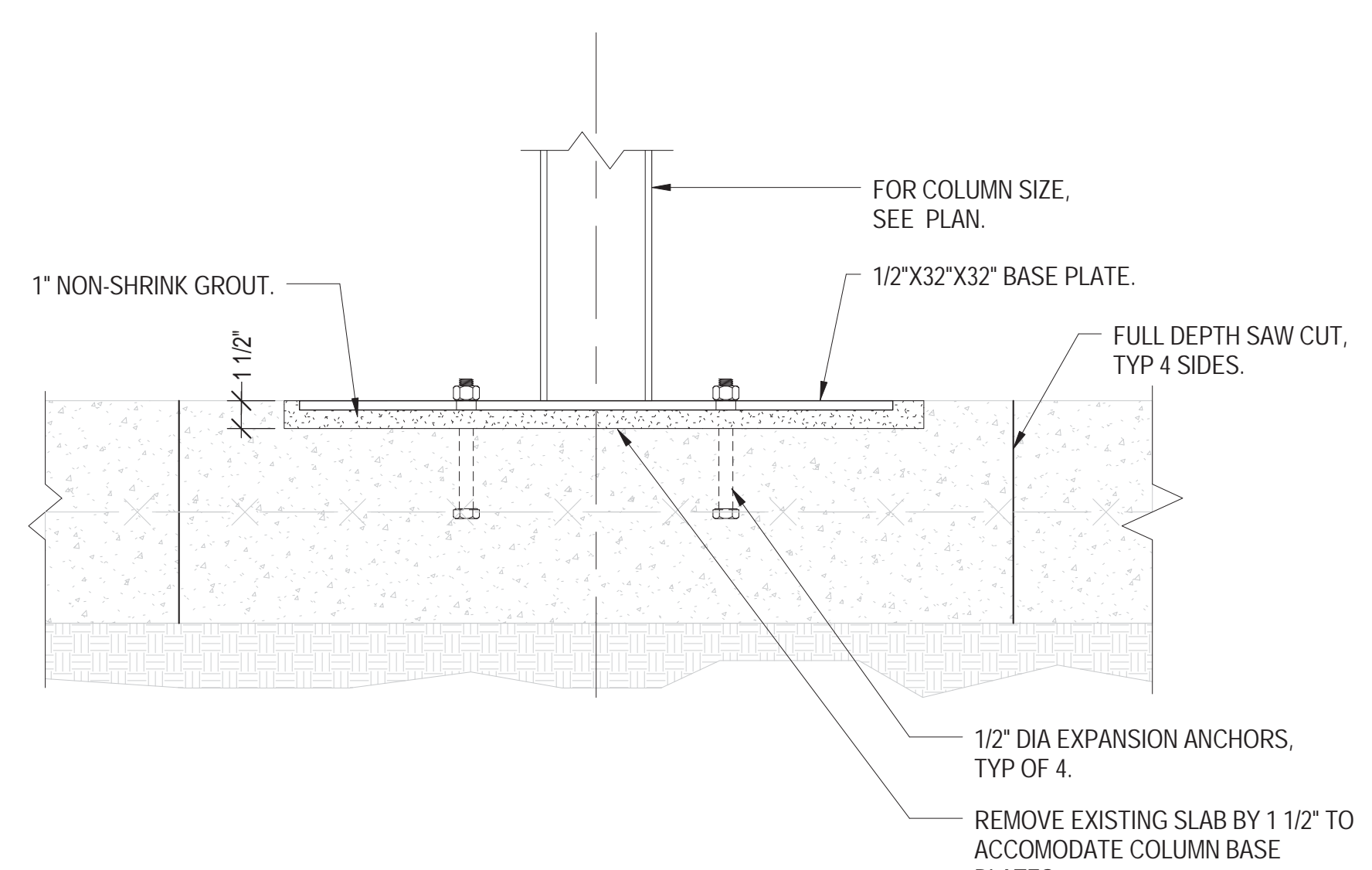
Date 09.03.2013
 Project No. 581-12-101
 S2.03
 SHEET 026 OF 115



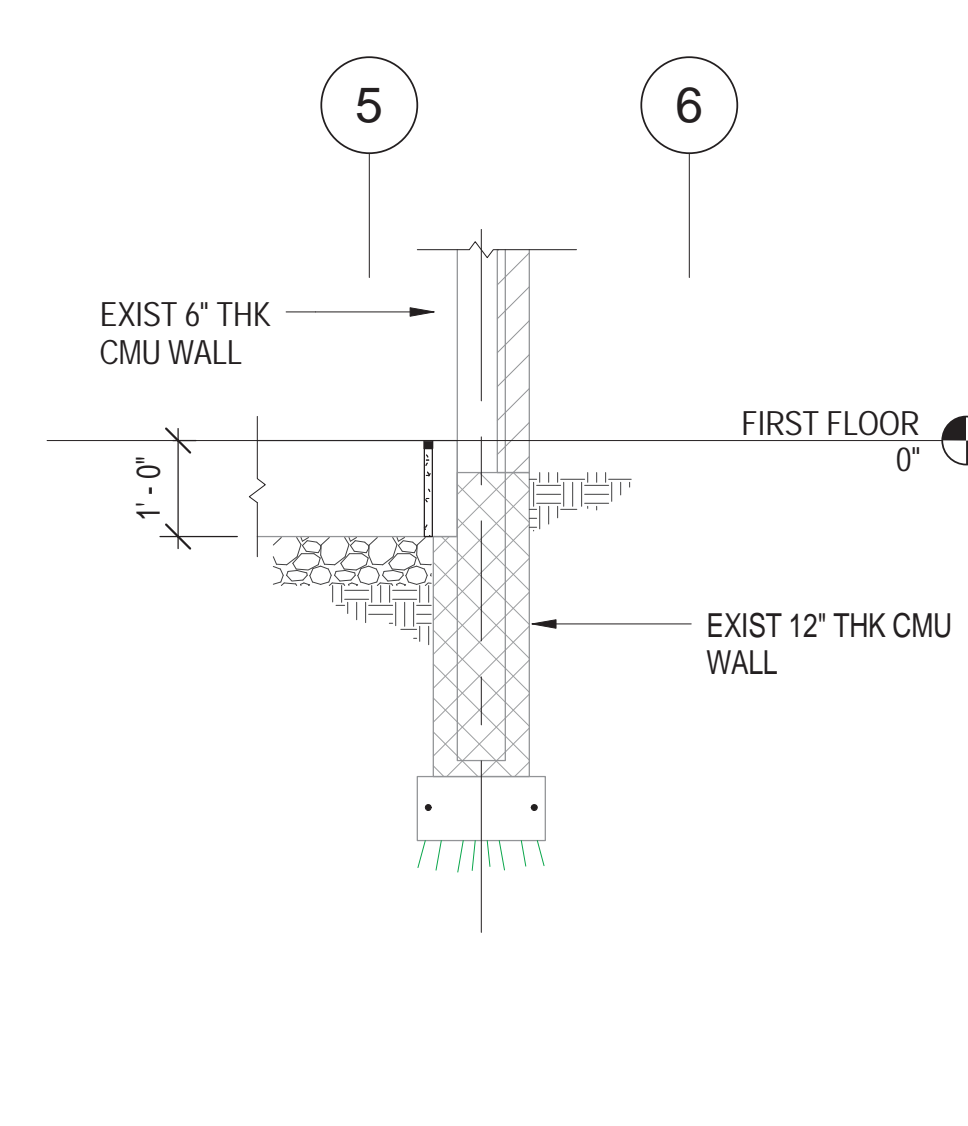
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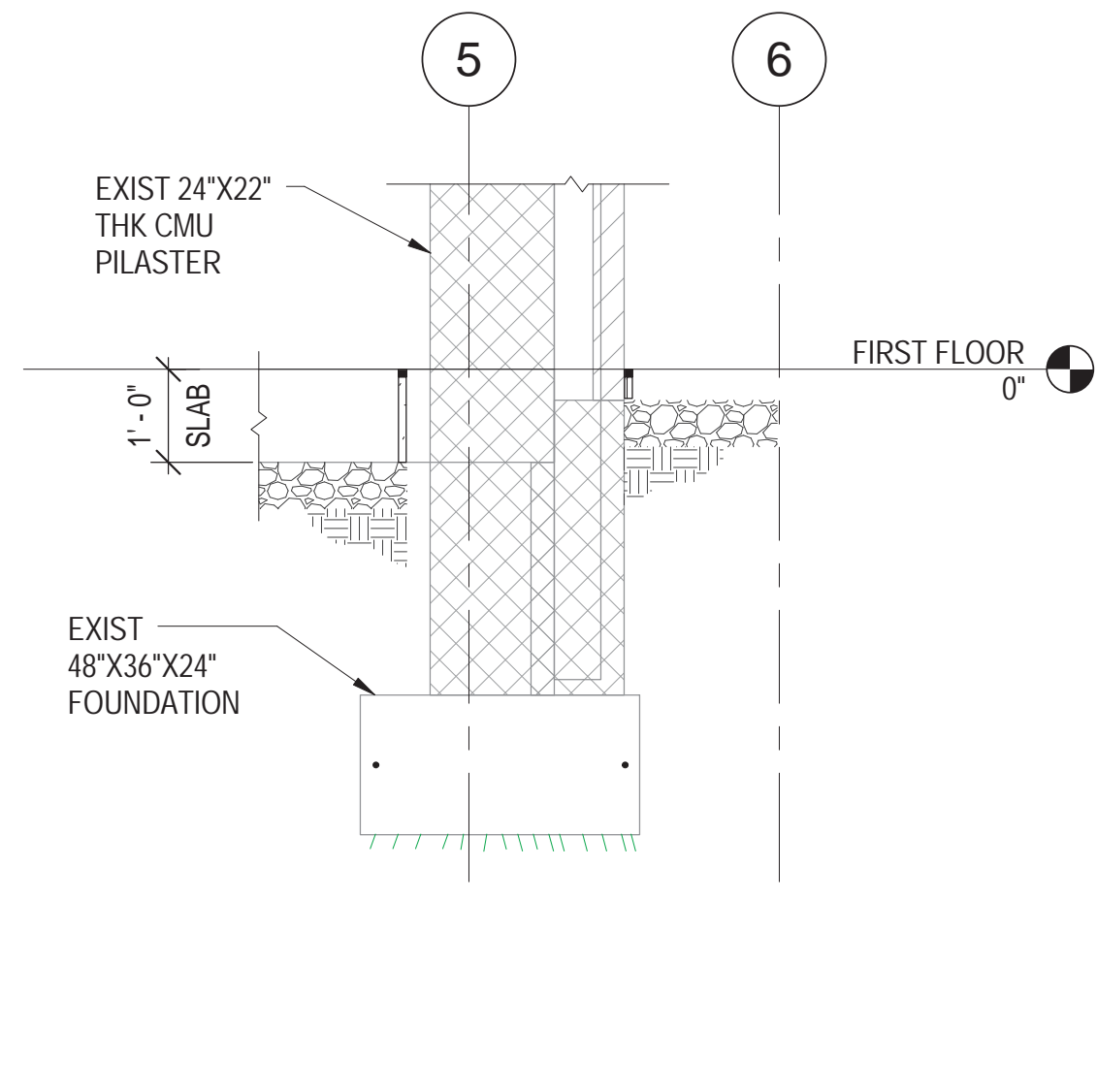
Scale bars for various drawing sections: three inches = one foot, one and one-half inches = one foot, one inch = one foot, three-quarters inch = one foot, one-half inch = one foot, one-quarter inch = one foot, three-eighths inch = one foot, one-eighth inch = one foot.



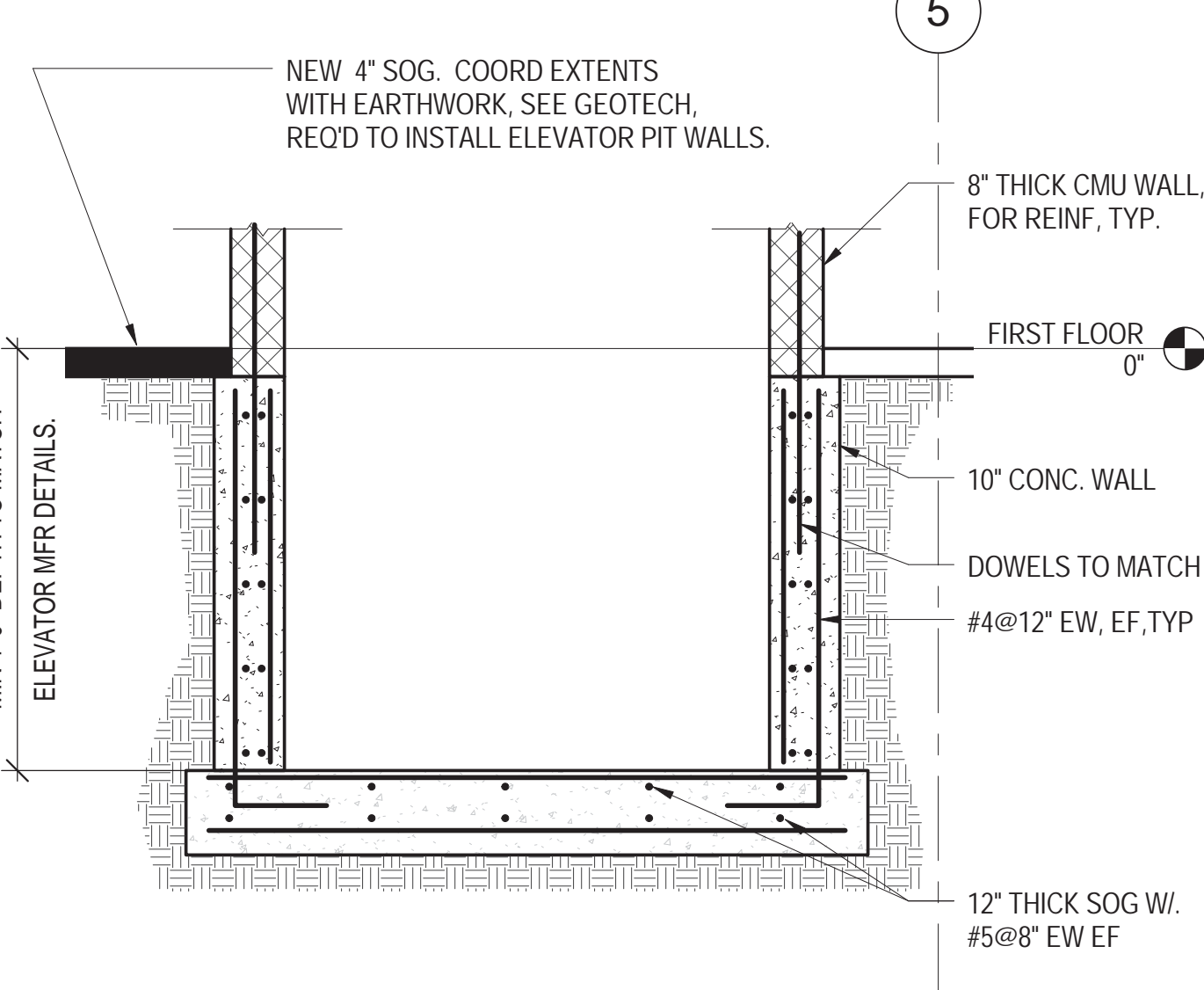
1 SECTION @ GYM COLUMNS
1 1/2" = 1'-0"



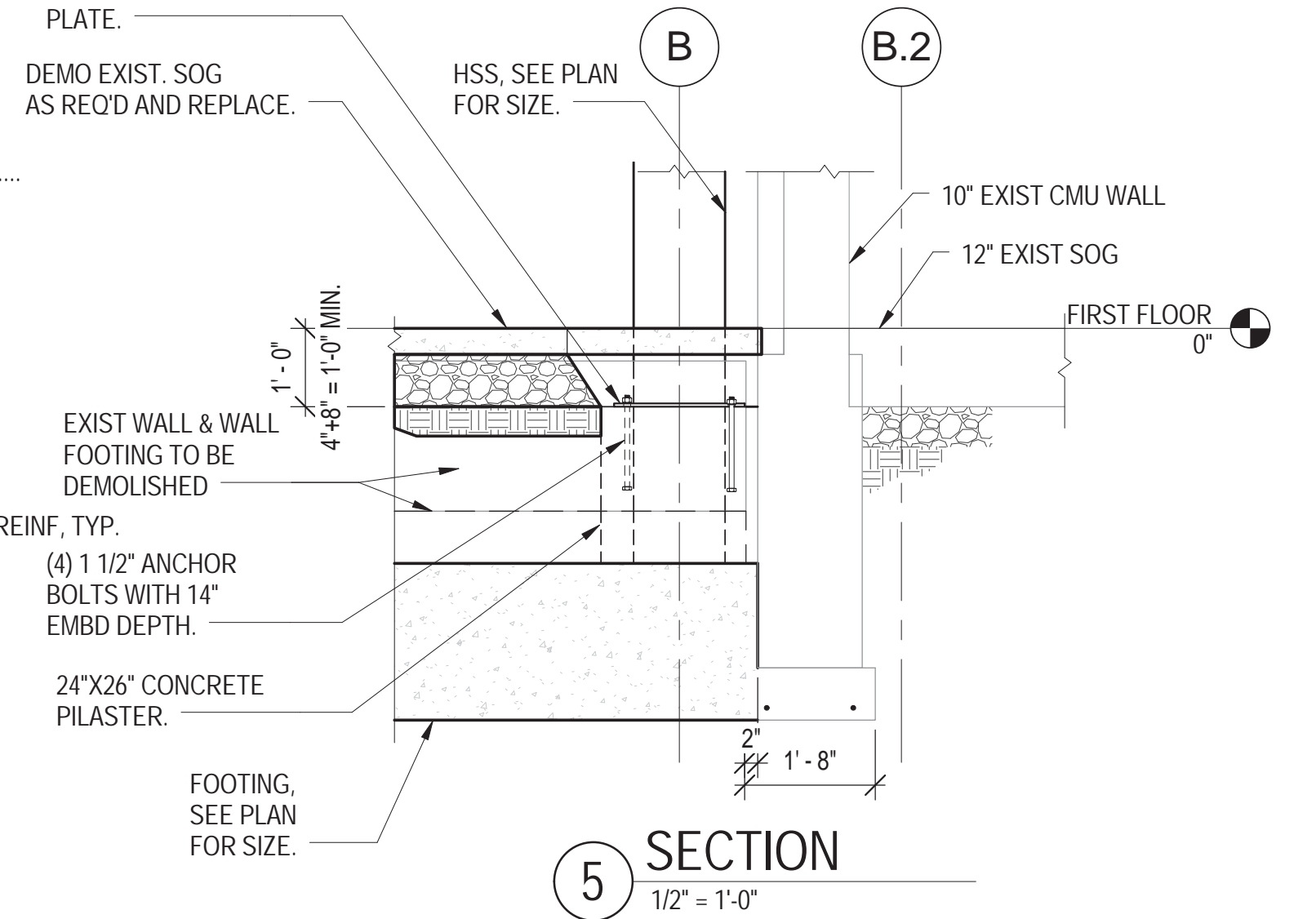
2 SECTION
1/2" = 1'-0"



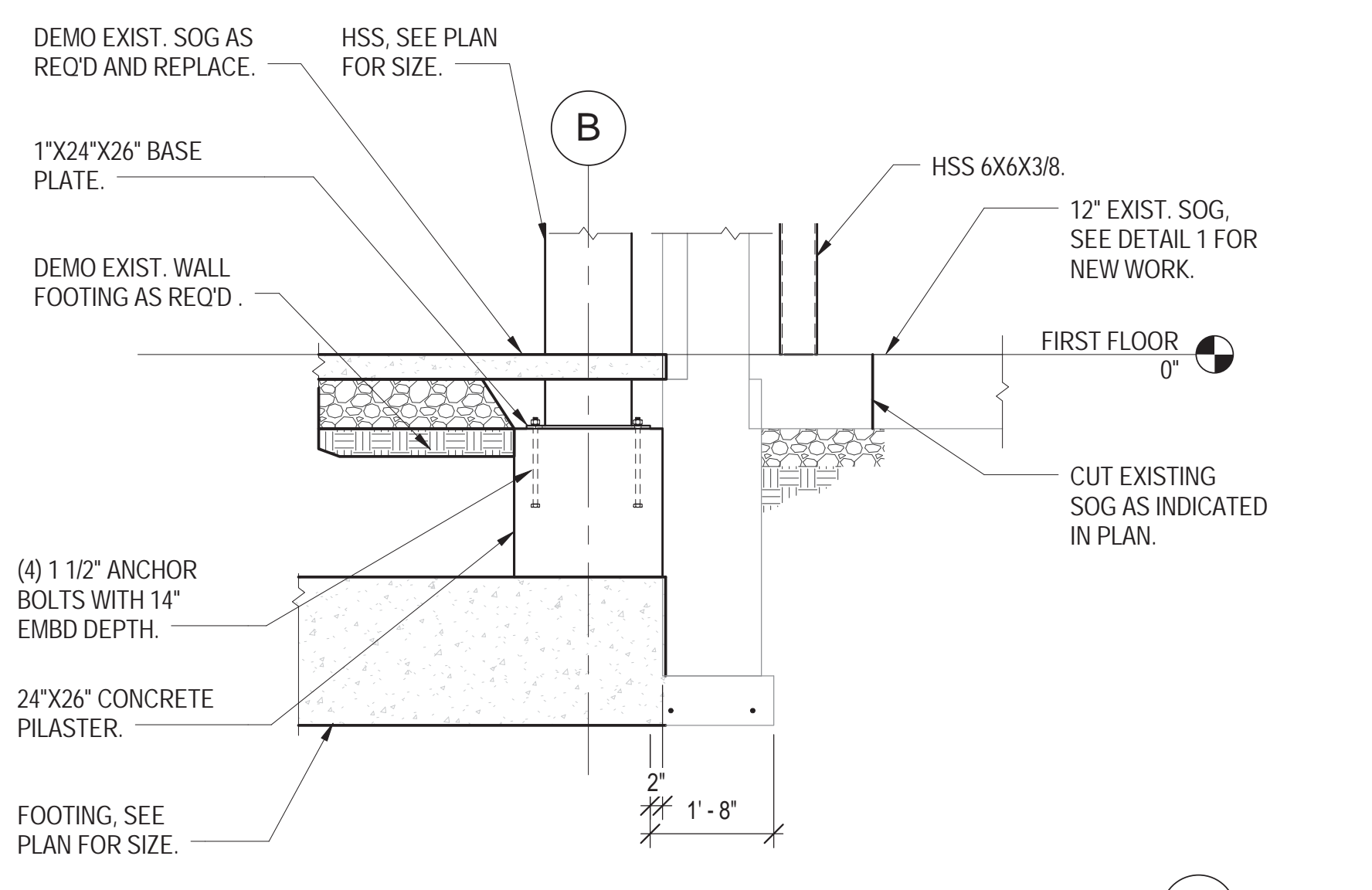
3 SECTION
1/2" = 1'-0"



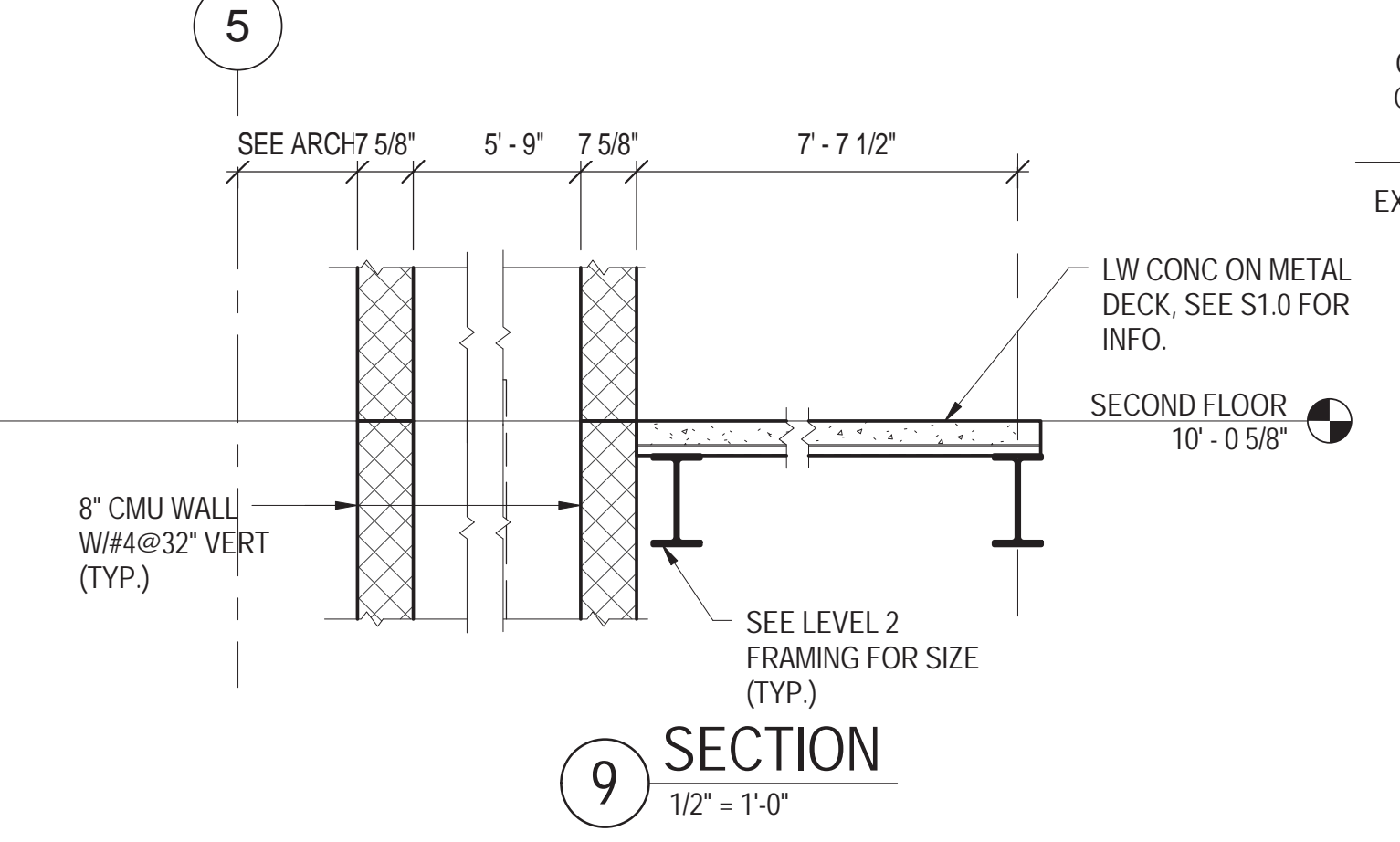
4 ELEVATOR PIT SECTION
1/2" = 1'-0"



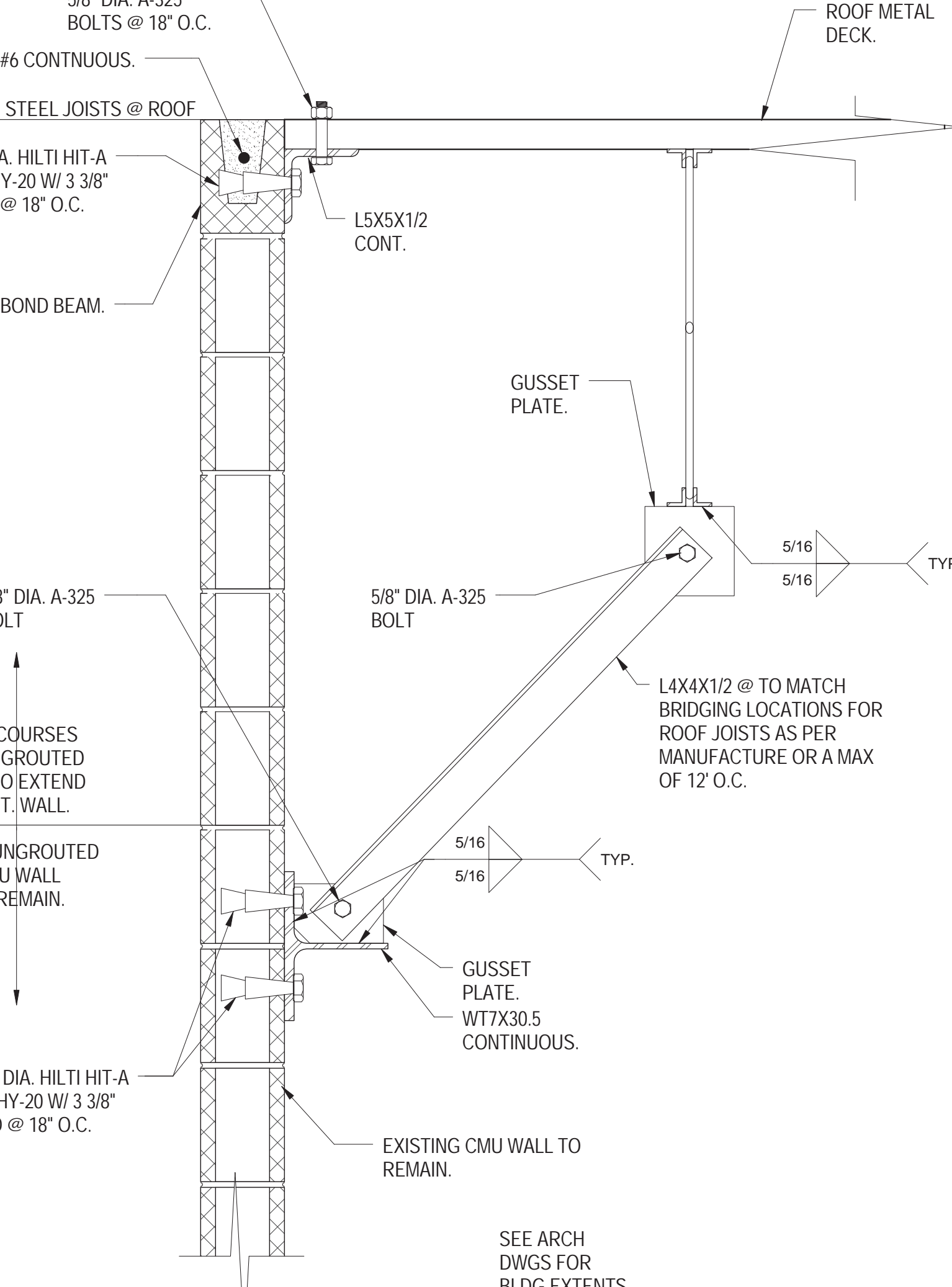
5 SECTION
1/2" = 1'-0"



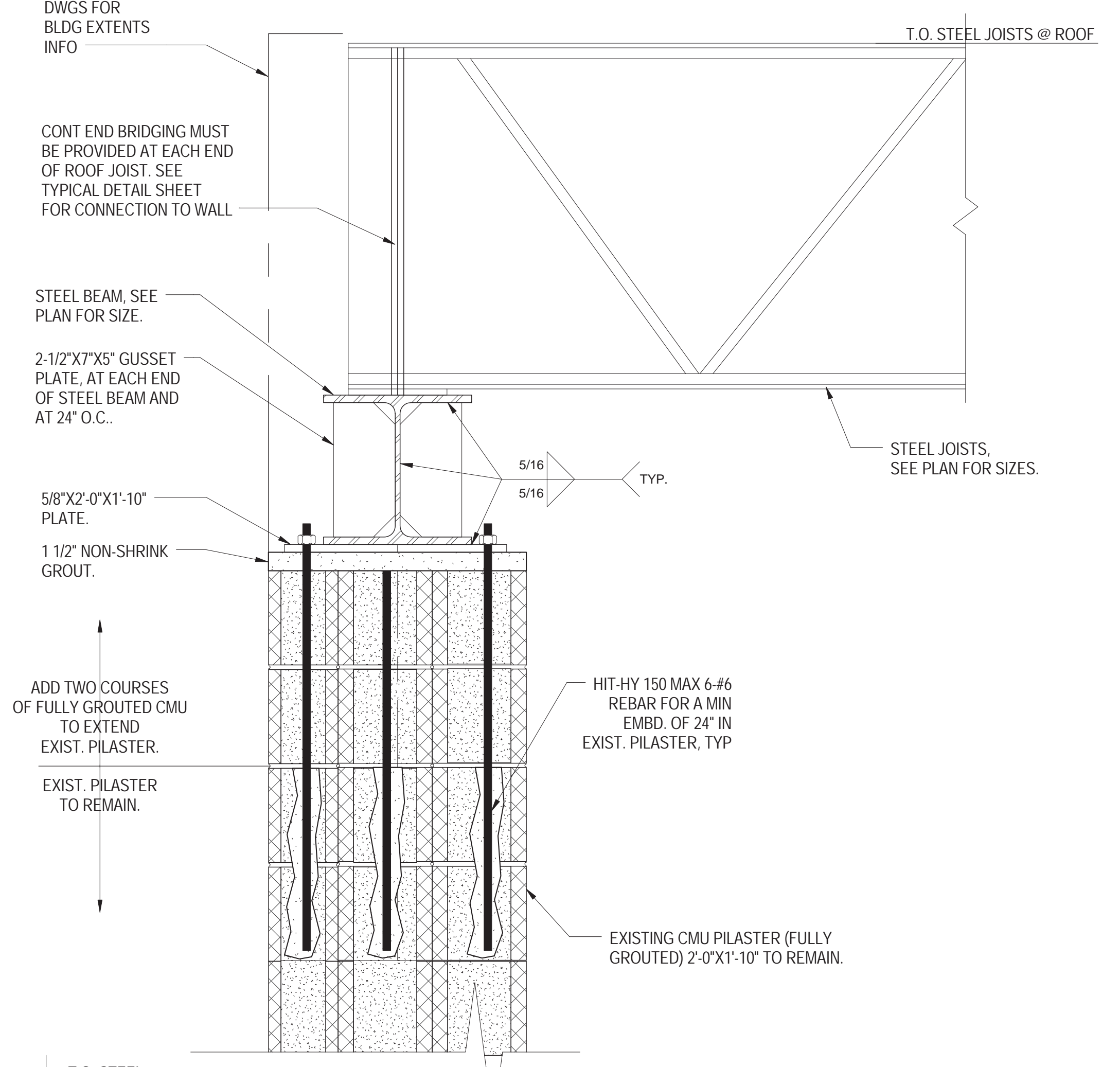
6 SECTION
1/2" = 1'-0"



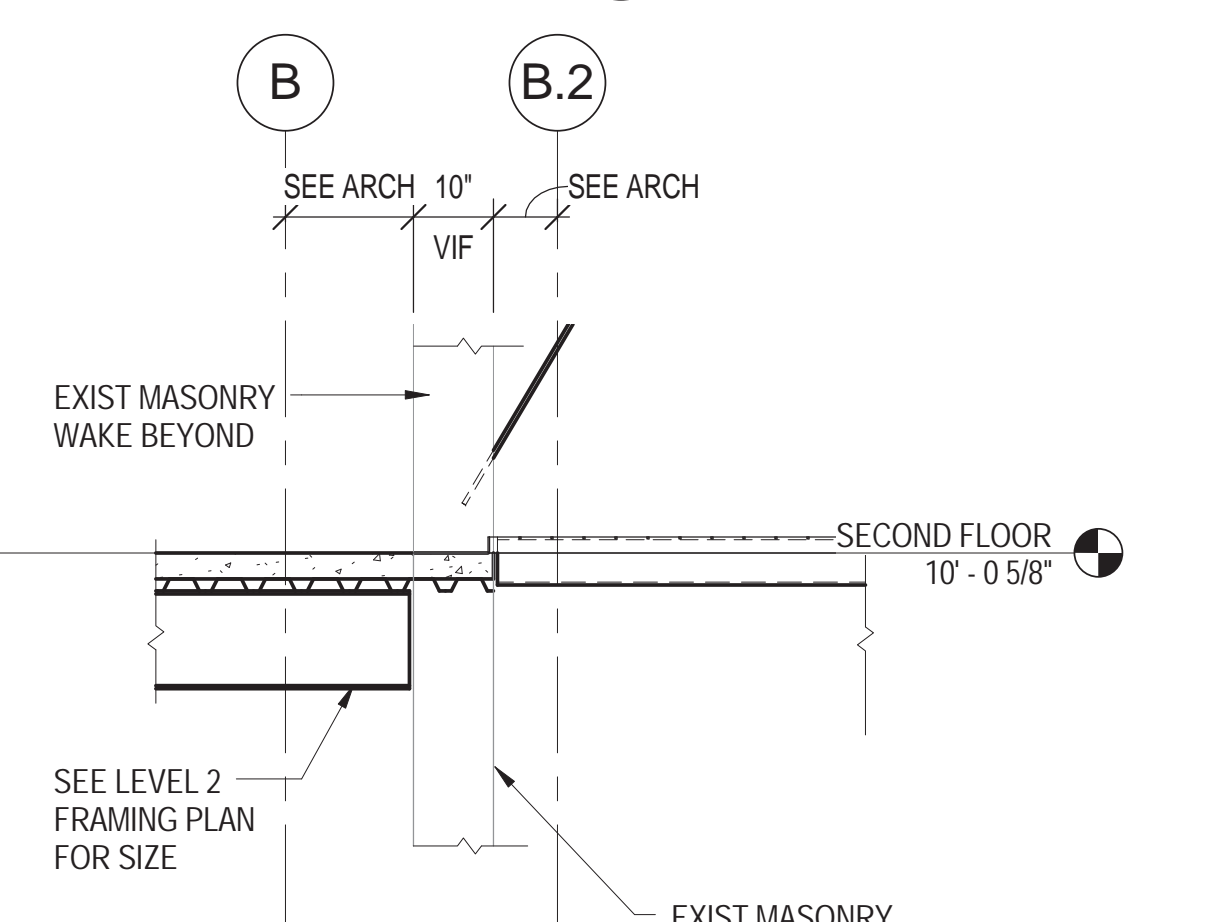
9 SECTION
1/2" = 1'-0"



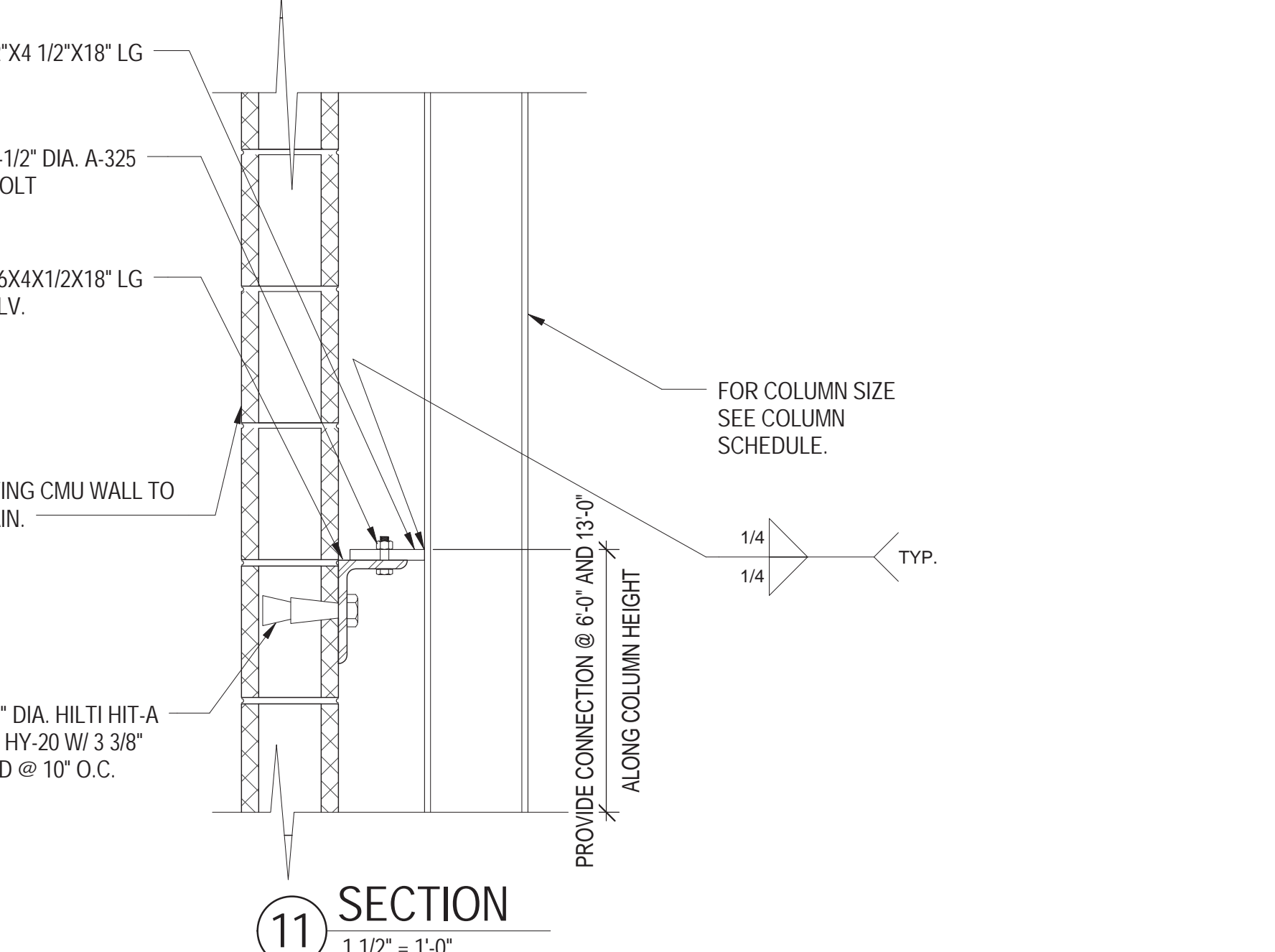
10 SECTION
1 1/2" = 1'-0"



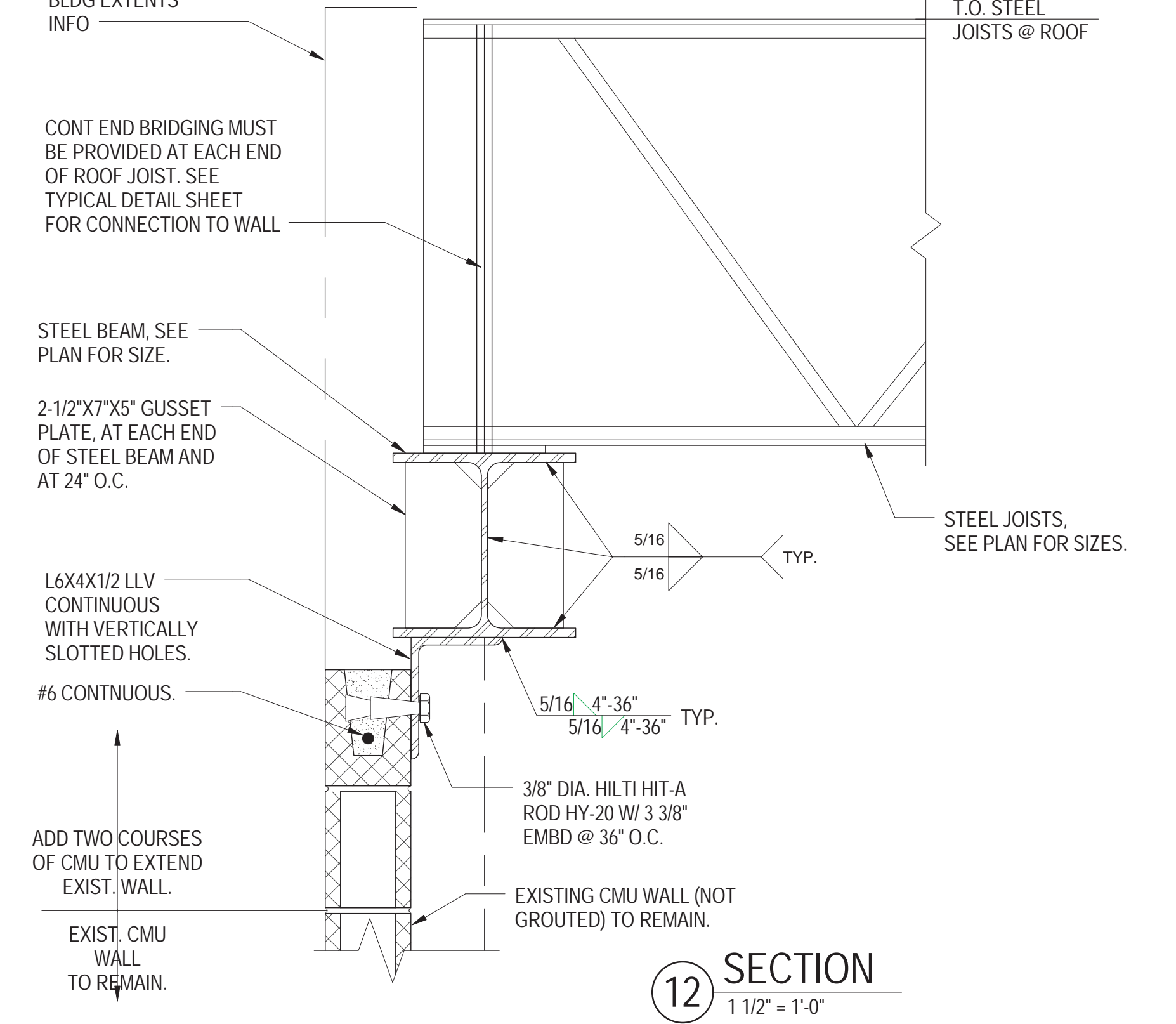
7 SECTION
1 1/2" = 1'-0"



8 SECTION
1/2" = 1'-0"



11 SECTION
1 1/2" = 1'-0"



12 SECTION
1 1/2" = 1'-0"

Revisions	Date

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Drawing Title SECTIONS I		Project Title Renovations to the Former BRAC Property	Date 09.03.2013
Building No. 20	Project Architect: PF&A	Checked by: MP	Drawn by: VC
Location Huntington, WV	Architects Proj. No. 2099.11	S3.01	SHEET 027 OF 115



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