

DRAWING NUMBER

DRAWING NAME

FOUNDATION PLAN

SECTION OR DETAIL DESCRIPTION

CONCRETE

STEPPED FOOTING LOCATION

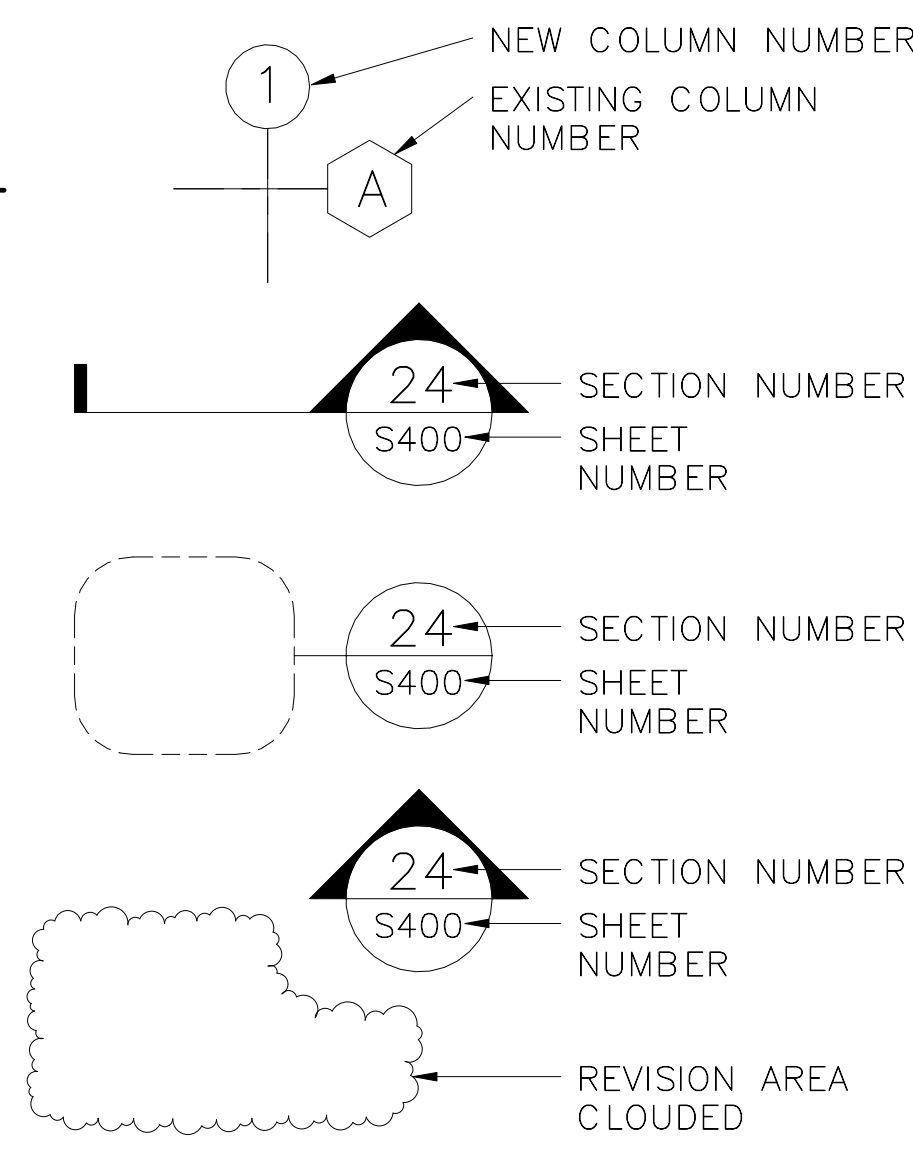
CONCRETE BLOCK

WOOD BLOCKING

EARTH/  
COMPACT FILL

STEEL

ELEVATION OR WORK POINT



A.B.	ANCHOR BOLT	E	EAST	L.L.	LIVE LOAD	R	RADIUS
ADD'L	ADDITIONAL	EA.	EACH	LLH	LONG LEG HORIZONTAL	R.D.	ROOF DRAIN
ALT	ALTERNATE	ELEV.	ELEVATION	LLV	LONG LEG VERTICAL	REINF.	REINFORCE(D), (ING)
ARCH.	ARCHITECT(URAL)	EMBED.	EMBEDMENT	M		REQ'D	REQUIRED
B		EQ.	EQUAL	MAS.	MASONRY	REV.	REVISION, REVISE(D)
BLDG	BUILDING	EXIST.	EXISTING	MATL.	MATERIAL		
BLK.	BLOCK	EXP.	EXPANSION	MAX.	MAXIMUM	S	
BLKG.	BLOCKING	EXT.	EXTERIOR	MECH.	MECHANICAL	S	SOUTH
BR	BRAVE	F	FABRICATE(OR)	MEZZ.	MEZZANINE	SCHED.	SCHEDULE
BOT.	BOTTOM	FD.	FLOOR DRAIN	MFG.	MANUFACTURE(R)	SM.	SIMILAR
BRG.	BEARING	FNDR	FOUNDATION	MIN.	MINIMUM	SJI	STEEL JOIST INSTITUTE
BTWN.	BLOCKING	FTG.	FOOTING	MISC	MISCELLANEOUS	SPA.	SPACE(S)
C				M.O.	MASONRY OPENING	SQ.	SQUARE
C.I.P.	CAST IN PLACE	G		N		STD.	STANDARD
C.J.	CONTROL JOINT	GA.	GAGE, GAUGE	N	NORTH	STL.	STEEL
CL.	CENTER LINE	GALV.	GALVANIZED	N.I.C	NOT TO CONTRACT	STRUCT.	STRUCTURAL
CLR.	CLEAR(ANCE)	G.C.	GENERAL CONTRACT(OR)	NTS	NOT TO SCALE	T	
CMU	CONCRETE MASONRY UNIT	H		O		TBE.	TOP OF BEAM ELEVATION
COL.	COLUMN	HORIZ.	HORIZONTAL	O.C.	ON CENTER(S)	TDE.	TOP OF DECK ELEVATION
COMP.	COMPOSITE	HK.	HOOK	O.H.	OVERHEAD	TEMP.	TEMPORARY
CONC.	CONCRETE	H.S.	HEADED STUDS	OPNG.	OPENING	TFE.	TOP OF FOOTING ELEVATION
CONN.	CONNECTION	H.S.S.	HOLLOW STRUCT. STEEL	OPP.	OPPOSITE	TPE.	TOP OF PIER ELEVATION
CONST.	CONSTRUCTION					TSE.	TOP OF SLAB ELEVATION
CONT.	CONTINUOUS	I		P		TY.	TYPICAL
COORD.	COORDINATE	INFO.	INFORMATION	P.C.	PRECAST CONCRETE	U	
CTRD.	CENTERED			PERIM.	PERIMETER	U.N.O.	UNLESS NOTED OTHERWISE
D		J		PL	PLATE	V	
DBL.	DOUBLE	J.BE.	JOIST BEARING ELEVATION	PLF	POUNDS PER LINEAR FOOT	VERT.	VERTICAL
DIA.	DIAMETER	JST.	JOIST	PROJ.	PROJECT	W	
DIAG.	DIAGONAL	JT.	JOINT	PSF.	POUNDS PER SQ. FOOT	W	WEST
D.L.	DEAD LOAD	K		PSI.	POUNDS PER SQ. INCH	w/	WITH
DO.	DITTO	K.	KIP	PSI.	POUNDS PER SQ. INCH	W.P.	WORK POINT
DTL.	DETAIL	K.O.	KNOCK-OUT			W.W.F.	WELDED WIRE FABRIC
DWG.	DRAWING						

SHEET NUMBER	SHEET NAME
S-1001-0.1	STRUCTURAL GENERAL NOTES – ADMIN
S-1001-1.1	SPECIAL INSPECTIONS/TESTING/SUBMITTALS
S-1001-2.1	ADMINISTRATION BLDG.–FOUNDATION PLAN
S-1001-3.1	ADMINISTRATION BLDG.–ROOF FRAMING PLAN
S-1001-4.1	FOUNDATION DETAILS/SECTIONS
S-1001-5.1	ROOF FRAMING DETAILS/SECTIONS
S-1001-6.1	TYPICAL MASONRY SECTIONS/DETAILS

## STRUCTURAL NOTES

I. DESIGN DATA

A. BUILDING CODE

1. 2007 OREGON SPECIALTY STRUCTURAL CODE (INTERNATIONAL BUILDING CODE 2006 EDITION)

B. DESIGN LOADS/DESIGN CRITERIA

1. WIND LOAD

BASIC WIND SPEED (3-SECOND GUST)----- 85 MPH

WIND IMPORTANCE FACTOR, I-----1.0

EXPOSURE-----C

INTERNAL PRESSURE COEFFICIENTS, GCp ----+/-0.18

COMPONENT AND CLADDING WIND LOADS

Trib. Area (sq'ft)	ROOF (psf)			WALL (psf)			OVERHANG (psf)	
	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	ZONE 2	ZONE 4	
Ax20	16.6	-16.6	-35.7	17.7	-21.3	-28.3	-28.3	
20cAc50	10.0	10.0	10.0	16.3	16.3	NA	NA	
	-16.1	-21.6	-25.9	-16.7	-19.3	-26.9	-26.9	
10.0	10.0	10.0	10.0	15.3	15.3	NA	NA	
50cAc100	-15.7	-18.6	-18.6	-16.0	-17.8	-26.1	-26.1	
	10.0	10.0	10.0	14.5	14.5	NA	NA	
100cA	15	-18.6	-18.6	-15.9	-16.3	-26.1	-26.1	
	10.0	10.0	10.0	14.1	14.1	NA	NA	

2. ROOF LOADS  
LIVE LOAD (L.L.)-----20 PSF\*  
ADMINISTRATIVE OFFICE  
DEAD LOAD (DESIGN D.L.)-----13 PSF\*  
TRUSS TOP CHORD DESIGN DL-----7 PSF  
TRUSS BTM. CHORD DESIGN DL-----10 PSF  
\* LOADS ARE INCREASED BY A SLOPE FACTOR FOR ANALYSIS.
3. ROOF SNOW LOAD  
GROUND SNOW LOAD,  $P_g$ -----30 PSF  
FLAT-ROOF SNOW LOAD,  $P_s$ -----25 PSF  
SLOPED ROOF SNOW LOAD-----25 PSF\*\*  
SNOW EXPOSURE FACTOR,  $C_e$ -----1.0  
SNOW LOAD IMPORTANCE FACTOR,  $I_s$ -----1.0  
THERMAL FACTOR,  $C_t$ -----1.0
4. EARTHQUAKE DESIGN DATA  
SEISMIC IMPORTANCE FACTOR-----1.0  
OCCUPANCY CATEGORY-----II  
MAPPED SPECTRAL RESPONSE ACCELERATIONS  
 $S_{DS}$ -----0.583  $S_{D1}$ -----0.255  
SPECTRAL RESPONSE COEFFICIENTS  
 $S_{RS}$ -----0.459  $S_{R1}$ -----0.264  
SITE CLASS-----C  
SEISMIC DESIGN CATEGORY D  
BASIC SEISMIC FORCE-RESISTING SYSTEM-----  
SPECIALLY DETAILED MASONRY SHEAR WALLS  
SEISMIC RESPONSE COEFFICIENT-----5  
RESPONSE MODIFICATION FACTOR-----0.092  
ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE
5. DEFLECTION CRITERIA  
ALL MEMBERS SUPPORTING MASONRY ARE DESIGNED FOR A MAXIMUM DEAD LOAD PLUS LIVE  
LOAD DEFLECTION OF SPAN/600 OR 0.3 INCHES, WHICHEVER IS LESS.
- \* MINIMUM ROOF LIVE LOAD (OVER-RIDDEN BY SNOW LOAD IN THIS CASE)  
\*\* AS DICTATED BY LOCAL BUILDING OFFICIAL, PLUS SNOW ACCUMULATION AS REQUIRED BY  
IBC, CHAPTER 16 SECTION 1608.

- C. FUTURE EXPANSION  
THIS PROJECT IS NOT DESIGNED FOR FUTURE EXPANSION.
- D. GENERAL NOTES  
1. IN ALL CASES WHERE A CONFLICT MAY OCCUR, SUCH AS BETWEEN REQUIREMENTS IN THE SPECIFICATION AND REQUIREMENTS ON THE DRAWINGS, THE MORE STRINGENT OF THE TWO REQUIREMENTS SHALL APPLY.
2. IN NO CASE, SHALL WORKING DIMENSIONS BE SCALED FROM PLANS, SECTIONS OR DETAILS ON THE STRUCTURAL DRAWINGS.
- II. SITE WORK
- A. GEOTECHNICAL REPORT
1. FOUNDATIONS, RETAINING & BASEMENT WALLS, FOUNDATION DRAINAGE, SLABS ON GRADE & OTHER ITEMS RELATED TO THE SOILS ARE DESIGNED & SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE RECOMMENDATIONS OF APPLIED GEOTECHNICAL ENGINEERING & GEOLOGIC CONSULTING REPORT DATED JANUARY 29, 2011 INCLUDING.
2. OVER-EXCAVATION IS REQUIRED, FOOTINGS WILL BE FOUNDED ON A MINIMUM 24" THICK GRANULAR BASE OF STRUCTURAL FILL. DESIGN NET SOIL BEARING CAPACITY IS AS FOLLOWS
- A. SPREAD FOOTINGS----- 2000 PSF
- B. STRIP FOOTINGS----- 2000 PSF
3. ALLOWABLE PASSIVE PRESSURE -----300 PCF
4. COEFFICIENT OF FRICTION-----0.4
5. MINIMUM DEPTH FROM EXTERIOR GRADE TO BOTTOM OF BUILDING PERIMETER FOOTINGS SHALL BE 1'-6" . ALL OPEN AIR FOUNDATIONS HAVE A MINIMUM OF 1'-6" Frost Protection.
6. FOUNDATION & RETAINING WALLS ARE DESIGNED FOR AN EQUIVALENT FLUID PRESSURE OF 35 PSF/FT. BACKFILL MATERIALS & OPERATIONS SHALL BE AS RECOMMENDED IN THE SOILS REPORT.
7. SLABS ON GRADE ARE DESIGNED USING A MODULUS OF SUBGRADE REACTION OF 100 PCF. OVER-EXCAVATION IS REQUIRED, SLAB-ON-GRADE FLOORS SHALL BE UNDERLAY BY A MINIMUM 24" THICK GRANULAR BASE OF STRUCTURAL FILL.

- III. CONCRETE**  
**A. CONCRETE MATERIAL PROPERTIES**  
 1. CONCRETE PROPERTIES:

A.	FOOTINGS -----	3000
B.	INTERIOR SLAB ON GRADE-----	28 DAYS
C.	EXTERIOR SLAB ON GRADE-----	4000
		4000

2. C/YLINDER TESTING SHALL BE COMPLETED PER IBC SECTION 1905.

**B. REINFORCING MATERIAL PROPERTIES**

1. REINFORCING PROPERTIES:		(y,ksi)	ASTM
A.	ALL BARS UNLESS NOTED-----	-60	A615
B.	TIES & STIRRUPS-----	-60	A615
C.	WELDED WIRE FABRIC (SMOOTH)-----	65	A185

**C. CAST IN PLACE CONCRETE**

  1. SPACING OF CONSTRUCTION OR CONTROL JOINTS IN WALLS EXPOSED TO VIEW SHALL NOT EXCEED 48 FEET UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. CUT HALF THE HORIZONTAL REINFORC AT CONTROL JOINTS.
  2. SLEEVES EMBEDDED IN SLABS AND WALLS SHALL BE LOCATED CLEAR BETWEEN REINFORCING BARS AND SHALL MAINTAIN CLEAR SPACING EQUAL TO THE DIAMETER OF THE LARGEST SLEEVE IN ANY DIRECTION. SLEEVE GROUPS THAT DO NOT COMPLY WITH THE ABOVE REQUIREMENT SHALL BE CONSIDERED AS AN OPENING AND REINFORCED PER NOTE #5 BELOW.
  3. UNLESS NOTED OTHERWISE ON THE DRAWINGS: PROVIDE EXTRA REINFORC ON ALL SIDES OF ALL MISCELLANEOUS WALL AND SLAB OPENINGS EQUAL TO ONE HALF THE INTERRUPTED REINFORCING BARS EACH SIDE BUT NOT LESS THAN 2 #5 FOR EACH LAYER OF REINFORCEMENT. EXTEND BARS CLASS 'B' LAP LENGTH BUT NOT LESS THAN 2 FEET BEYOND EDGE OF OPENINGS. PROVIDE 2 #4x4'-0" DIAGONAL BARS AT EACH CORNER FOR EACH LAYER OF REINFORCEMENT.
  4. PROVIDE A 3/4" CHAMFER ON ALL EXPOSED CORNERS OF CONCRETE.
  5. PROVIDE ISOLATION JOINTS AROUND COLUMNS AT SLAB ON GRADE AREAS.
  6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

		MINIMUM COVER IN
A.	CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO EARTH-----	3
B.	CONCRETE EXPOSED TO EARTH OR WEATHER:	

- |    |   |       |
|----|---|-------|
|    | #6 THRU #18 BARS-----   | 2     |
|    | #5 & SMALLER BARS-----  | 1 1/2 |
| C. | CONCRETE NOT EXPOSED TO WEATHER OR IN<br>CONTACT WITH GROUND:<br>SLABS & WALLS: #14 & #18 BARS----- | 1 1/2 |
|    | #11 & SMALLER BARS-----   | 3/4   |
|    | BEAMS & COLUMNS:<br>PRIMARY REINFORCEMENT, TIES & STIRRUPS-----                                     | 1 1/2 |

- |     |                 |                                       |          |                |
|-----|-----------------|---------------------------------------|----------|----------------|
| IV. | MASONRY         |                                       |          |                |
| A.  | MASONRY         | MATERIAL PROPERTIES                   |          |                |
|     |                 | 1. MASONRY PROPERTIES:                |          |                |
|     |                 |                                       |          | STRENGTH (PSI) |
|     |                 |                                       |          | ASTM           |
|     |                 |                                       |          | C90-N-I        |
|     |                 | A. HOLLOW MASONRY UNITS-----          | 1900     |                |
|     |                 | B. UNIT MASONRY (ASSY)-----           | 1500 Pm  |                |
|     |                 | C. BRICK MASONRY (ASSY)-----          | 1400 Pm  | C216-SW        |
|     |                 | D. MORTAR TYPE S (FOR ALL BLOCK)----- | 1400 Pm  | C476           |
|     |                 | E. GROUT-----                         | 2000 min | C476           |
|     |                 | F. REINFORCING BARS-----              | 60,000   | A615           |
|     |                 | G. COLD DRAWN STEEL WIRE-----         | 70,000   | A82            |
| B.  | GENERAL MASONRY |                                       |          |                |

1. DESIGN IS BASED ON VALUES AS PUBLISHED IN THE "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES" (ACI-530 / ASC-5 / TMS-402, 2005 EDITION).
2. ALL HOLLOW UNIT BLOCK COMPRESSION TEST STRENGTHS REQUIRED TO ACHIEVE THE 1"m STRESS ABOVE SHALL BE ACCORDING TO "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI-530.1 / ASC-6 / TMS-602, 2005 EDITION, SECTION 1-4) BASED ON STRENGTHS BY THE UNIT STRENGTH METHOD.
3. DESIGN IS BASED ON ENGINEERED MASONRY / ALLOWABLE STRESS DESIGN.
4. SHOP DRAWINGS SHALL BE SUBMITTED SHOWING CMU REINFORCEMENT SIZES, SPACING, LOCATIONS, QUANTITIES AND BENDING AND CUTTING SCHEDULES

- |    |   |        |              |  |
|----|---|--------|--------------|--|
| V. | STEEL   |        |              |  |
|    | STEEL MATERIAL PROPERTIES   |        |              |  |
|    | STEEL PROPERTIES:   |        |              |  |
| A. | STRUCTURAL WIDE FLANGE SHAPES-----  | FY,PSI | ASTM         |  |
| B. | OTHER STRUCT. SHAPES  |        | A992         |  |
| C. | PLATES, ETC.  | 36,000 | A36          |  |
| D. | HIGH STRENGTH BOLTS, U.N.C.   | 74,000 | A325         |  |
| E. | ANCHOR BOLTS  | 58,000 | F1554        |  |
| F. | WELDING ELECTRODES-----   | E70XX  | A233         |  |
| F. | DECK WELDING ELCTRODES-----   | 360XX  | A233         |  |
| G. | STRUCTURAL TUBES-----   | 46,000 | A500 GRADE B |  |
| B. | STRUCTURAL STEEL  |        |              |  |
| 1. | STRUCTURAL STEEL DESIGN & CONSTRUCTION SHALL CONFORM TO IBC CHAPTER 22, SECTION     |        |              |  |
| 2. | 2001, AISC LOAD & RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL       |        |              |  |
| 3. | BUILDINGS & 2201, AISC STEEL DESIGN - PRACTICE 2005, 3RD EDITION                    |        |              |  |
|    | STRUCTURAL STEEL SUPPLIER SHALL SUBMIT SHOP DRAWINGS FOR ALL MATERIAL SUPPLIED, IN  |        |              |  |
|    | ADDITION, THE STRUCTURAL STEEL SUPPLIER SHALL SUBMIT DRAWINGS AND CALCULATIONS      |        |              |  |
|    | CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT FOR ALL |        |              |  |
|    | WELDS, LOADS, RAILINGS, GAP PLATES, BEARING PLATES, BASE PLATES, STIFFENERS, SPICES |        |              |  |
|    | & CONNECTIONS DESIGNED BY THE SUPPLIER.   |        |              |  |

- WOOD / TIMBER CONSTRUCTION
- A. DIMENSION LUMBER
1. STRUCTURAL LUMBER SHALL BE DESIGNED & CONSTRUCTED IN ACCORDANCE W/IBC CHAPTER 23 & THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION, 2005 EDITION.
2. DIMENSION LUMBER SHALL BE #2 OR BETTER SPRUCE-PINE-FIR (GRADED PER NLGA) WITH THE FOLLOWING MINIMUM PROPERTIES: F<sub>b</sub>=875 PSI, F<sub>t</sub> = 450 PSI F<sub>v</sub>= 135 PSI, F<sub>c</sub>= 425 PSI (COMPRESSION PERPENDICULAR TO GRAIN), E<sub>ll</sub> = 1,150 (COMPRESSION PARALLEL TO GRAIN), E<sub>t</sub>=1,400,000 PSI
3. LOAD DURATION FACTOR C<sub>d</sub> = 1.33 FOR WIND LOAD = 1.15 FOR SNOW LOAD.
4. ALL MEMBER SIZES SHOWN ON THE DRAWINGS ARE NOMINAL DIMENSIONS. DRESSED SIZES PUBLISHED IN AMERICAN SOFTWOOD LUMBER STANDARDS PS 20-70 SHALL BE ACCEPTED AS MINIMUM NET SIZES CONFORMING TO SUCH NOMINAL SIZES.
5. ALL PREFABRICATED METAL HARDWARE SHOWN ON THE DRAWINGS IS BY SIMPSON STRONG-TIE COMPANY.
6. WOOD JOISTS SHALL BEAR ON THE FULL WIDTH OF SUPPORTING MEMBERS U.N.O.
7. NAILING SHALL BE IN ACCORDANCE WITH IBC SECTION 2304.9 (AS A MINIMUM) OR AS SHOWN ON THE DRAWINGS. SPACE NAILS TO ALLOW SPLITTING, ALL NAILS ARE COMMON U.N.O.
8. FASTENERS IN TREATED WOOD COMPONENTS (FRT AND PRESERVATIVE) SHALL BE HOT DIP ZINC-COATED GALVANIZED STEEL, U.N.O. ON PLAN.
9. SLEEPERS AND SILLS BEARING ON CONCRETE OR MASONRY AND PERMANENT COLUMNS SUPPORTED BY FOOTINGS IN CONTACT WITH EARTH SHALL BE PRESERVATIVE TREATED SOUTHERN PINE.
10. NAILS AND STAPLES USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL CONFORM TO ASTM F 1667 AND SHALL HAVE THE FOLLOWING MINIMUM BENDING YIELD STRENGTHS:
- | SHANK DIAMETER (IN)               | BENDING YIELD STRENGTH |
|-----------------------------------|------------------------|
| 0.178" - 0.254"                   | 80 KSI                 |
| 0.143" - 0.177"                   | 60 KSI                 |
| 0.099" - 0.142" (16d AND SMALLER) | 100 KSI                |

- B. GLULAM MEMBERS**
1. DESIGN AND CONSTRUCTION SHALL CONFORM TO AITC MANUAL, 2002 EDITION.
  2. SIMPLE SPAN GLULAM MEMBERS SHALL BE 24F-V4, CONTINUOUS OR CANTILEVERED MEMBERS SHALL BE 24F-V8. THE FOLLOWING MINIMUM DESIGN VALUES UNDER DRY USE SHALL APPLY:  
 $F_b = 2400 \text{ PSI}$ ,  $F_v = 180 \text{ PSI}$ ,  $E = 450 \text{ PSI}$ ,  $E = 1,800,000$ .
  3. LOAD DURATION FACTOR  $C_d = 1.33$  FOR WIND LOAD, 1.15 FOR SNOW LOAD
  4. ALL GLULAM MEMBERS SHALL BE WRAPPED.
- C. OPEN WEB TRUSSES**
1. THE DESIGN AND FABRICATION OF ALL TRUSSES SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND ITS FASTENINGS BY NATIONAL FOREST PRODUCTS ASSOCIATION AND NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION BY THE TRUSS PLATE INSTITUTE.
  2. WOOD TRUSS SUPPLIER SHALL DESIGN AND SUPPLY ALL MATERIAL, CONNECTORS, AND ACCESSORIES REQUIRED FOR INSTALLATION INCLUDING, BUT NOT LIMITED TO HANGERS, WALL CONNECTIONS, BLOCKING, WEB STIFFENERS, AND RIM BOARDS TO PROVIDE A COMPLETE INSTALLATION.
  3. WOOD JOIST SUPPLIER SHALL PRODUCE A TRUSS SUBMITTAL PACKAGE INCLUDING THE INFORMATION REQUIRED BY IBC SECTION 2303.4.
  4. ENGINEERED UPLIFT CONNECTIONS SHALL BE PROVIDED AT ALL ROOF MEMBER CONNECTIONS TO WALL TOP PLATES. DESIGN OF CONNECTORS IS BY THE TRUSS SUPPLIER UNLESS NOTED OTHERWISE ON DRAWINGS. TOE NAILING IS NOT ALLOWED. ACCOMMODATE DIFFERENT BEARING CONDITIONS AS SHOWN ON THE DRAWINGS.
  5. TRUSS SUPPLIER SHALL EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND DESIGN TRUSSES FOR SUSPENDED EQUIPMENT SUCH AS UNIT HEATERS. THE RESULTING LOAD SHALL BE CONSIDERED AS DEAD LOAD BEYOND THAT PROVIDED IN SECTION 1.B.
  6. WOOD TRUSS SUPPLIER'S TRUSS SUBMITTAL PACKAGE SHALL BE SIGNED AND SEALED BY AN ENGINEERED LICENSED IN THE STATE OF THE PROJECT.
  7. NOTCHES SHALL NOT BE CUT IN WOOD JOISTS. SEE MANUFACTURER INSTRUCTIONS FOR LOCATING AND SIZING OPENINGS IN THE WEBS OF WOOD JOISTS.
  8. SEE THIS SHEET AND PLANS FOR LOADS APPLIED TO TRUSSES. LOAD DURATION FACTOR  $C_d = 1.33$  FOR WIND LOAD, 1.15 FOR SNOW LOAD. DEAD LOAD LISTED INCLUDES THE WEIGHT OF THE TRUSS.
  9. ROOF TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF  $\text{SPAN}/360$  OR  $1" \text{ MAX}$ .

- D. STRUCTURAL SHEATHING
- GENERAL
1. PLYWOOD SHEATHING SHALL CONFORM TO IBC SECTION 2303.1.4 AND SHALL BE GRADE STAMPED BY APA. PROVIDE THE THICKNESS AND GRADE SHOWN ON THE DRAWINGS.
  2. NAIL SIZE AND PATTERN SHALL BE AS INDICATED ON THE DRAWINGS.
  3. PLYWOOD STAMP SHALL HAVE A MINIMUM OF 4 PLIES.
- E. WOOD DECKING (LOBBY AREA ROOF)
1. COMPLY WITH AITC 112, "STANDARD FOR TONGUE-AND-GROOVE HEAVY TIMBER ROOF DECKING".
  2. COMPLY WITH DOC PS 20, "AMERICAN SOFTWOOD LUMBER STANDARD," AND WITH APPLICABLE GRADING RULES OF INSPECTION AGENCIES CERTIFIED BY ALSC'S BOARD OF REVIEW.
  3. PROVIDE SOLID-SAWN WOOD DECKING WITH EACH PIECE FACTORY MARKED WITH GRADE STAMP OF INSPECTION AGENCY EVIDENCING COMPLIANCE WITH GRADING RULE REQUIREMENTS AND IDENTIFYING GRADING AGENCY, SPECIES, GRADE, MOISTURE CONTENT AT TIME OF SURFACING, AND MILL. APPLY GRADE STAMP TO SURFACES THAT WILL NOT BE EXPOSED TO VIEW.
  4. PROVIDE WOOD DECKING WITH 15 PERCENT MAXIMUM MOISTURE CONTENT AT TIME OF DRESSING.
  5. FABRICATE DECKING IN LENGTHS TO ALLOW FOR MULTIPLE SPAN CONTINUOUS LAYOUT.
  6. PROVIDE TEMPORARY WATERPROOF COVERING TO PROTECT EXPOSED DECKING BEFORE APPLYING ROOFING.
  7. DECK SPECIES: SPRUCE PINE-FIR.  
DECKING NOMINAL SIZE: AS INDICATED ON DRAWINGS.  
DECKING GRADE: COMMERCIAL GRADE.  
EDGE PATTERN: VEE GROOVED.
- ROOF SHEATHING - HORIZONTAL USE

1. PANEL ROOF SHEATHING SHALL BE 3/4" x 6G, APA EXPOSURE 1, WITH 48/24 SPAN RATING UN ON PLAN.
2. FASTENERS SHALL BE A MINIMUM OF 10d COMMON NAILS (0.148" DIAMETER) DRIVEN FLUSH WITH THE SURFACE OF THE SHEATHING.
3. FASTEN ROOF PANELS TO SUPPORTS USING COMMON SPACING AT 12" O.C. IN THE FIELD AND AT 6" O.C. AT THE EDGES AT A MINIMUM. PROVIDE TIGHTER SPACING WHERE INDICATED ON THE DRAWINGS.
4. SHEATHING SHALL BE ORIENTED WITH THE STRONG AXIS PERPENDICULAR TO SUPPORTS WITH STAGGERED END JOINTS.

**BID DOCUMENTS 7/1/2011**

[illegible]

Advanced Structural Technologies  
5001 American Blvd. W., Suite 800  
Bloomington, MN 55437  
(952) 854-9302 tel  
(952) 854-9690 fax  
[www.astmn.com](http://www.astmn.com)

# A s T

**Anderson Engineering of Minnesota, LLC**  
 13605 1st AVENUE NORTH, SUITE 100, PLYMOUTH, MN  
 TEL (763) 412-4000 FAX (763) 412-4090

www.ae-mn.com



NATIONAL  
CEMETERY  
ADMINISTRATION

Drawing Title		Project Title		Date	
STRUCTURAL GENERAL NOTES - ADMIN		REPLACE ADMINISTRATION AND MAINTENANCE BUILDING AND SITE IMPROVEMENTS		07/01/2011	
				Project No.	
				906CM3009	
Approved: Director, Office of Construction Management		Building Number	Checked	Drawn	DRAWING NO.
			J. LEVAR	J. RIECK	S-1001-0.1
Approved: Director, Project Management Service		Location	EAGLE POINT NATIONAL CEMETERY		Dwg. 77 Of 181
			2763 RILEY ROAD FAGU E POINT, 08425-24		