

GENERAL NOTES:

I. GENERAL

- MATERIALS AND WORKMANSHIP TO CONFORM WITH THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE, WITH TITLE 24 AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- THESE GENERAL NOTES SUPPLEMENT THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS. IN CASE OF CONFLICT BETWEEN THE PLANS AND SPECIFICATIONS, CONTACT THE VA COR.
- VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE CONDITIONS BEFORE STARTING WORK. NOTIFY THE VA COR OF DISCREPANCIES.
- REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR SITE SLABS AND WALLS.
- DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW BY THE VA COR.
- DETAILS ON SHEETS TITLED "TYPICAL DETAILS" APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. SUCH DETAILS ARE NOT NOTED AT EACH LOCATION THAT THEY OCCUR.
- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND FOR CHECKING DIMENSIONS. NOTIFY THE RESIDENT ENGINEER OF ANY DISCREPANCIES AND RESOLVE BEFORE PROCEEDING WITH THE WORK.
- DO NOT SCALE THE DRAWINGS.
- INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENTS THE PRESENT KNOWLEDGE, BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE ARCHITECT. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE VA COR.
- REFER TO ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS OF FLOOR AND ROOF OPENINGS. COORDINATE THE SIZE AND LOCATION OF OPENINGS ASSOCIATED WITH, BUT NOT LIMITED TO, ELECTRICAL, MECHANICAL, AND PLUMBING TRADES. SUBMIT FINAL SIZING AND LOCATION REQUIREMENTS OF OPENINGS TO THE ARCHITECT FOR REVIEW.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING A SAFE PLACE TO WORK AND MEETING THE REQUIREMENTS OF ALL APPLICABLE JURISDICTIONS. EXECUTE WORK TO ENSURE THE SAFETY OF PERSONS AND ADJACENT PROPERTY AGAINST DAMAGE BY HAZARDS IN CONNECTION WITH THIS WORK.
- SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS.

II. CONSTRUCTION MEANS AND METHODS ENGINEERING (SHORING)

- CONTRACTOR TO PROVIDE MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES INCLUDE BUT ARE NOT LIMITED TO BRACING, UNDERPINNING, AND SHORING FOR LOADS DURING CONSTRUCTION. RETAIN CALIFORNIA REGISTERED CIVIL ENGINEER WHOM IS PROPERLY QUALIFIED TO DESIGN BRACING, SHORING, ETC.
- VISITS TO THE SITE BY THE STRUCTURAL ENGINEER OF RECORD WILL NOT INCLUDE OBSERVATION OF THE CONSTRUCTION MEANS AND METHODS SHORING.

III. FOUNDATION AND SITE WORK

- THE DESIGN OF THE FOUNDATION SYSTEM IS BASED UPON THE CRITERIA AND RECOMMENDATIONS CONTAINED IN THE REPORT ENTITLED, "GEOTECHNICAL AND GEOLOGICAL INVESTIGATION FOR BUILDING 334 AND PHARMACY VETERANS AFFAIRS MEDICAL CENTER", PREPARED BY LANGAN TREADWELL ROLLO, DATED 25 NOVEMBER 2014.
- GROUNDWATER ELEVATION IS ESTIMATED IN THE GEOTECHNICAL REPORT. PROVIDE SITE DE- WATERS AS REQUIRED.
- LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
- REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED. BACKFILL VOIDS PER GEOTECH REPORT.
- NOTIFY THE COR IF ANY BURIED STRUCTURES NOT INDICATED ARE FOUND.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
- REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE PER THE GEOTECHNICAL REPORT.
- EXCAVATIONS FOR FOUNDATIONS MUST BE ACCEPTED BY THE GEOTECHNICAL ENGINEER AND THE VA COR PRIOR TO PLACING REINFORCING AND CONCRETE. NOTIFY THE VA'S COR ENGINEER WHEN EXCAVATIONS ARE READY FOR INSPECTION.
- FOUNDATIONS ARE TO BEAR ON COMPETENT SOILS. UNDOCUMENTED IF ENCOUNTERED IS TO BE REMOVED AND REPLACED W/ ENGINEERED FILL OR LEAN CONCRETE PER GEOTECHNICAL REPORT.
- PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.
- MECHANICALLY COMPACT EXCAVATION BACKFILLS IN LAYERS PER THE GEOTECH REPORT.

IV. FORMWORK

- PROVIDE FOUR POCKETS IN FORMS AND UNDER EXISTING STRUCTURAL MEMBERS AS REQUIRED TO PREVENT AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE EXISTING MEMBERS. CONCRETE CAST WITH AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE MEMBERS IS NOT ACCEPTABLE.
- REMOVE FORMS AND SHORES IN ACCORDANCE WITH THE FOLLOWING:

LOCATION	REMOVE FORMS AND SHORES NO SOONER THAN
BOTTOM FORMS AND SHORES FOR MILDLY REINFORCED SLABS, BEAMS AND GIRDERS	3 DAYS AND MIN. 75% F _c
SIDE FORM FOR BEAMS AND GIRDERS	24 HOURS
COLUMNS AND WALLS	24 HOURS
FOOTINGS, PILE CAPS, AND GRADE BEAMS	24 HOURS

- PROVIDE CURING WHERE FORMS ARE REMOVED IN LESS THAN 7 DAYS, INCLUDING BUT NOT LIMITED TO WALLS, COLUMNS, AND UNDERSIDE OF ELEVATED SLABS.

V. REINFORCING STEEL

- REINFORCING TO CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED:

LOCATION	TYPE
ALL REINFORCING STEEL IN SUSPENDED SLAB, WALLS, FOUNDATIONS, AND ALL REINFORCEMENT REINFORCING STEEL #8 AND LARGER	ASTM A706, 60 KSI
REINFORCING STEEL #7 AND SMALLER OR IN SLABS ON GRADE, HOUSEKEEPING PADS OR CURBS.	ASTM A615, 60 KSI
WELDED STEEL WIRE FABRIC	ASTM A185, 70 KSI
SMOOTH DOWELS IN SLAB ON GRADE	ASTM A36, 36 KSI

- ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCEMENT FROM DISPLACING DUE TO FORMWORK, CONSTRUCTION, OR CONCRETE PLACEMENT OPERATIONS. LOCATE AND SUPPORT REINFORCING BY METAL CHAIRS, RUNNERS, BOLSTERS, SPACERS, OR HANGERS.
- MECHANICAL COUPLERS: TYPE 2, SEE SPECS FOR ADDITIONAL INFORMATION.
- WELD REINFORCING STEEL ONLY WHERE NOTED AND IN ACCORDANCE WITH AWS D1.4 USING QUALIFIED WELDERS.
- TERMINATE REINFORCING STEEL IN STANDARD HOOKS, UNLESS OTHERWISE SHOWN.
- INFORM REINFORCING SHOWN OR NOTED CONTINUOUS IN LENGTHS AS LONG AS PRACTICABLE.
- PROVIDE REINFORCING BAR DOWELS IN FOOTINGS AND OTHER SUPPORTING MEMBERS OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCING, U.O.N. LAP SPLICE AS INDICATED ON TYPICAL DETAILS.

VI. CAST-IN-PLACE CONCRETE

- CONCRETE IS REINFORCED AND CAST-IN-PLACE UNLESS OTHERWISE NOTED. WHERE REINFORCING IS NOT SPECIFICALLY SHOWN OR WHERE DETAILS ARE NOT GIVEN, PROVIDE REINFORCING SIMILAR TO THAT SHOWN FOR SIMILAR CONDITIONS. SUBJECT TO REVIEW BY THE VA COR.
- ROUGHEN CONCRETE SURFACES OF CONSTRUCTION JOINTS TO 1/4 INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES. SUBMIT JOINT LOCATIONS OR JOINTS NOT SHOWN TO THE VA COR FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH THE WORK.
- AT LOCATIONS WHERE CONCRETE IS CAST AGAINST EXISTING CONCRETE, ROUGHEN CONTACT SURFACES TO 1/4 INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF ADDITIONAL CONCRETE CURBS AND HOUSEKEEPING PADS NOT SHOWN.
- CONCRETE CLEAR COVER TO REINFORCING BARS IS AS FOLLOWS, UNLESS OTHERWISE NOTED:

LOCATION	CLEAR COVER
CONCRETE PLACED AGAINST EARTH	3 INCHES
FORMED SURFACES EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: #6 BARS AND LARGER #5 BARS AND SMALLER	2 INCHES 1 1/2 INCHES
SLABS ON GRADE (TOP CLEARANCE)	1 1/2 INCHES
BEAMS, GIRDERS AND COLUMNS NOT EXPOSED TO WEATHER OR EARTH	1 1/2 INCHES
WALL OR SLAB SURFACES NOT EXPOSED TO WEATHER OR EARTH: #5 & SMALLER #6 & #7 #8, #9, #10 & #11 #14 & #18	1/2 INCH 1 INCH 1 1/2 INCHES 2 1/2 INCHES

- CONCRETE TYPES:

CLASS	28-DAY STRENGTH	TYPE	LOCATION	CEMENT	MAX W/C	MAX AGGR
A	4000 PSI	NWC	GRADE BEAMS, FOUNDATIONS,	I-II	0.45	SIZE 467
B	4000 PSI	NWC	SLABS ON GRADE, PADS, CURBS MISC.	I-II	0.45	SIZE 67
C	4000 PSI	NWC	FILL ON METAL DECK	I-II	0.55	SIZE 67

- LIGHT WT CONC. MAX. DRY UNIT WT=115 PCF.
- WATER REDUCING OR HIGH RANGE WATER REDUCING ADD MIXTURES SATISFYING ASTM C494 ARE ANTICIPATED.
- NO AIR ENTRAINMENT IS ANTICIPATED. FOR STRUCTURAL CONC.
- CONTINUOUSLY MOIST CURE ARCHITECTURALLY EXPOSED CONCRETE SLABS IN PUBLIC SPACES FOR 7 DAYS MINIMUM. WATER FOG SPRAYS, PONDING, SATURATED ABSORPTIVE COVERS, OR MOISTURE RETAINING COVERS MAY BE USED. CURING COMPOUNDS ARE ACCEPTABLE AT NON-ARCHITECTURALLY EXPOSED SLABS.
- CONCRETE FILL THICKNESS SHOWN ON THE FRAMING PLANS ARE MINIMAL THICKNESSES. NO ALLOWANCES HAVE BEEN SHOWN FOR ADDITIONAL CONCRETE FILL REQUIRED TO COMPENSATE FOR FRAME, DECK, OR FORMWORK DEFLECTIONS TO MAINTAIN SURFACE TOLERANCES.
- NON-SHRINK GROUT, 7000 PSI MIN. @ 28 DAYS.
- SEE SPECIFICATION SECTION 033000 FOR FLOOR FLATNESS REQUIREMENTS.

VII. UNIT MASONRY

- MINIMUM COMPRESSIVE STRENGTH OF MASONRY, F_m, EQUAL TO 2,000 PSI AT 28 DAYS.
- MASONRY UNITS: ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT, HOLLOW, LOAD BEARING UNITS. MINIMUM COMPRESSIVE STRENGTH 2,800 PSI.
- MORTAR: ASTM C-270, TYPE S.
- GROUT: ASTM C-476, MINIMUM COMPRESSIVE STRENGTH 2000 PSI.
- REINFORCING STEEL:

LOCATION	TYPE
REINFORCING TO BE WELDED	ASTM A706, GRADE 60
ALL OTHER REINFORCING	ASTM A615, GRADE 60
JOINT REINFORCING	UBC STD 21-10
- CENTER VERTICAL REINFORCING IN WALL UNLESS NOTED OTHERWISE.
- LAY UNITS IN RUNNING BOND AND MAINTAIN VERTICAL CONTINUITY OF CORES OR CELL CAVITIES. USE OPEN END UNITS AT LOCATIONS OF VERTICAL REINFORCING. PLACE HORIZONTAL REINFORCING IN BOND BEAM UNITS.
- FILL ALL CELLS SOLIDLY WITH GROUT. CLEAN CELLS AND BOND BEAMS OF MORTAR PROTRUSIONS AND DEBRIS BEFORE GROUTING.
- DOWELS FROM THE FOUNDATION TO MATCH SIZE AND LOCATION OF VERTICAL REINFORCING IN MASONRY, UNLESS NOTED OTHERWISE.

VIII. STRUCTURAL STEEL

- STRUCTURAL STEEL TO CONFORM TO THE FOLLOWING UNLESS OTHERWISE NOTED:

SECTIONS	TYPE
ROLLED SHAPES WIDE FLANGES CHANNELS, ANGLES, & OTHER	ASTM A992, GR 50 ASTM A36
PLATES COLUMN BASE PLATES BEAM COVER/SIDE PLATES BEAM SHEAR PLATES COLUMN CONTINUITY PLATES BEAM STIFFENER PLATES OTHER, U.O.N.	ASTM A572, GR 50 ASTM A36 ASTM A572, GR 50 ASTM A36 ASTM A572, GR 50
STEEL PIPE COLD FORMED HOLLOW STRUCTURAL SECTION (HSS)	ASTM A53 GRADE B ASTM A500 GRADE B
STAINLESS STEEL SHAPES, PLATES AND BARS	ASTM A276
BOLTS	ASTM 325X
MACHINE BOLTS	ASTM A307
ANCHOR BOLTS AND RODS	ASTM F1554 GR 55 U.O.N.
THREADED AND HANGER ROD	ASTM A307 OR A36
WELDED SHEAR CONNECTORS	ASTM A108, GRADE 1015 THROUGH 1020
NUTS FOR BOLTS AND MACHINE BOLTS	ASTM A563
HARDENED WASHERS	ASTM F436
UNHARDENED WASHERS	ASTM F844
PLAIN WASHERS	ANSI B18.22.1
BEVELED WASHERS	ANSI B18.23.1

- HOT DIP GALVANIZE IN ACCORDANCE WITH ASTM A123 AND ASTM A153 STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER. REPAIR GALVANIZING AFTER WELDING IN ACCORDANCE WITH ASTM A780.
- ARC-WELDING ELECTRODES/FILLER METALS TO BE LOW HYDROGEN TYPES E7XTX, E7XTX OR E70XXX MINIMUM AS APPLICABLE.
- WELDERS TO BE CERTIFIED BY AWS AND THE GOVERNING JURISDICTION.
- WHERE FIELD WELDING IS NOTED, THE DESIGNATION IS GIVEN AS A SUGGESTED CONSTRUCTION PROCEDURE ONLY.
- PROVIDE NATURAL CAMBER UP, UNLESS NOTED OTHERWISE. EXCEPT AT CANTILEVERS. AT CANTILEVERS PROVIDE CAMBER SUCH THAT TIP OF CANTILEVER IS ABOVE FINAL ELEVATION.
- SPLICE MEMBERS ONLY WHERE INDICATED.
- DETAIL, FABRICATE AND ERECT IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS".

IX. METAL DECKING

- METAL FLOOR AND ROOF DECK TO HAVE MINIMUM SECTION PROPERTIES SHOWN ON SHEET "TYPICAL METAL DECK DETAILS".
- METAL DECK TO MEET ASTM A653
- FLOOR AND ROOF DECK TO BE GALVANIZED IN ACCORDANCE WITH ASTM A653 COATING CLASS G60. REPAIR DAMAGED COATING.
- WHERE POSSIBLE, LAYOUT METAL DECK TO SPAN AT LEAST THREE SPANS CONTINUOUSLY. TERMINATE ENDS OVER SUPPORTS EXCEPT AT OPENINGS OR BUILDING EDGES WHERE METAL DECKS MAY BE CANTILEVERED AS SHOWN.
- PROVIDE MINIMUM STUDS SHOWN IN 9/16" U.O.N.
- SEE SPECIFICATION SECTION 033000 FOR FLOOR FLATNESS REQUIREMENTS.

X. ROUGH CARPENTRY AND LIGHT GAGE METAL FRAMING

- PANEL SHEATHING: IDENTIFY WOOD STRUCTURAL PANELS WITH THE APPROPRIATE TRADEMARK OF APA- THE ENGINEERED WOOD ASSOCIATION AND MEET THE REQUIREMENTS OF THE VOLUNTARY PRODUCT STANDARD PS-1 OR PS-2 AND APA PRP-108 PERFORMANCE STANDARD.
 - PANEL SHEATHING TO BE EXPOSURE 1.
 - PLYWOOD PANELS TO BE 4-PLY MINIMUM (OSB NOT ALLOWED).
- PLYWOOD TO BE CC GRADE AT LOCATIONS EXPOSED TO WEATHER, CD GRADE ELSEWHERE.
- PROVIDE THE FOLLOWING GRADE AND SPAN RATINGS:

PANEL THICKNESS	MINIMUM GRADE	ROOF/FLOOR RATING
15/32	STRUCTURAL 1	32/16
- ROUGH HARDWARE:
 - SCREWS: ASTM A307, ANSII/ASME STANDARD B18.6.1. USE CADMIUM-PLATED PAN OR ROUND HEADED SCREWS AT STEEL TO WOOD AND WOOD TO WOOD CONNECTIONS.
 - COLD-FORMED STEEL FRAMING @ SHEAR WALLS AS INDICATED ON PLAN:

SECTIONS	MINIMUM STRENGTH	MINIMUM THICKNESS
WALL STUDS	50 KSI	0.054 (16 GA)
TOP/BOTTOM TRACK	50KSI	0.054 (16 GA)
 - INSTALL BOLTS THROUGH COLD-FORMED STEEL WITH WASHERS.
 - PLYWOOD SHALL NOTE BE GLUED TO STUDS

XI. MECHANICAL AND ADHESIVE ANCHORS

- ALL MECHANICAL ANCHORS ARE TO HAVE ICC APPROVAL FOR USE IN CRACKED CONCRETE UNDER SEISMIC LOADS.
- EXPANSION ANCHORS: HILTI KB-TZ (ICC ESR-1917), ITW REDHEAD TRIBOLT+ (ICC ESR-2427), POWERS POWER-STEEL (ICC ESR-2502), OR SIMPSON STRONG BOLT 2 (ICC ESR-3037).
- CONCRETE SCREW ANCHORS: POWERS WEDGE-BOLT (ICC ESR-2526), OR SIMPSON TITEN HD (ICC ESR-2713).
- ADHESIVE ANCHORS: HILTI HIT RE-500-SD (ICC ESR-2322), POWERS PE1000+ (ICC ESR-2583), OR SIMPSON SET-XP (ICC ESR-2508)
- INSTALL ANCHORS IN ACCORDANCE WITH THE LATEST ICC-ESR REPORT.
- PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN PERMANENTLY EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.
- ADHESIVE ANCHORS SHALL BE ASTM A36 THREADED RODS. REINFORCING DOWELS SHALL BE ASTM A615 GRADE 60 REINFORCING STEEL. UNLESS OTHERWISE NOTED.
- EXPANSION AND SCREW ANCHOR MINIMUM EMBEDMENTS AND TEST LOADS IN CONCRETE, UNLESS OTHERWISE NOTED:

ANCHOR DIA. (SIZE)	MIN. EMBED (INCH)	WEDGE LOAD (LBS.)	TORQUE (FT-LBS.)
1/4"	2"	800	10
3/8"	2 1/2"	1100	25
1/2"	2 1/2"	2000	50
5/8"	3 1/2"	2300	80
3/4"	4 1/2"	3700	150

* MINIMUM EMBEDMENT MUST BE INCREASED FOR SIMPSON TITEN HD ANCHOR. USE 3 1/4" FOR 1/2" DIAMETER AND 5 1/2" FOR 5/8" DIAMETER SCREWS.

- ADHESIVE ANCHOR MINIMUM EMBEDMENT DEPTH AND TEST LOADS IN CONCRETE, UNLESS OTHERWISE NOTED:

ANCHOR/BAR SIZE	MIN. EMBEDMENT	TENSION LOAD
3/8"	4"	3,000 LBS.
1/2"	5"	6,000 LBS.
5/8"	6"	8,000 LBS.
3/4"	7"	12,000 LBS.
7/8"	9"	15,000 LBS.
NO. 3	4"	3,000 LBS.
NO. 4	5"	6,000 LBS.
NO. 5	6"	8,000 LBS.
NO. 6	7"	12,000 LBS.
NO. 7	9"	15,000 LBS.

- TEST ANCHORS PER 130541 AND THESE NOTES.
- TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
- REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING.
- TEST QUANTITY OF EXPANSION AND ADHESIVE ANCHORS FOR EACH SIZE AND TYPE AS NOTED BELOW:

APPLICATION	QUANTITY
ADHESIVE, EXPANSION, AND SCREW ANCHORS	10%
ADHESIVE, EXPANSION, AND SCREW ANCHORS AFTER TEST FAILURE OF ONE ANCHOR	100% OF 20 PREVIOUS
- APPLY THE TEST LOAD TO EXPANSION ANCHORS PER EITHER THE HYDRAULIC RAM METHOD OR THE TORQUE WRENCH METHOD. APPLY THE TEST LOAD TO SCREW ANCHORS PER THE TORQUE WRENCH METHOD. APPLY THE TEST LOAD TO ADHESIVE ANCHORS PER THE HYDRAULIC RAM METHOD.
- THE FOLLOWING CRITERIA APPLY FOR THE TESTING AND ACCEPTANCE OF INSTALLED ANCHORS:
 - HYDRAULIC RAM METHOD: MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST. E.G AS EVIDENCED BY LOOSENING OF THE WASHER UNDER THE NUT.
 - TORQUE WRENCH METHOD: ATTAIN THE SPECIFIED TORQUE WITHIN 1/2 TURN OF THE NUT.

XII. STRUCTURAL TESTS, INSPECTIONS, AND OBSERVATIONS

- AN INDEPENDENT TESTING AGENCY AND SPECIAL INSPECTORS WILL BE RETAINED BY THE CONTRACTOR AND APPROVED BY THE VA TO PERFORM THE TESTS AND INSPECTION REQUIRED BY SPEC 014529. PROVIDE ACCESS AND FURNISH SAMPLES TO THE AGENCY AS REQUIRED BY THE CONTRACT DOCUMENTS. THE VA'S GEOTECHNICAL ENGINEER WILL PROVIDE OBSERVATION AND TESTING OF EARTHWORK ACTIVITIES AND CHECK FOOTING EXCAVATIONS.
- IF INITIAL TESTS OR INSPECTIONS MADE BY THE TESTING AGENCY REVEAL THAT ANY PORTION OF THE WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS WILL BE MADE AT THE CONTRACTOR'S EXPENSE.

XIII. DESIGN CRITERIA

- APPLICABLE CODES / DESIGN STANDARDS:

VA STRUCTURAL DESIGN MANUAL FOR HOSPITAL PROJECTS
VA PROGRAM GUIDE PG-18-1, MASTER CONSTRUCTION SPECIFICATIONS
VA PROGRAM GUIDE PG-18-3, DESIGN AND CONSTRUCTION PROCEDURES
VA PROGRAM GUIDE PG-18-15, AE/ SUBMISSION INSTRUCTIONS, VOLUME B
VA HANDBOOK H-18-9, SEISMIC DESIGN REQUIREMENTS, FEBRUARY 2011
DEPARTMENT OF DEFENSE 2010 UNIFIED FACILITIES CRITERIA (UFC)
2012 IBC
ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
2010 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION STEEL BUILDINGS
2010 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SEISMIC PROVISIONS
2011 AMERICAN CONCRETE INSTITUTE (ACI) 318

- FOUNDATIONS HAVE BEEN DESIGNED WITH THE FOLLOWING CRITERIA, PER LANGAN TREADWELL ROLLO GEOTECHNICAL AND GEOLOGICAL INVESTIGATION. (SEE FOUNDATION AND SITE NOTE 1)

SPREAD FOOTINGS:
ALLOWABLE NET SOIL PRESSURE FOR DL + LL = 1,500 PSF
ALLOWABLE NET SOIL PRESSURE FOR DL + LL + EQ = 2,000 PSF

- GRAVITY LOADS:

A. DEAD LOADS - VARY BASED ON ACTUAL BUILDING AND EQUIPMENT OPERATING WEIGHTS

B. LIVE LOADS:
FLOOR 100 PSF
ROOF 20 PSF

C. LIVE LOAD DEFLECTION
TYPICAL INTERIOR L/360
PERIMETER L/360

- SNOW LOADS = 0 PSF

- FLOOD LOADS = 0 LBS

- SEISMIC DESIGN- RESPONSE SPECTRUM

WHERE: FOR METAL STUD & PLYWOOD SHEAR WALLS
R = 7.0
CD = 4.5
RD = 2.5
I = 1.5
SS = 1,500
S1 = 0.639
SDS = 1,000
SD1 = 0.639
SITE CLASS = D
OCCUPANCY CATEGORY = IV
SEISMIC DESIGN CATEGORY = D

INELASTIC INTERSTORY DRIFT =

- WIND DESIGN:
BASIC WIND SPEED = 115 MPH
WIND EXPOSURE = B

- NON-STRUCTURAL ELEMENTS, EQUIPMENT (NEW AND RELOCATED) AND UTILITIES ARE TO BE ANCHORED AND BRACED. DESIGN OF THIS ANCHORAGE IS THE RESPONSIBILITY OF THE CONTRACTOR PER SPEC 130541.

- NON-STRUCTURAL ELEMENTS, EQUIPMENT, AND UTILITY LINES SUPPORTED BY STRUCTURAL FRAMING ARE TO BE CONCENTRICALLY CONNECTED TO THE FRAMING MEMBERS. BRACING TO THE BOTTOM 2/3 OF BEAMS AND JOISTS IS PROHIBITED.

- HANGING AND BRACING NON-STRUCTURAL ELEMENTS, EQUIPMENT AND UTILITIES FROM THE ROOF DECK IS PROHIBITED. PROVIDE SUPPLEMENTAL FRAMING BETWEEN STRUCTURAL BEAM FRAMING AS NEEDED PER SPEC. 130541.
- EQUIPMENT, ARCHITECTURAL ELEMENTS, AND DISTRIBUTION SYSTEMS ARE TO BE ANCHORED AND BRACED PER SPECIFICATION 130541.

- DESIGN TEAM
JAMES O MALLEY SENIOR PRINCIPAL
ROBERT GRAFF PROJECT MANAGER
DESIGNER
EDGAR DIAZ SENIOR CAD SPECIALIST

100% CD / BID SUBMISSION
FEBRUARY 2, 2016

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Drawing Title
GENERAL NOTES

Approved: Project Director

Project Title
POST TRAUMATIC STRESS DIAGNOSIS (PTSD)
EXPANSION & RENOVATION

Location
795 WILLOW ROAD, MENLO PARK CA

Date
11/03/2014

Checked
RG

Drawn
ED

Project Number
640-235003

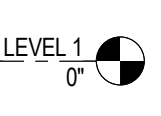
Building Number

Drawing Number

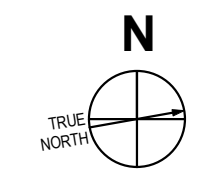
360-J-S001

Office of
Construction
and Facilities
Management





1/4" = 1'-0"



65% SUBMISSION	03/16/15
Revisions:	Date

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Drawing Title

FOUNDATION PLAN - PHARMACY (BLDG 360 - 1 WING)

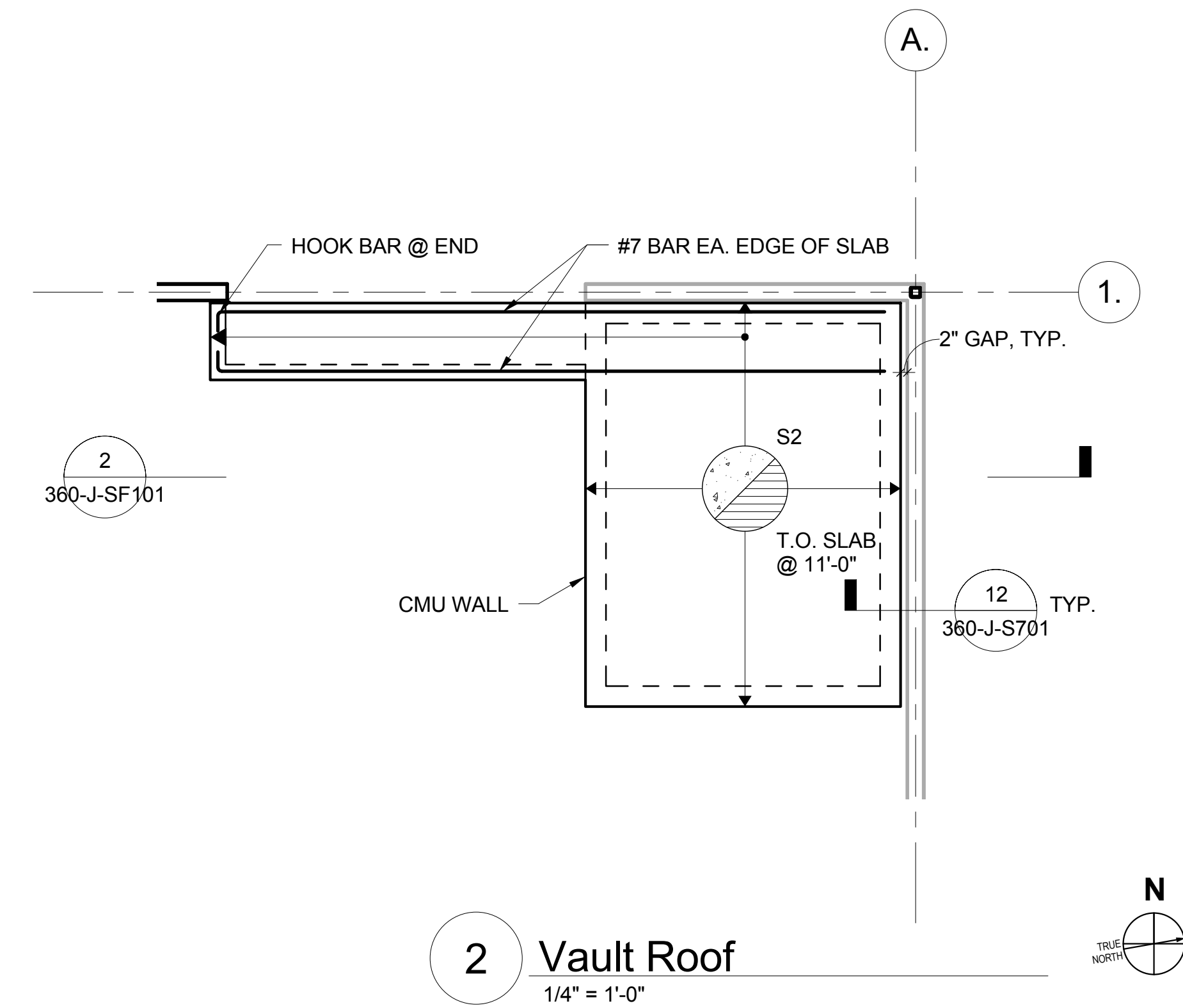
Approved: Project Director

Project Title POST TRAUMATIC STRESS DIAGNOSIS (PTSD) EXPANSION & RENOVATION			
Location 795 WILLOW ROAD, MENLO PARK CA			
Date 11/03/2014	Checked RG		Drawn ED

Project Number	640-Z35003
Building Number	
Drawing Number	360-J-S

Office of
Construction
and Facilities
Management

 Department of
Veterans Affairs



- ## KEY PLAN



Department of
Veterans Affairs

Project Number 640-Z35003
Building Number
Drawing Number 360-J-SF102

Project Title POST TRAUMATIC STRESS DIAGNOSIS (PTSD) EXPANSION & RENOVATION		
Location 795 WILLOW ROAD, MENLO PARK CA		
Date 11/03/2014	Checked RG	Drawn ED

Drawing Title
ROOF PLAN - PHARMACY (BLDG 360 - 1 WING)

Approved: Project Director

ARCHITECT

POLYTECH ASSOCIATES INC.

235 Pine Street, 17th Floor
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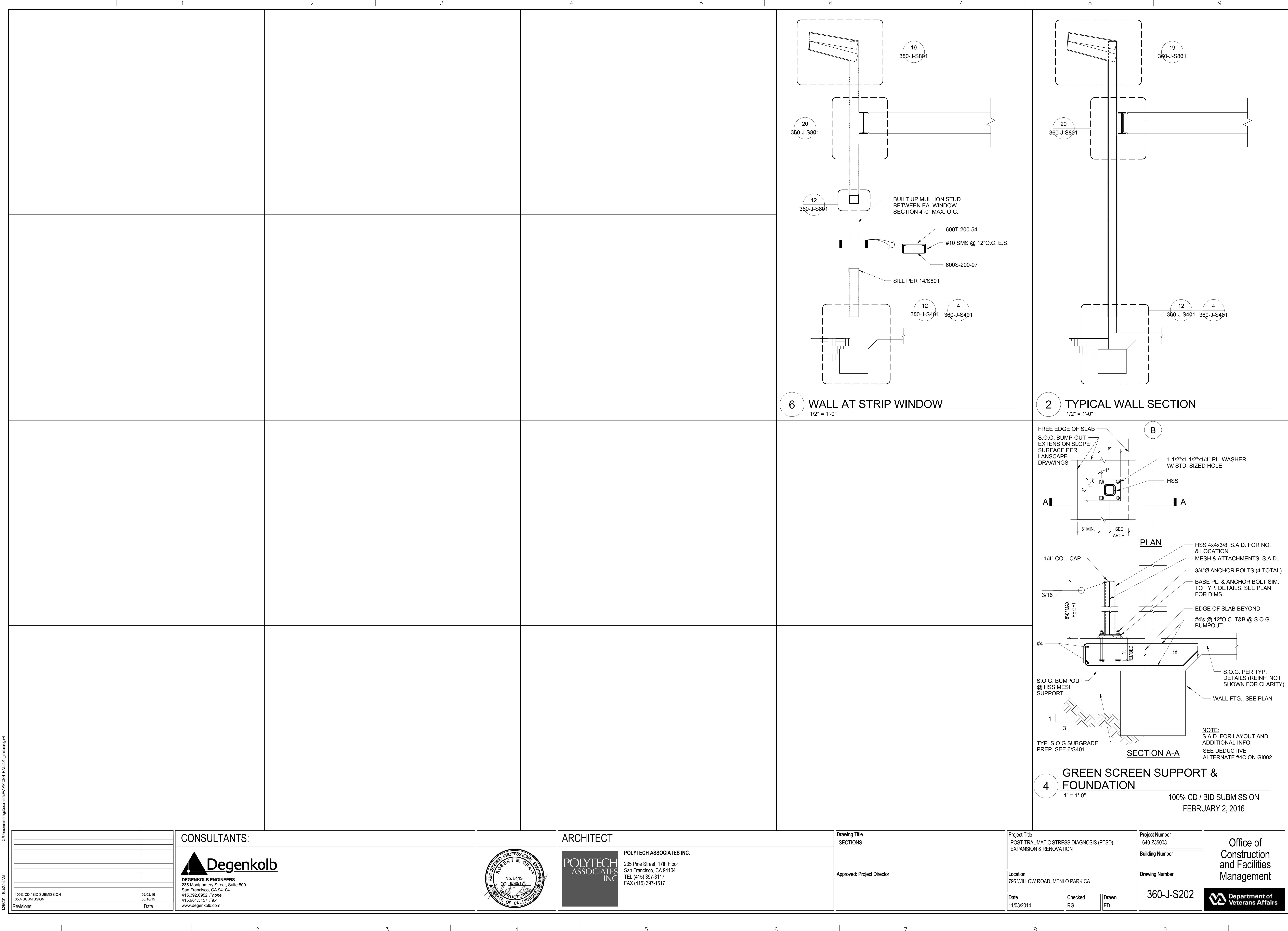
DEGENKOLB ENGINEERS
 235 Montgomery Street, Suite 500
 San Francisco, CA 94104
 415.392.6952 Phone
 415.981.3157 Fax
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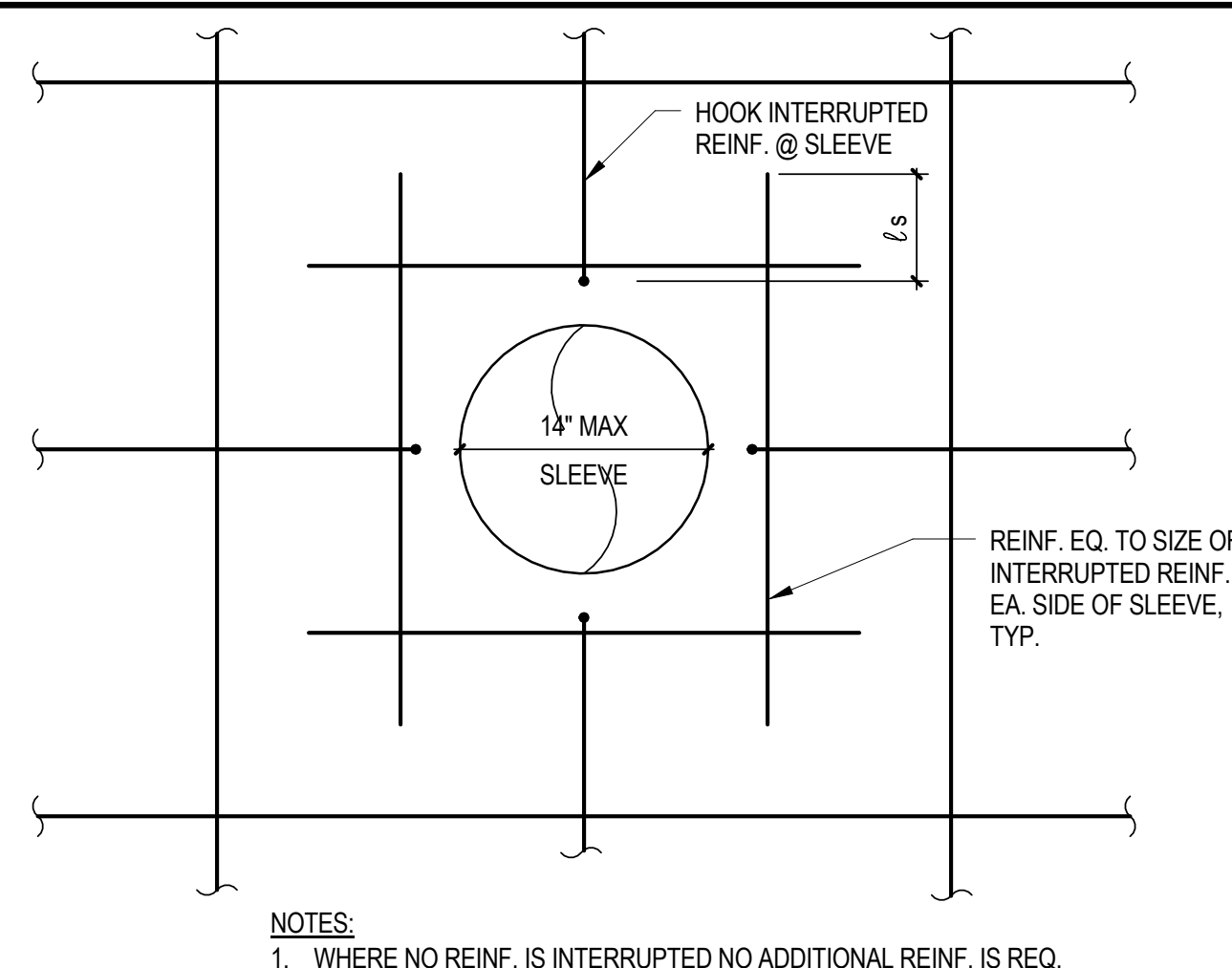
100% CD / BID SUBMISSION	02/02/16
85% SUBMISSION	03/16/15
Deductions:	Date:



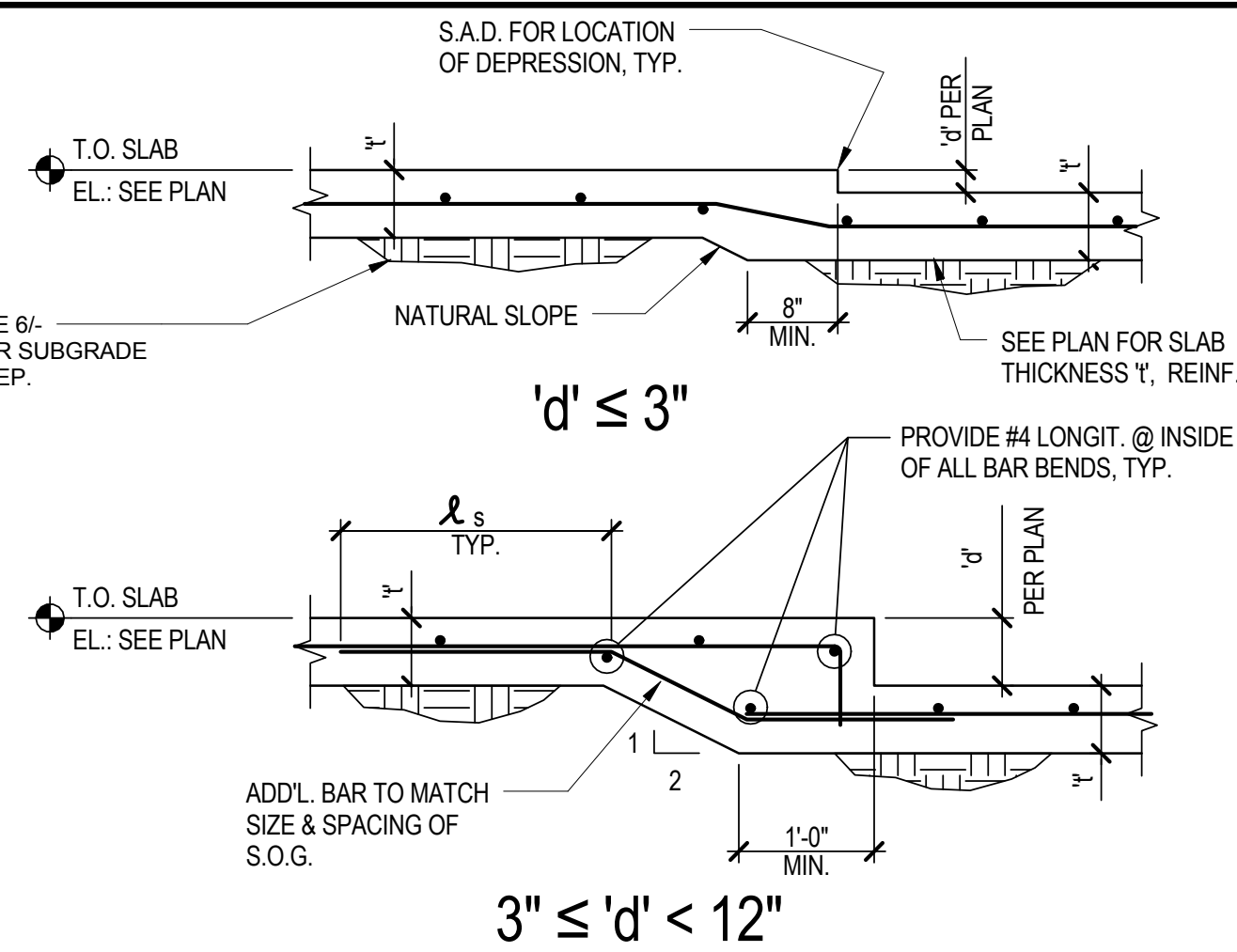
Office of
Construction
and Facilities
Management

 Department of
Veterans Affairs

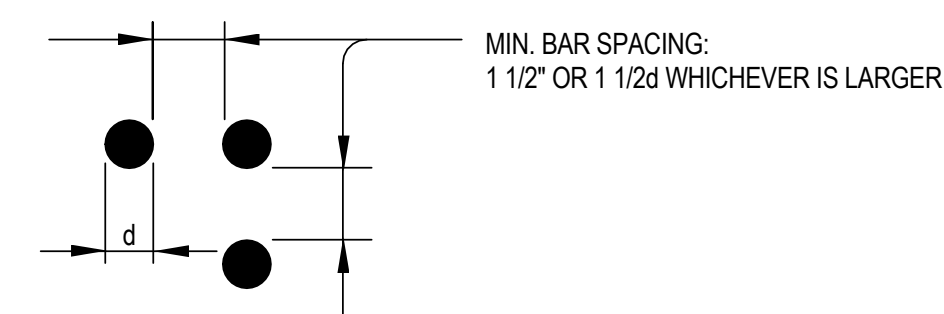




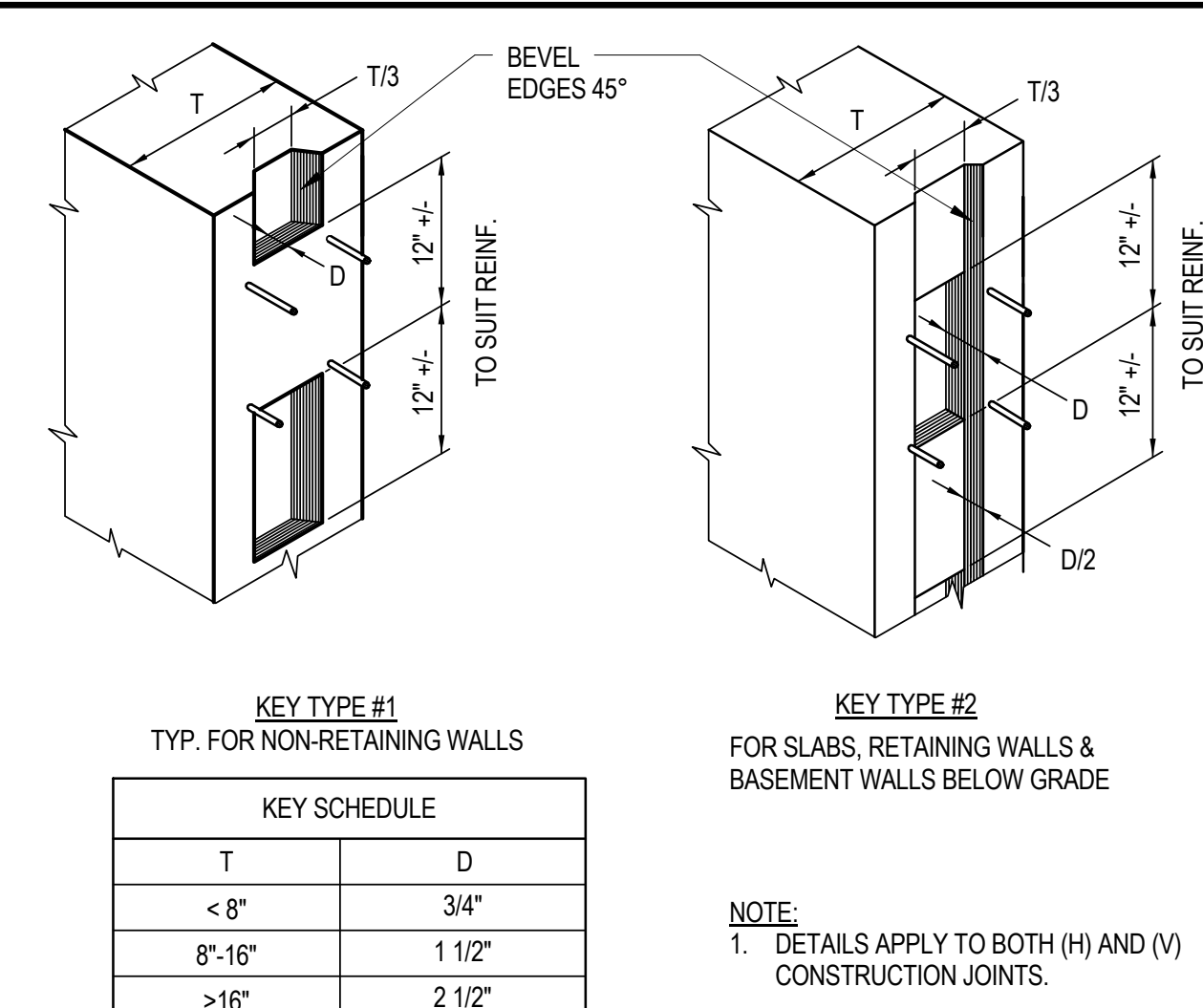
17 SLEEVES IN SLABS



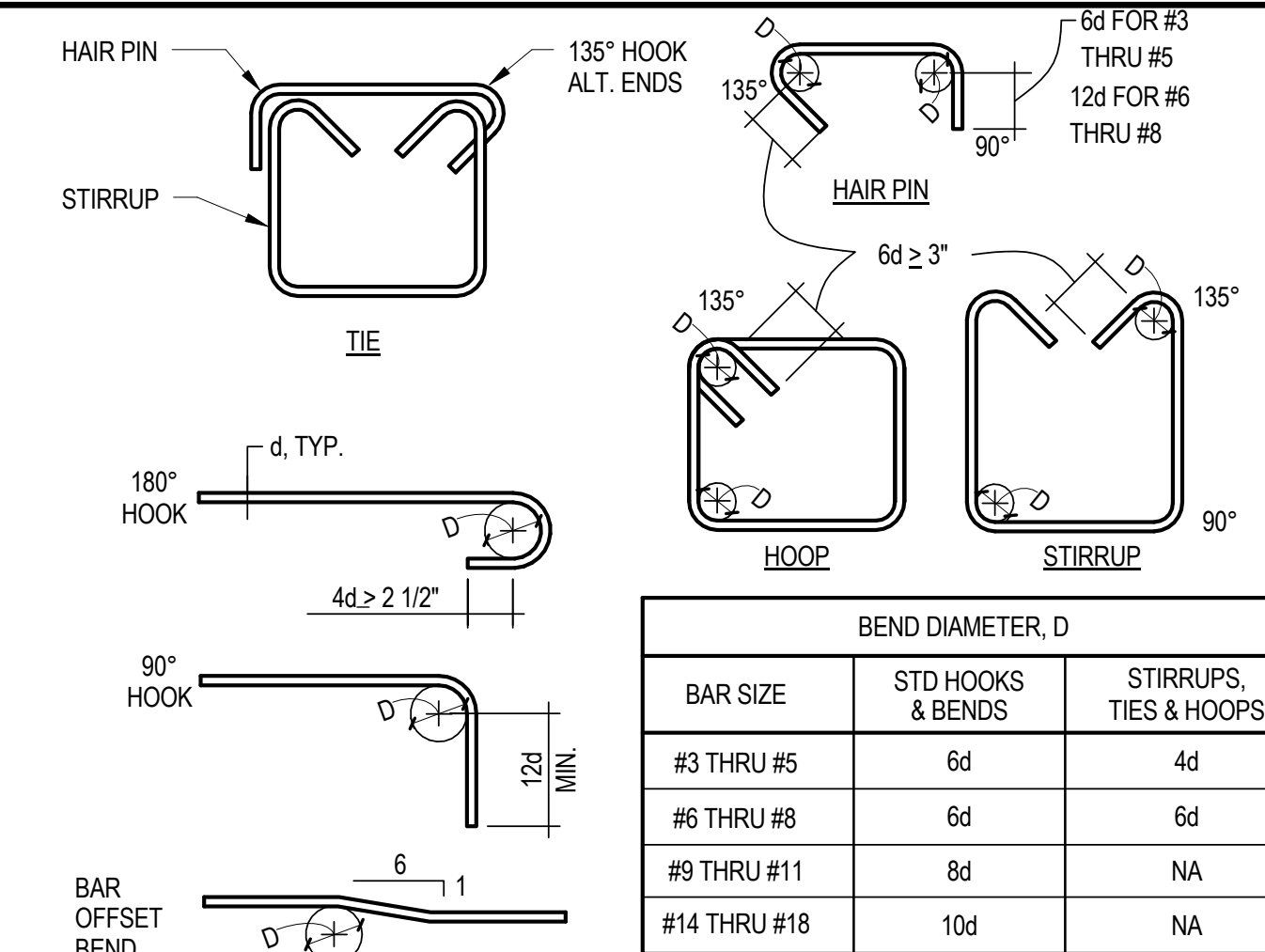
13 SOG DEPRESSION
N.T.S.



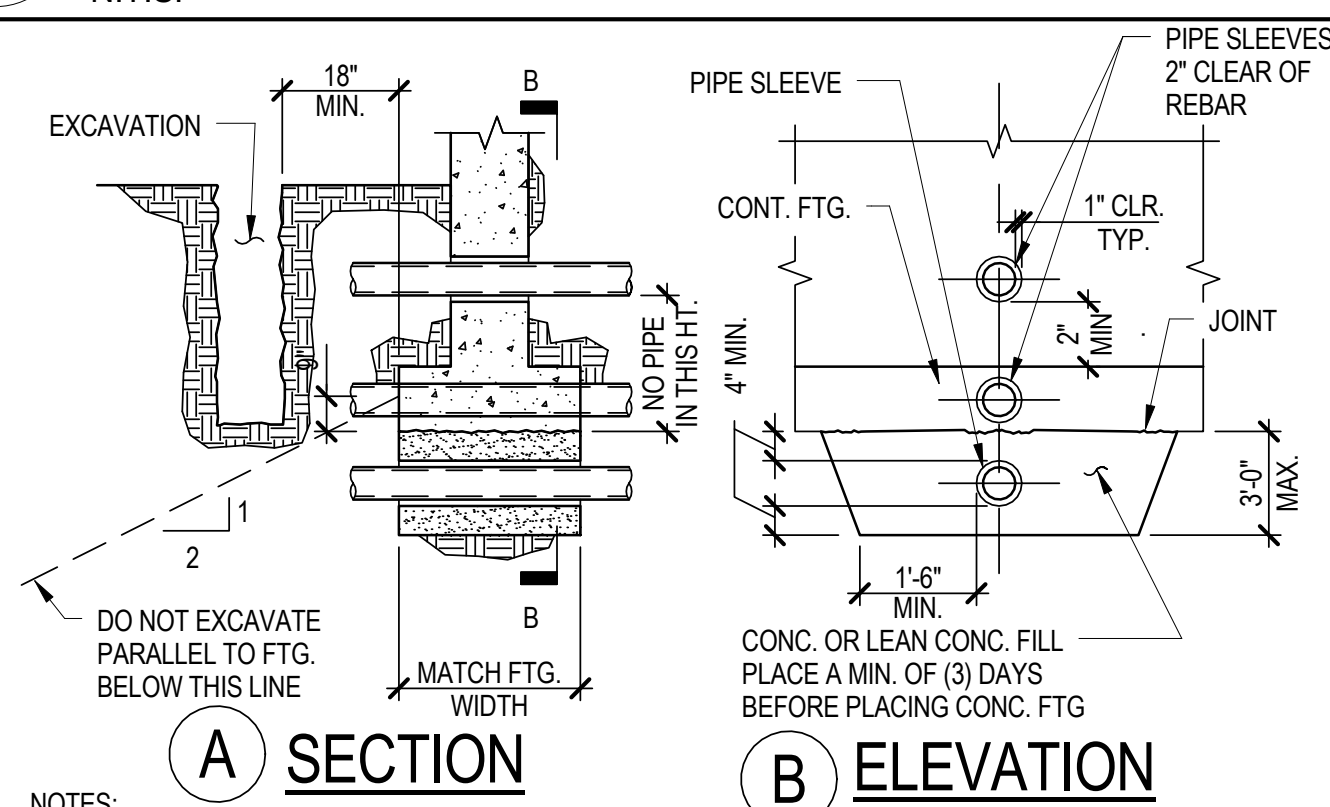
9 BAR SPACING IN CONCRETE



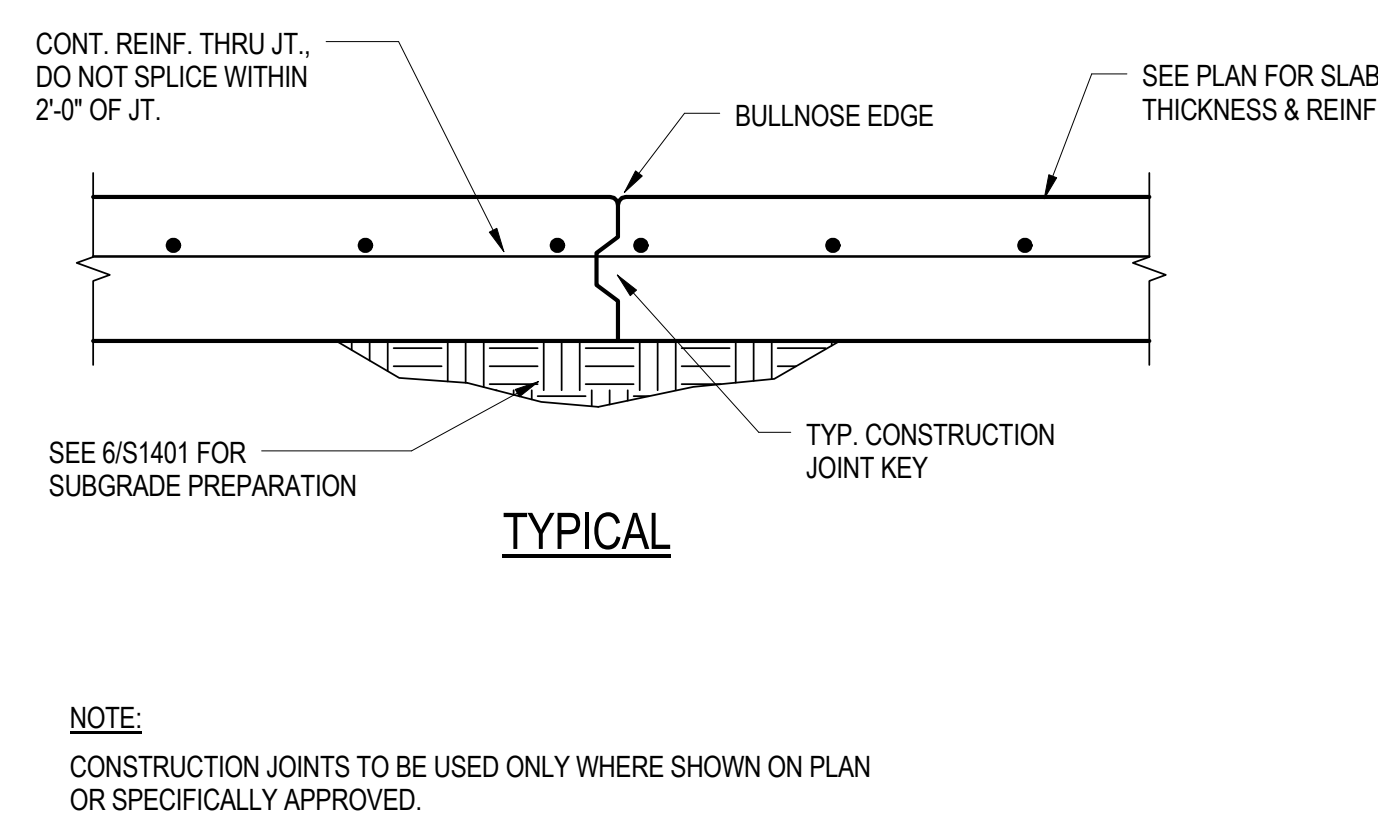
5 CONST. JOINTS
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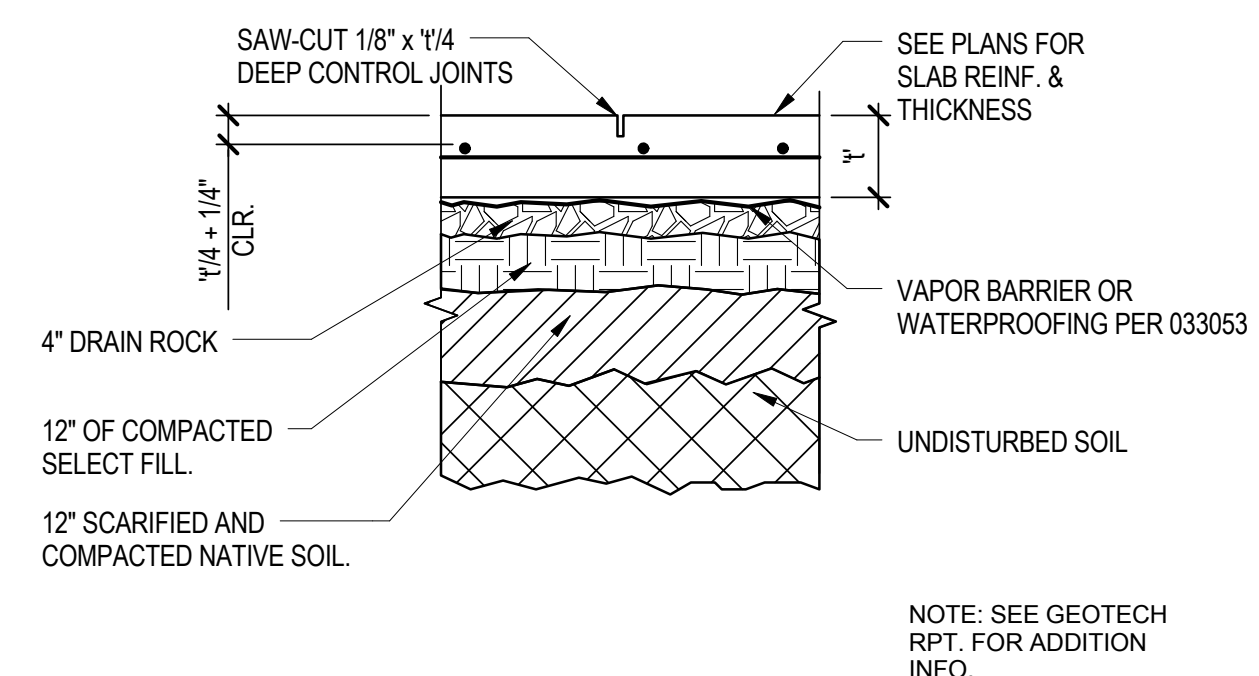
1 HOOKS & BENDS



18 SLEEVES AT FTG.
NTS

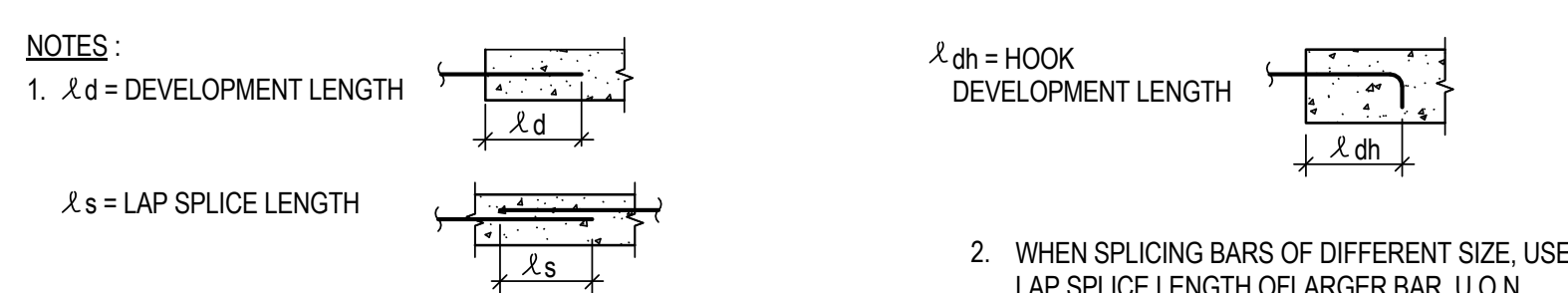


10 S.O.G. CONSTRUCTION JOINT
NTS

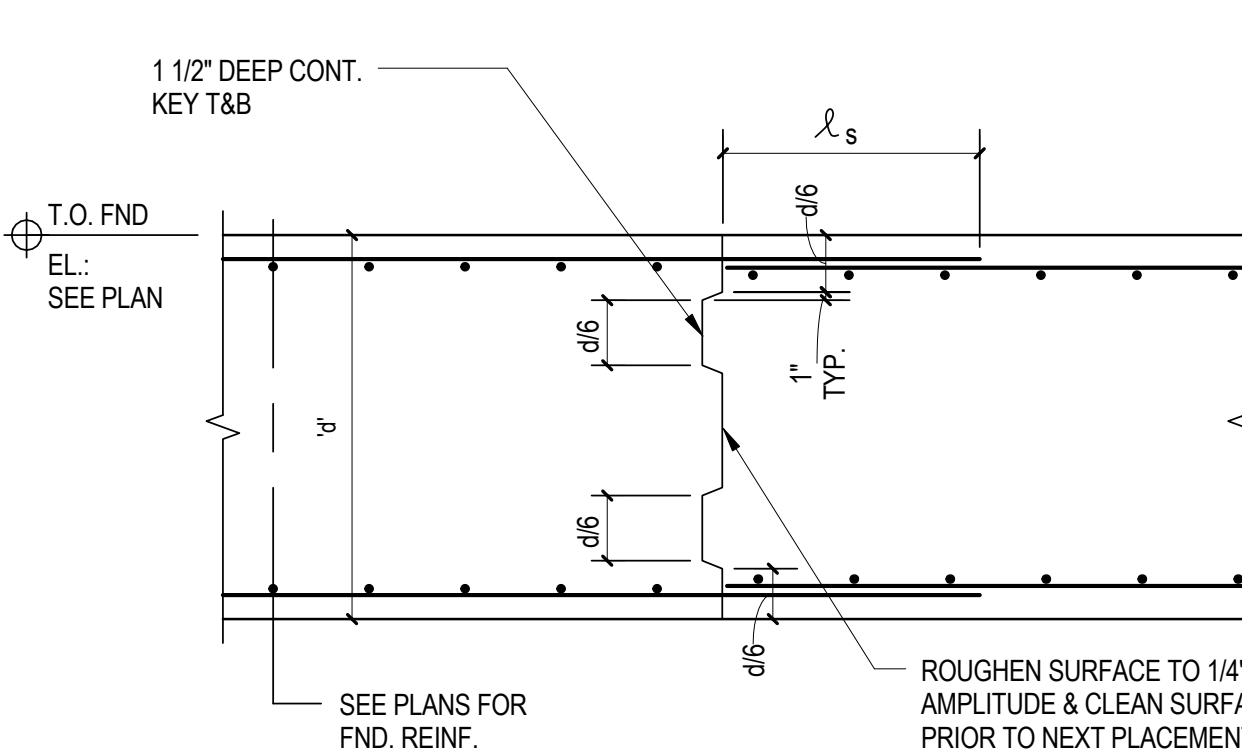
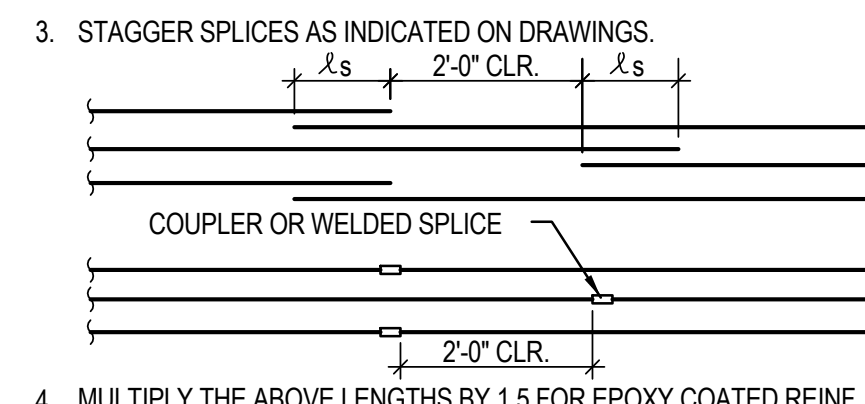


6 SOG & SUBGRADE PREP
N.T.S.

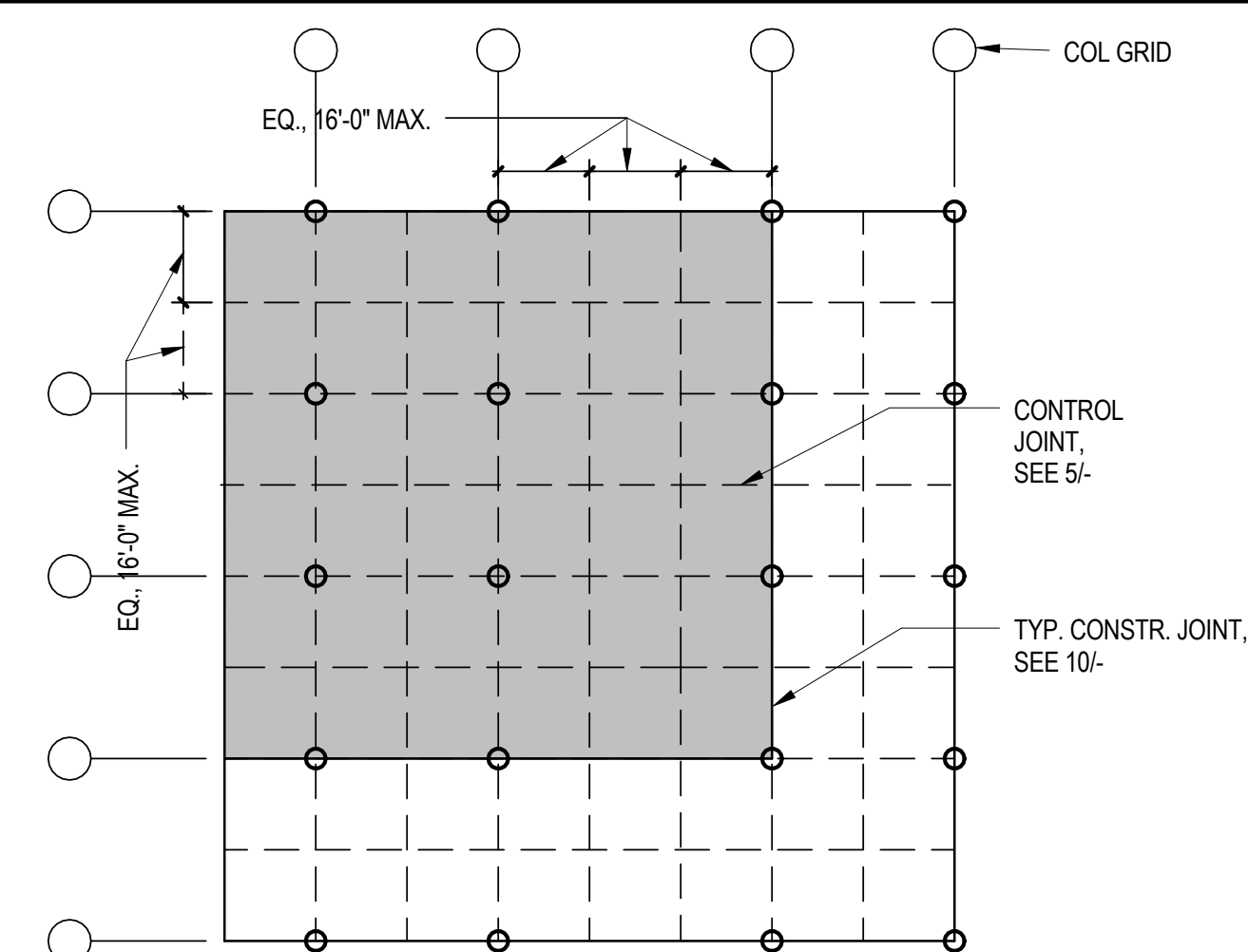
BAR LOCATION			CONCRETE		REINFORCING DEVELOPMENT AND SPLICE LENGTHS																															
					3		4		5		6		7		8		9		10		11															
			TYPE	MIN. STRENGTH	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh	ld	ls	ldh		
FOOTING TOP AND WALL HORIZONTAL REIN.			NWC	4 KSI	14	18	7	20	26	9	31	40	12	44	57	14	59	77	17	78	101	19	99	129	21	126	164	24	154	200	26	-	-	-	-	
FOOTING BOTTOM, WALL VERTICAL, SLAB ON GRADE, AND SLAB ON CHOCK			NWC	4 KSI	12	16	7	15	20	9	24	31	12	34	44	14	46	60	17	60	78	19	76	99	21	97	126	24	119	155	26	-	-	-	-	



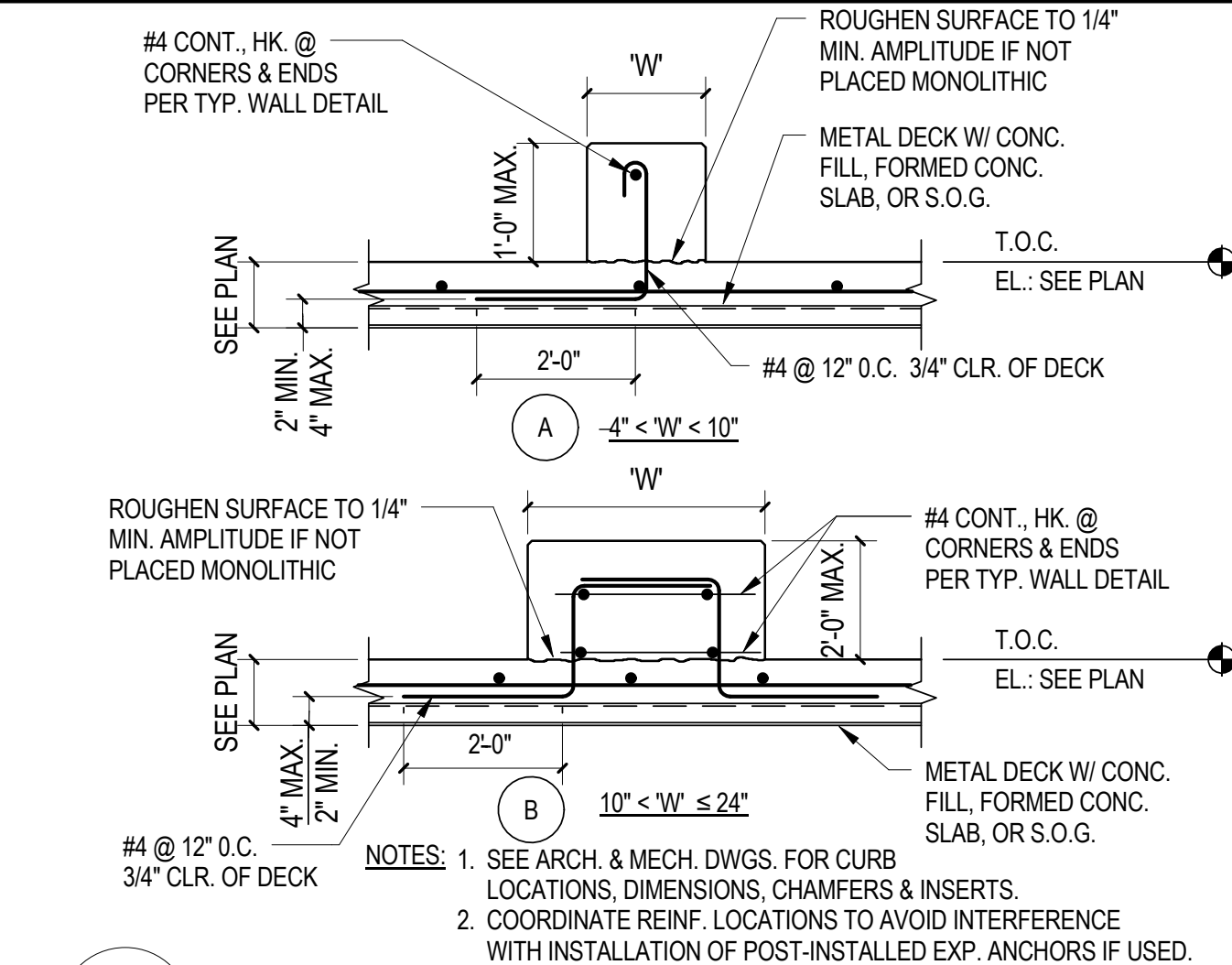
19 REINFORCEMENT DEVELOPMENT



11 FOUNDATION
CONSTRUCTION JOINT
N.T.S.

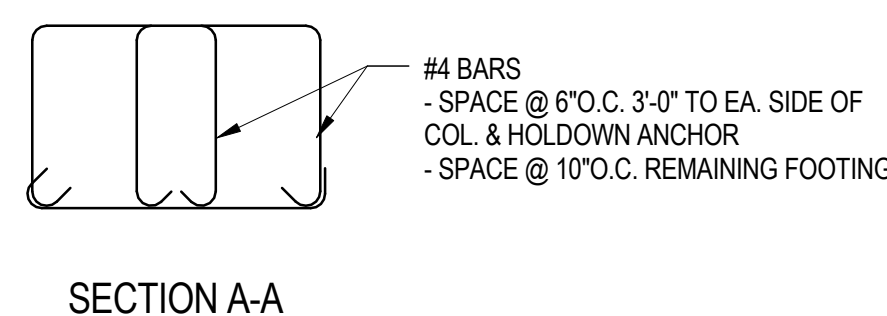


7 CONTROL JTS. IN SOG
N.T.S.



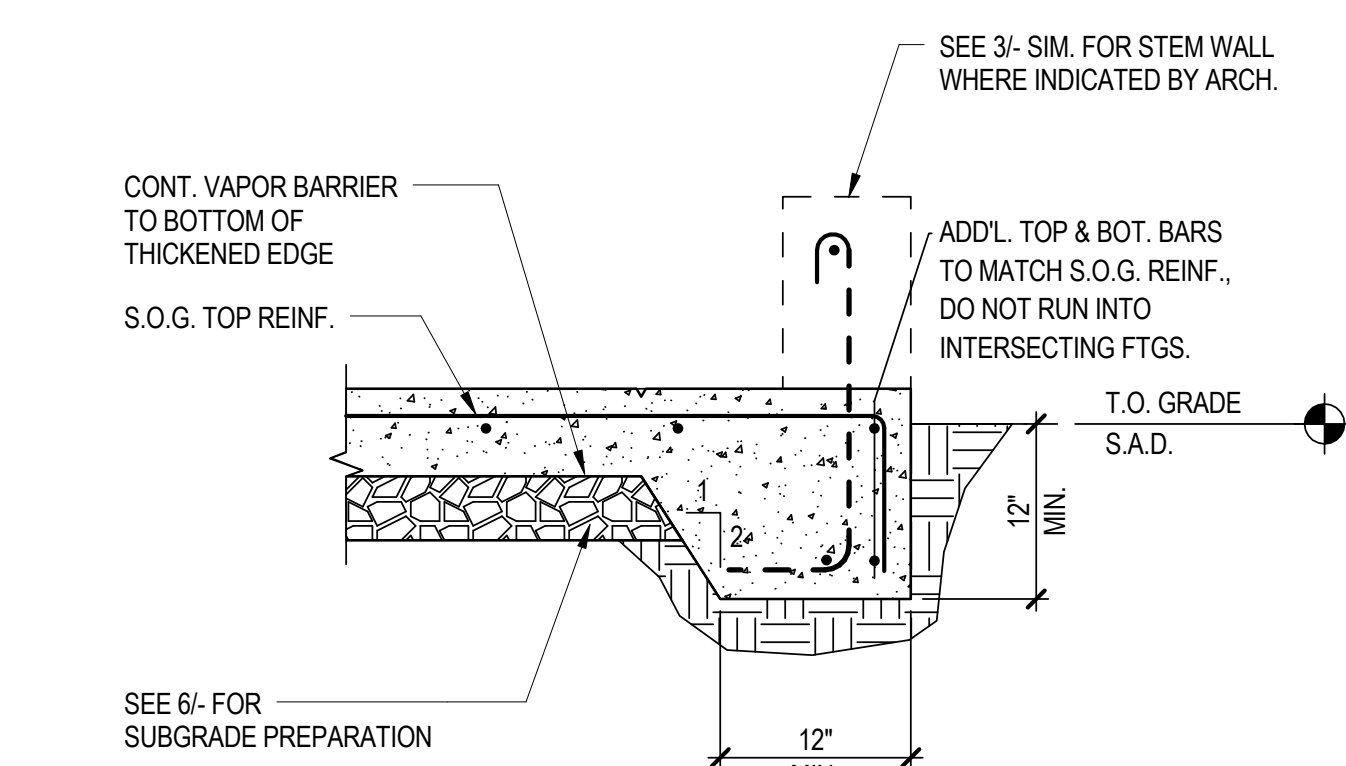
3 CONCRETE CURB

FOOTING SCHEDULE										
MARK	'H DEPTH	'B' LENGTH	'D' WIDTH	BOTTOM REINFORCING		TOP REINFORCING		SLAB DOWELS	DETAIL	REMARKS
				LONG.	TRANSVERSE	LONG.	TRANSVERSE			
CF1	2'-0"	N/A	1'-9"	4-#7	SEE A-A	4-#7	SEE A-A	#4 @ 18" O.C.	12'-	
CF2	2'-0"	N/A	2'-0"	6-#9	SEE A-A	6-#9	SEE A-A	#4 @ 18" O.C.	12'-	
CF3	2'-0"	N/A	2'-0"	6-#9	SEE A-A	6-#7	SEE A-A	#4 @ 18" O.C.	12'-	



20 FOOTING SCHEDULE

12 CONTINUOUS FOOTING DETAIL



4 SOG THICKENED 100% CD / BID SUBMISSION
EDGE FEBRUARY 2, 2016

[illegible]

CONSULTANTS:



ARCHITECT

POLYTECH ASSOCIATES INC.
235 Pine Street, 17th Floor
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Drawing Title	TYPICAL CONCRETE DETAILS
---------------	--------------------------

Approved: Project Director

Project Title	POST TRAUMATIC STRESS DIAGNOSIS (PTSD) EXPANSION & RENOVATION
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Location	795 WILLOW ROAD, MENLO PARK CA
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Project Number	640-Z35003
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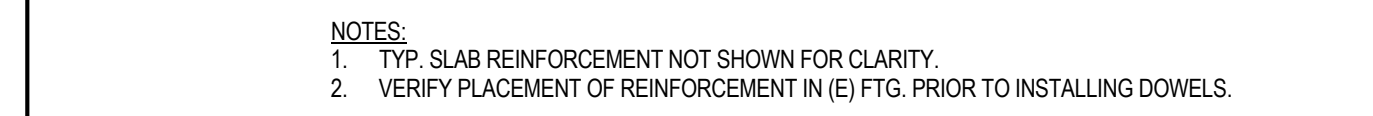
Building Number

Drawing Number

360-J-S401

Office of
Construction
and Facilities
Management



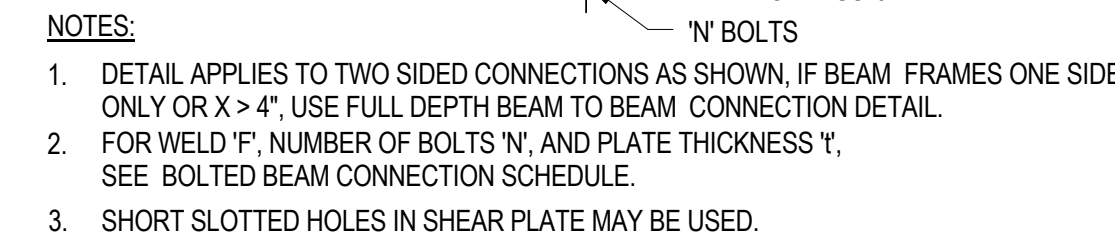




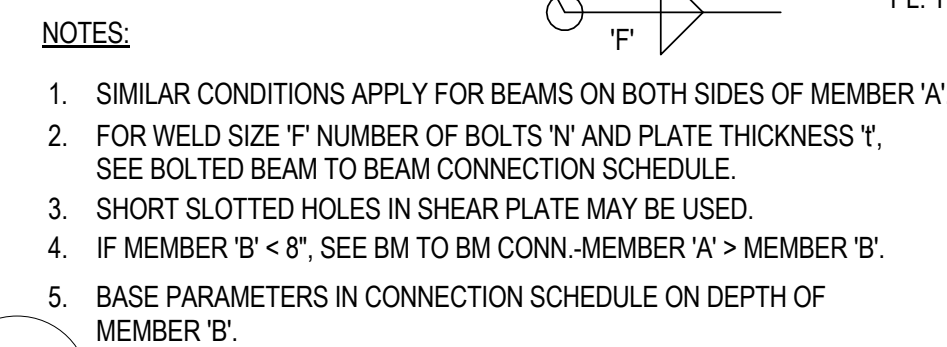
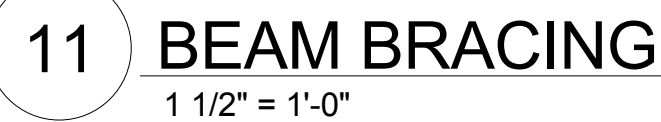
NOTES:

1. BOLT HOLES TO BE STD. HOLES 1/16" GREATER THAN BOLT DIAMETER. SHORT SLOTTED HOLES IN SHEAR PLATE MAY BE USED ANYWHERE NOTED.
2. BOLT'S TO BE FULLY PRETENSIONED W/ HARDENED WASHERS, U.O.N.
3. SCHEDULE BASED ON NOMINAL DEPTH OF WIDE FLANGE BEAMS, CHANNELS, TUBES, & OTHER MISCELLANEOUS SHAPES.

1 BOLTED BEAM SCHEDULE DETAIL



2 DETAIL



3 BM TO BM $A \leq B$
N.T.S.

[illegible]

Degenkolb

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INC

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Drawing Title	TYPICAL STEEL DETAILS
---------------	-----------------------

Approved: Project Director

Project Title	POST TRAUMATIC STRESS DIAGNOSIS (PTSD), EXPANSION & RENOVATION
---------------	---

Location	795 WILLOW ROAD, MENLO PARK CA
----------	--------------------------------

Date
11/03/2014

Checked
RG

Drawn
ED

Project Number	640-Z35003
Building Number	

Drawing Number

360-J-S501

Office of
Construction
and Facilities
Management



100% CD / BID SUBMISSION
FEBRUARY 2, 2016

A

three inches = one foot

one and one half inches = one foot

B

one and one half inches = one foot

one inch = one foot

one inch = one foot

one inch = one foot

one inch = one foot

D

one half inch = one foot

one half inch = one foot

one half inch = one foot

one half inch = one foot

one eighth inch = one foot

one eighth inch = one foot

one eighth inch = one foot

one eighth inch = one foot

one eighth inch = one foot

one eighth inch = one foot

one eighth inch = one foot

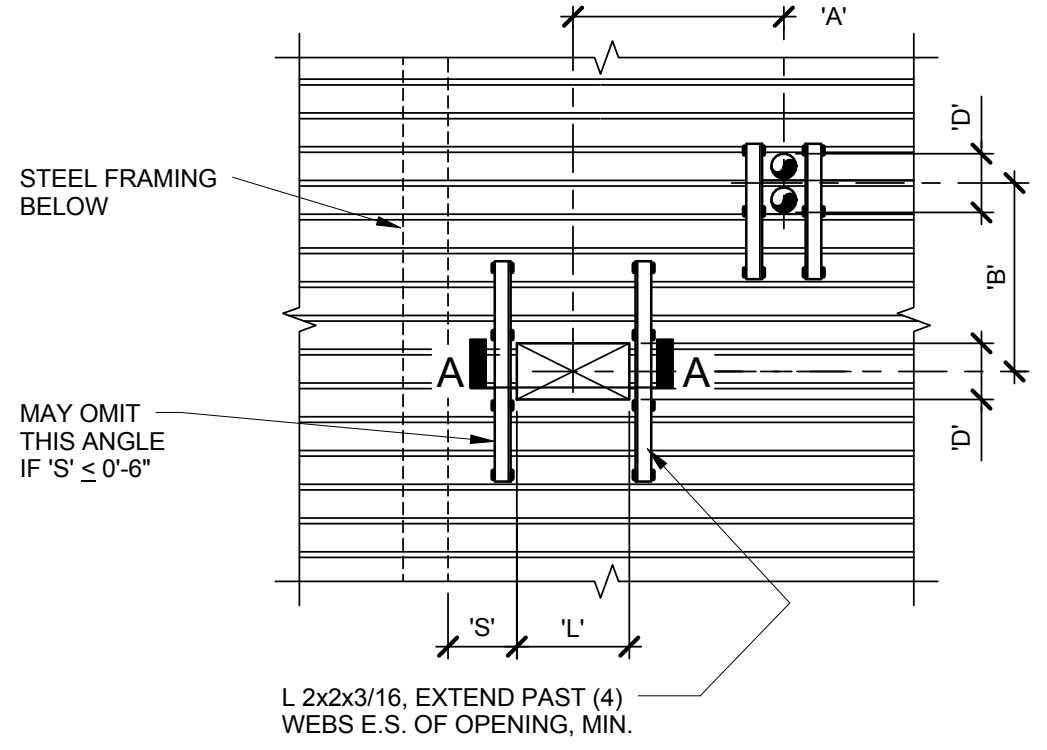
one eighth inch = one foot

one eighth inch = one foot

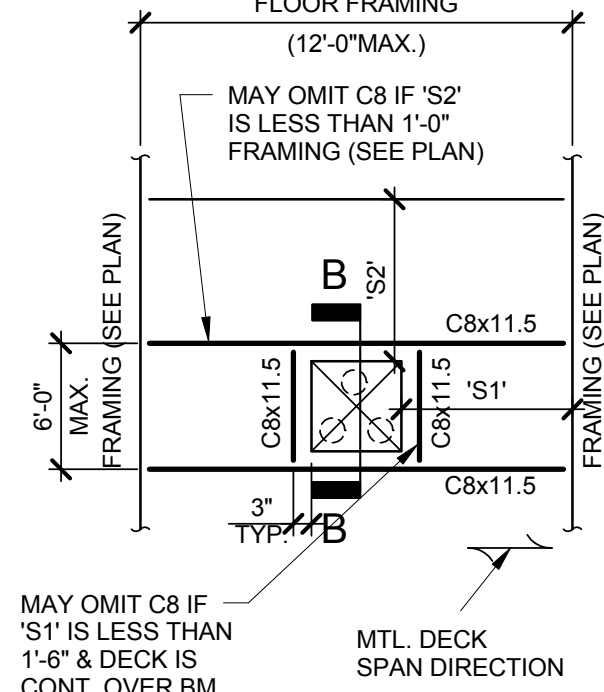
17

NON-COMPOSITE DECK OPENING REINFORCING

N.T.S.



A ANGLE REINF.

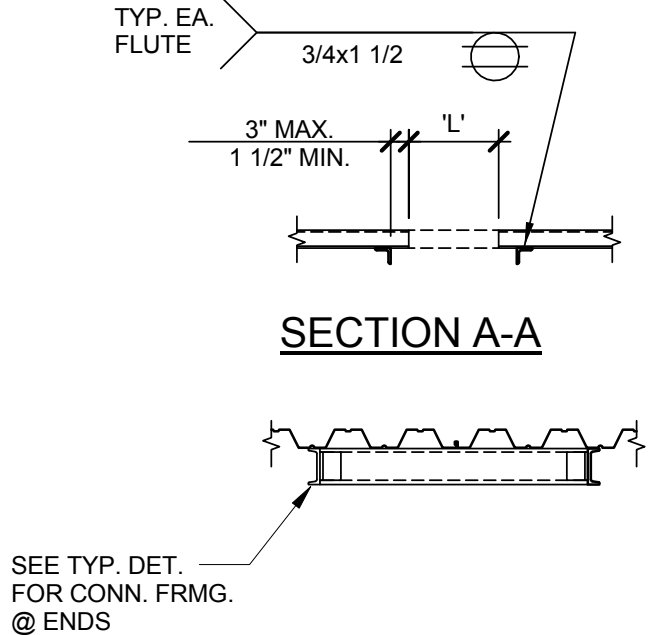


B FRAMED OPENING

- NOTES:
1. DETAIL 'B' APPLICABLE TO ALL OPENINGS.
 2. DETAIL 'A' APPLICABLE ONLY TO OPENINGS SATISFYING ALL OF THE FOLLOWING:
'D' ≤ 2'-0" (DIM. PERPENDICULAR TO DECK SPAN)
'L' ≤ 2'-0" (DIM. PARALLEL TO DECK SPAN)
'A' OR 'B' GREATER THAN 4D & 32", WHERE 'D' IS MAX. OF ADJACENT OPENINGS (DIMENSIONS TO ADJACENT OPENINGS).

SECTION A-A

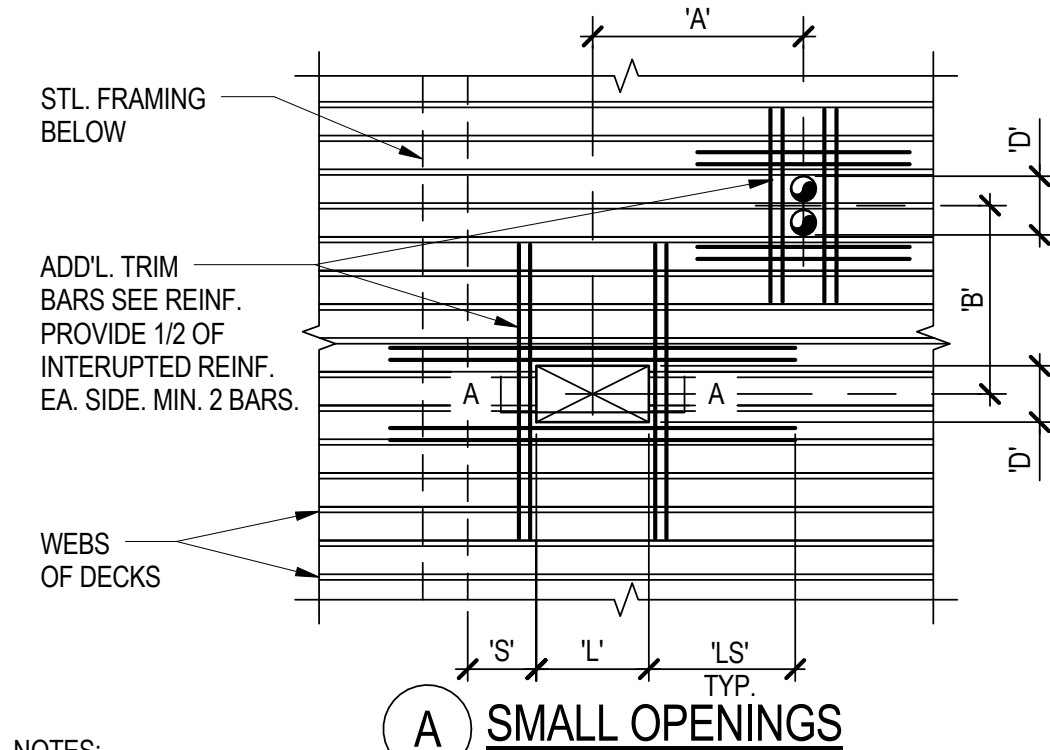
SECTION B-B



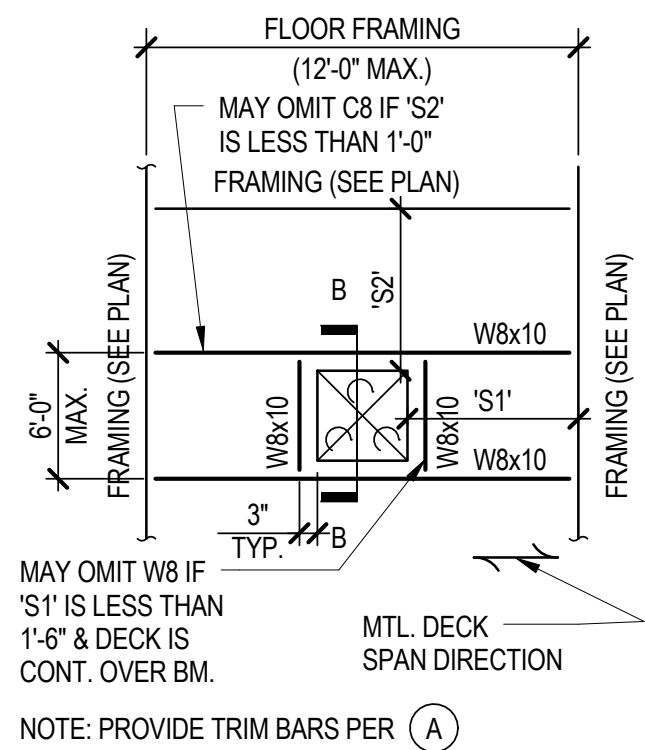
9

COMPOSITE DECK OPENING REINFORCING

N.T.S.

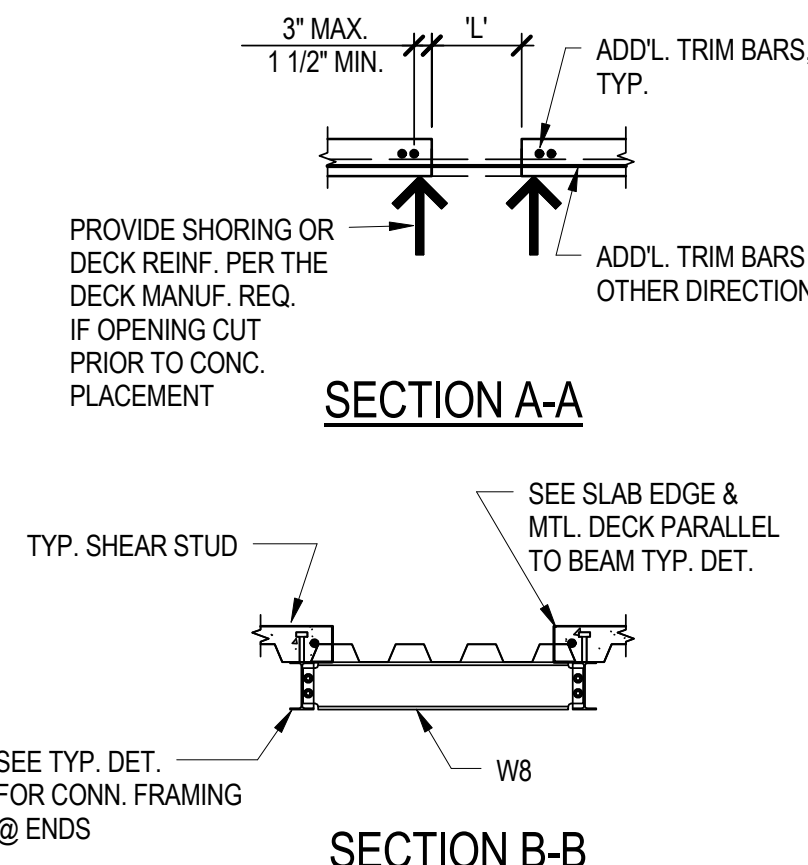


A SMALL OPENINGS



B FRAMED OPENING

- NOTES:
1. DETAIL 'B' APPLICABLE TO ALL OPENINGS.
 2. DETAIL 'A' APPLICABLE ONLY TO OPENINGS SATISFYING ALL OF THE FOLLOWING:
'D' ≤ 16" (DIM. PERPENDICULAR TO DECK SPAN)
'L' ≤ 4'-0" (DIM. PARALLEL TO DECK SPAN)
'A' OR 'B' GREATER THAN 4D & 32", WHERE 'D' IS MAX. OF ADJACENT OPENINGS (DIMENSIONS TO ADJACENT OPENINGS).
 3. HOLES UP TO 6" MAX. IN BOTH DIRECTIONS MAY OMIT TRIM REINF. PROVIDED NO MORE THAN ONE RIB IS CUT.



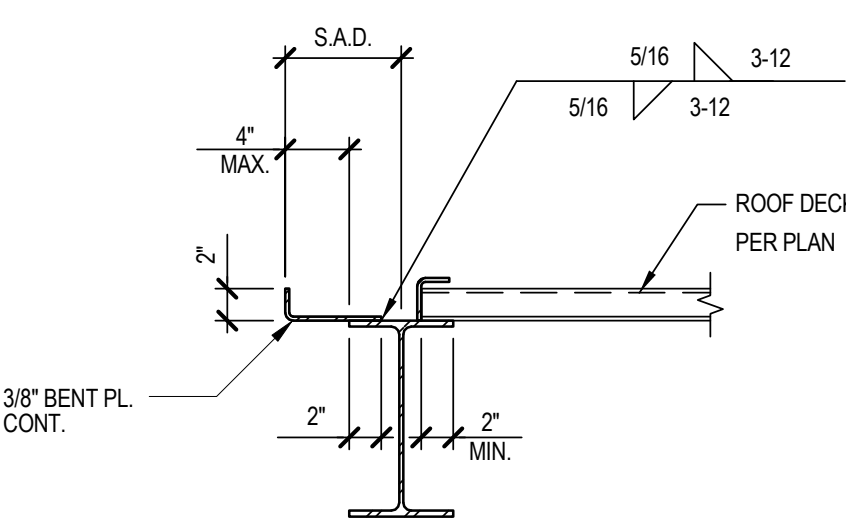
SECTION A-A

SECTION B-B

2

EDGE OF ROOF DECK

N.T.S.



- NOTES:
1. DETAIL SIMILAR FOR DECK SPAN PARALLEL TO EDGE OF DECK.

10

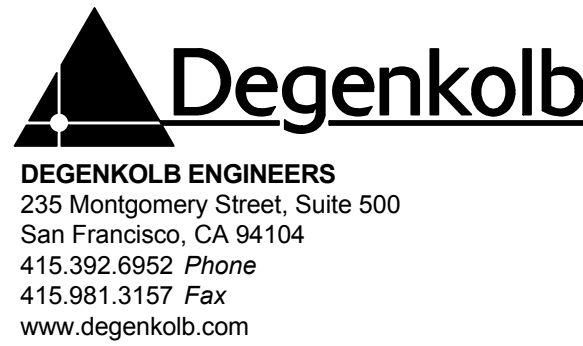
METAL DECK SCHEDULE

N.T.S.

SLAB TYPE	DECK TYPE	HEIGHT	GAUGE	FACTORY VENTED	MINIMUM SECTION PROPERTIES			FILL		STUD SIZE	SLAB REINFORCING	ATTACHMENT TYPE				SIDE LAP	REMARKS	
								TYPE	MINIMUM THICKNESS ABOVE TOP FLUTE			TO PERIMETER SUPPORT		TO INTERMEDIATE SUPPORT				
					I (IN ⁴)	S+ (IN ³)	S- (IN ³)					PERPENDICULAR TO DECK	PARALLEL TO DECK	PERPENDICULAR TO DECK	PARALLEL TO DECK			
S1	HSB	1 1/2"	18	NO	0.302	0.314	0.331	-	-	-			1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	1 1/2" TOP SEAM WELD @ 12" O.C.	NO MEP SUPPORTS TO DECK.
S2	W	3"	18	YES	1.203	0.763	0.767	NWC	2 1/2	3/4"Ø4 1/2"	#4 @ 12"O.C.E.W.	1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	BUTTON PUNCH	-	

- NOTES:
1. WHENEVER POSSIBLE, DECK LAYOUTS SHALL PROVIDE SHEETS OF SUFFICIENT LENGTH TO SPAN CONTINUOUSLY ACROSS AT LEAST THREE SPANS. ENDS SHALL TERMINATE OVER A SUPPORT PERPENDICULAR TO THE DECK SPAN, EXCEPT AT OPENINGS OR BUILDING EDGES WHERE DECKS MAY BE CANTILEVERED.
 2. PROVIDE A MINIMUM OF 2" BEARING AT SUPPORTING MEMBERS PERPENDICULAR TO DECK SPAN AND 1 1/2" AT MEMBERS PARALLEL TO DECK SPAN.
 3. DIAMETER OF PUDDLE WELD AND DIMENSION OF SEAM WELD SHOWN REPRESENTS EFFECTIVE FUSION AREA.
 4. EACH PUDDLE WELD SHOWN MAY BE REPLACED WITH A SHEAR STUD WELDED THROUGH DECK.
- CONCRETE FILL THICKNESS SHOWN ON FRAMING PLANS AND DETAIL SHEETS ARE MINIMUM THICKNESS. PROVIDE ADDITIONAL CONCRETE FILL AS REQUIRED TO COMPENSATE FOR BEAM OR DECK DEFLECTIONS AND TO MAINTAIN SURFACE TOLERANCES SPECIFIED.

CONSULTANTS:



ARCHITECT



Drawing Title
TYPICAL METAL DECK DETAILS

Approved: Project Director

Project Title
POST TRAUMATIC STRESS DIAGNOSIS (PTSD)
EXPANSION & RENOVATION

Location
795 WILLOW ROAD, MENLO PARK CA

Date
11/03/2014

Checked
RG

Drawn
ED

Project Number
640-235003

Building Number

Drawing Number

360-J-S601

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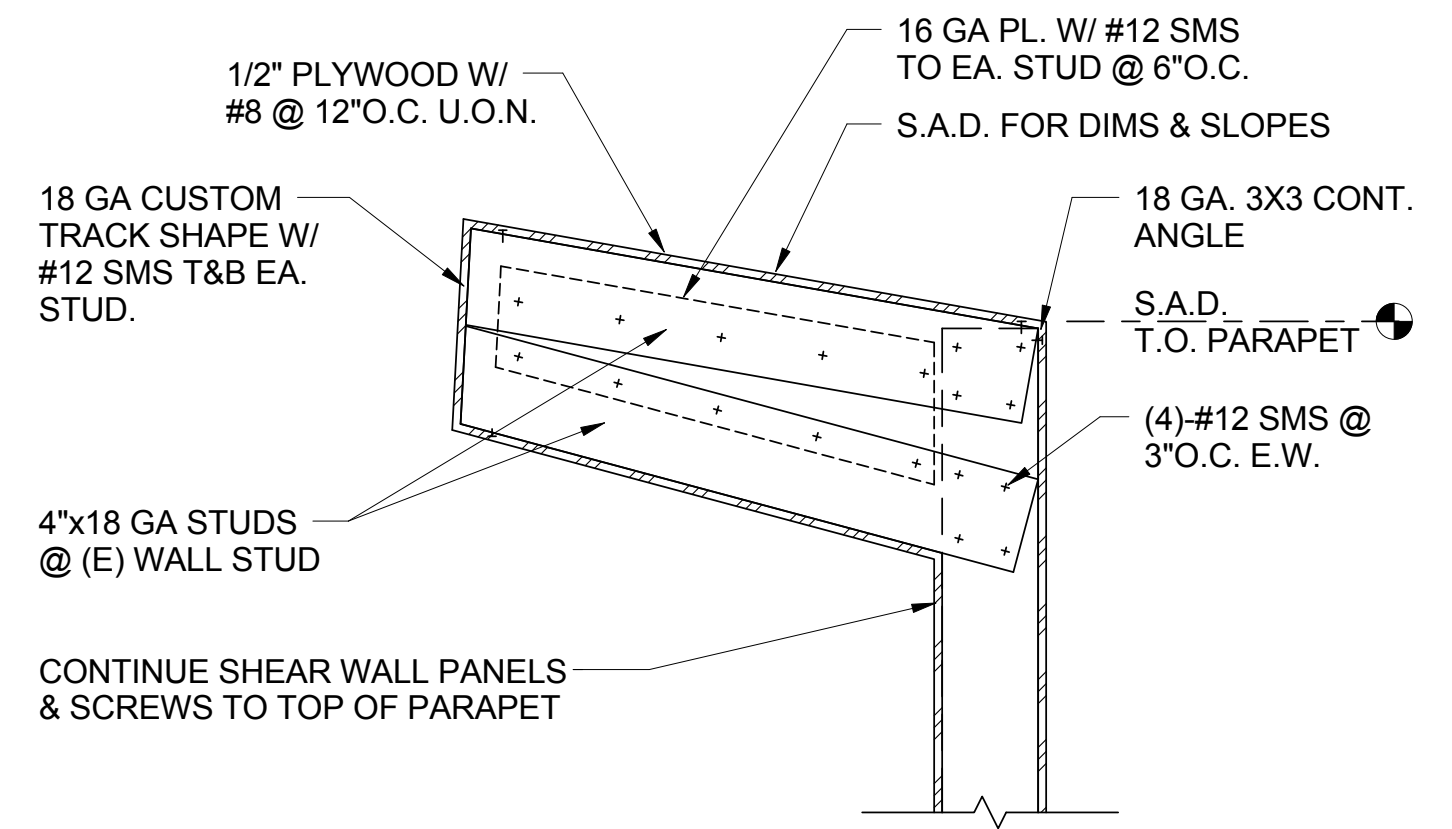
100% CD / BID SUBMISSION
FEBRUARY 2, 2016

SHEAR WALL SCHEDULE					
WALL MARK	SHEATHING	MIN. STUD THICKNESS (MILS)	EDGE SCREW SPACING	BOTTOM TRACK FASTENERS	REMARKS
W1	1532" STRUCTURAL 1 SHEATHING - 4 PLY, ONE FACE	54	#8 SMS @ 2" O.C.	5/8" @ 12" O.C. SPACING	
W2	1532" STRUCTURAL 1 SHEATHING - 4 PLY, ONE FACE	54	#8 SMS @ 3" O.C.	5/8" @ 16" O.C. SPACING	SHEATH AND SCREW FULL WALL ON GRID B (ABOVE AND BELOW WINDOWS INCLUDED)
W3	1532" STRUCTURAL 1 SHEATHING - 4 PLY, ONE FACE	54	#8 SMS @ 6" O.C.	5/8" @ 24" O.C. SPACING	
W4	1532" STRUCTURAL 1 SHEATHING - 4 PLY, ONE FACE	54	#8 SMS @ 4" O.C.	5/8" @ 20" O.C. SPACING	BOTTOM TRACK FASTENERS POST-INSTALLED W/ EPOXY
W5	1532" STRUCTURAL 1 SHEATHING - 4 PLY, ONE FACE	54	#8 SMS @ 12" O.C.	5/8" @ 48" O.C. SPACING	ALL EXTERIOR WALLS NOT SPECIFICALLY NOTED AS W# ON PLAN

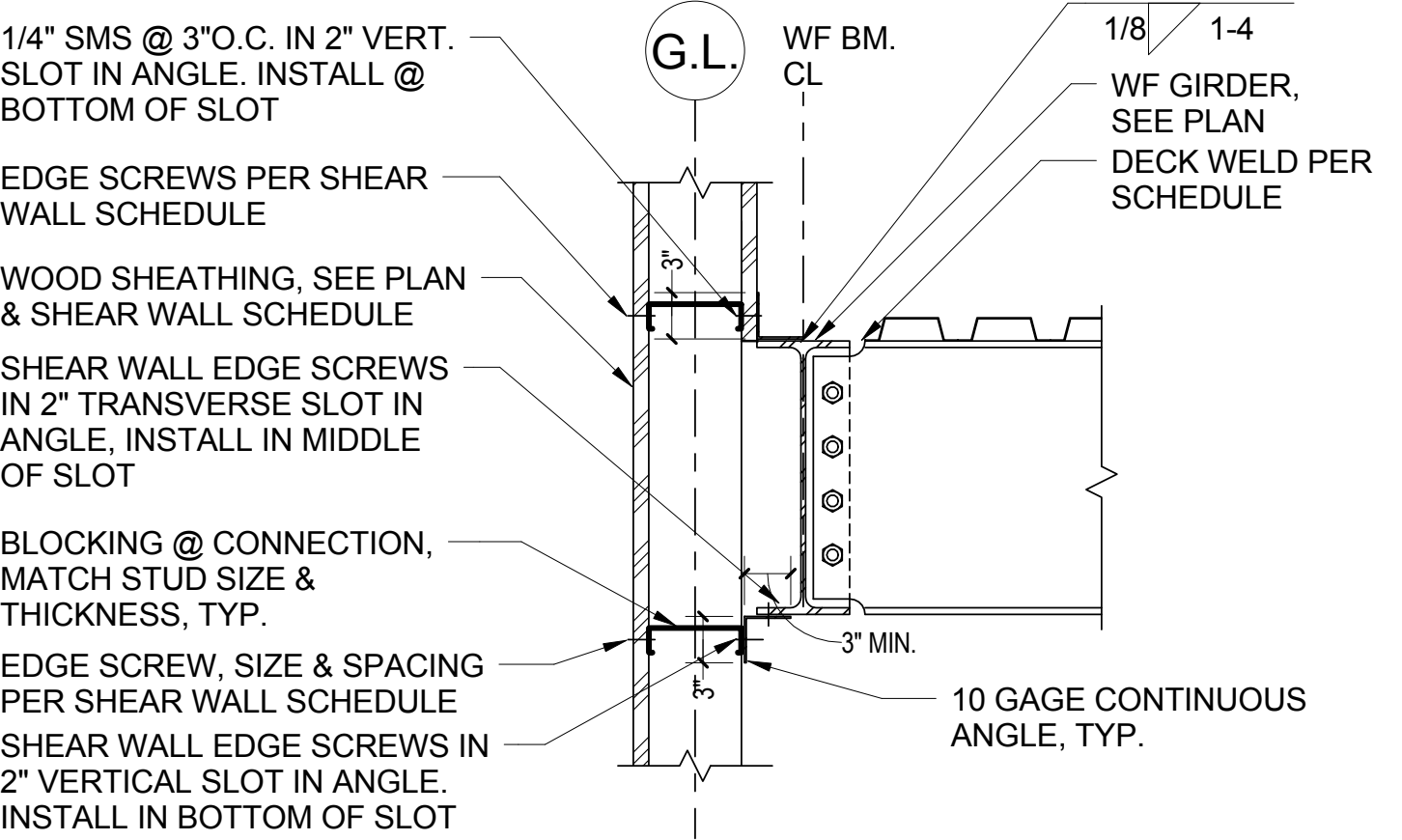
NOTES:

- WOOD STRUCTURAL PANELS SHALL COMPLY WITH DOC P51 OR PS2.
- TOP AND BOTTOM TRACK SHALL MATCH STUD SIZE AND THICKNESS.
- BLOCK ALL PANEL EDGES, BLOCKING SHALL MATCH STUD SIZE AND THICKNESS.
- SEE PLAN AND METAL STUD WALL SCHEDULE FOR STUD SIZE AND THICKNESS REQUIREMENTS FOR BRAEING WALLS. THICKNESS IDENTIFIED IN SHEAR WALL SCHEDULE SHALL BE CONSIDERED A MINIMUM.
- SCREWS SHALL BE SELF-DRILLING / SELF-TAPPING BUGLE HEAD.
- SEE DETAIL 15/- FOR SHEAR WALL ELEVATION.
- USE HOLDOWN INDICATED ON PLAN. SEE DETAIL 7/- FOR MORE INFORMATION.
- FOR BOTTOM TRACK FASTENERS, EXPANSION ANCHORS PER SCHEDULE REQUIRED AT CONNECTION TO CONCRETE SING REQUIRED AT ALL OTHER LOCATIONS.
- SEE MECHANICAL AND ADHESIVE NOTE 3/S001 FOR POST-INSTALLED BOTTOM TRACK FASTENERS INTO EXISTING CONCRETE.

18 SHEAR WALL SCHEDULE
N.T.S.



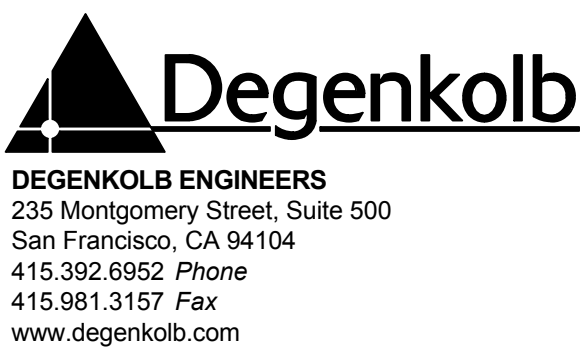
19 PARAPET FRAMING
1" = 1'-0"



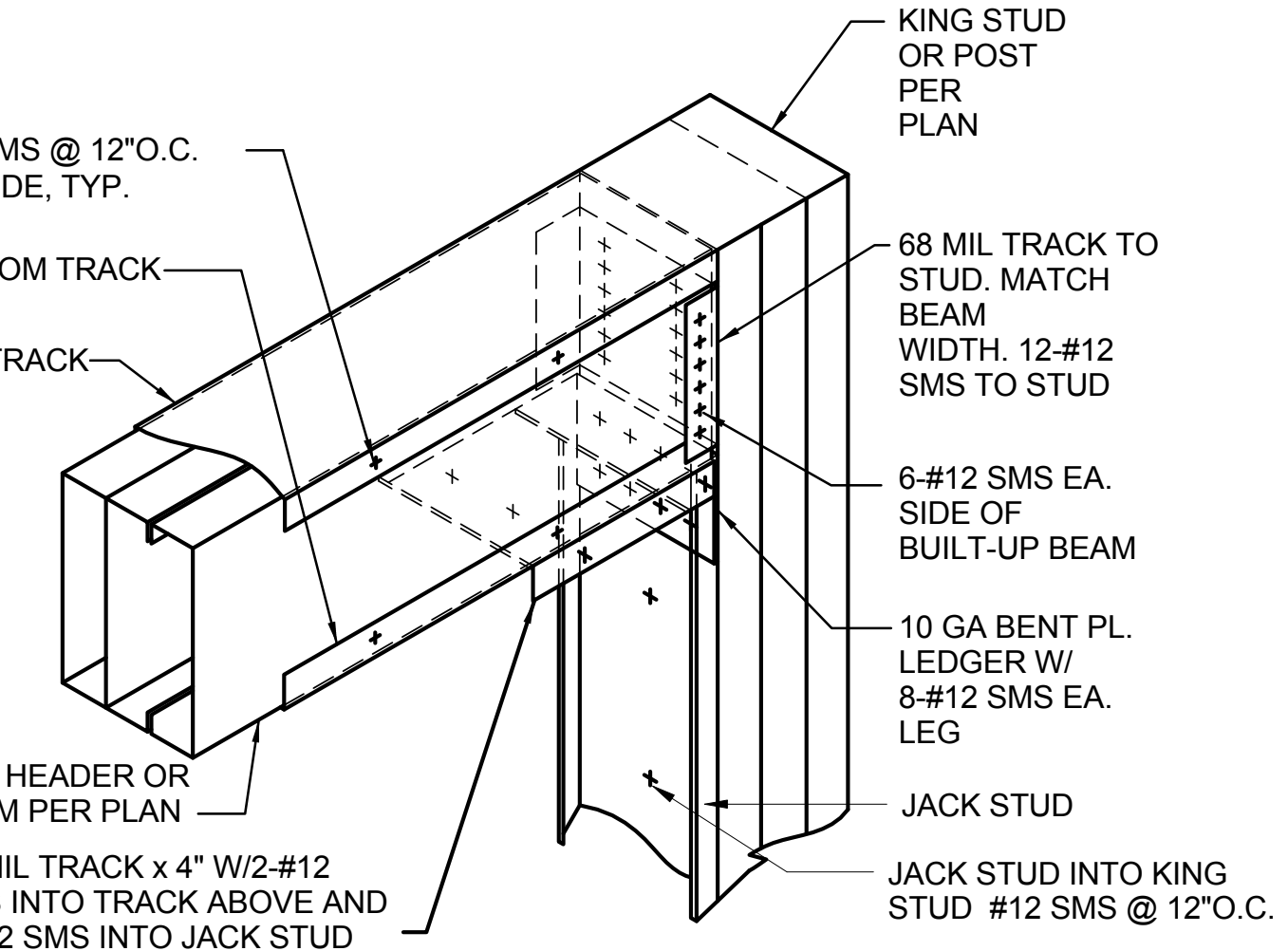
20 WALL TO ROOF CONN.
N.T.S.

100% CD / BID SUBMISSION	02/02/16
60% SUBMISSION	03/16/15
Revisions:	Date

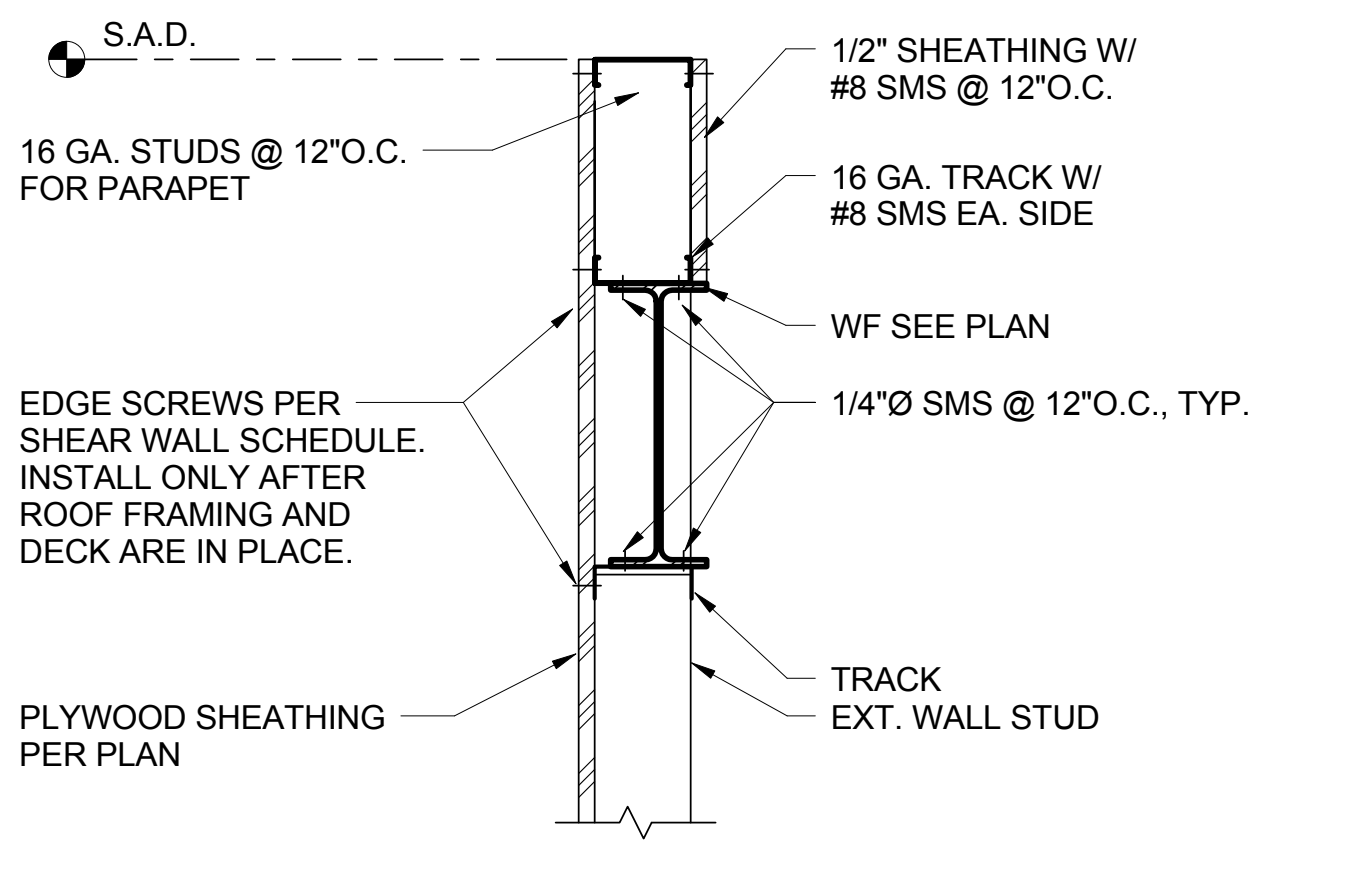
CONSULTANTS:



14 EXTERIOR WALL/SHEAR WALL FRAMING
1/2" = 1'-0"



15 BEAM BOX CONN. TO POST
N.T.S.



16 EXT. WALL TO WF BEAM ADJ. TO (E) BLDG.
N.T.S.



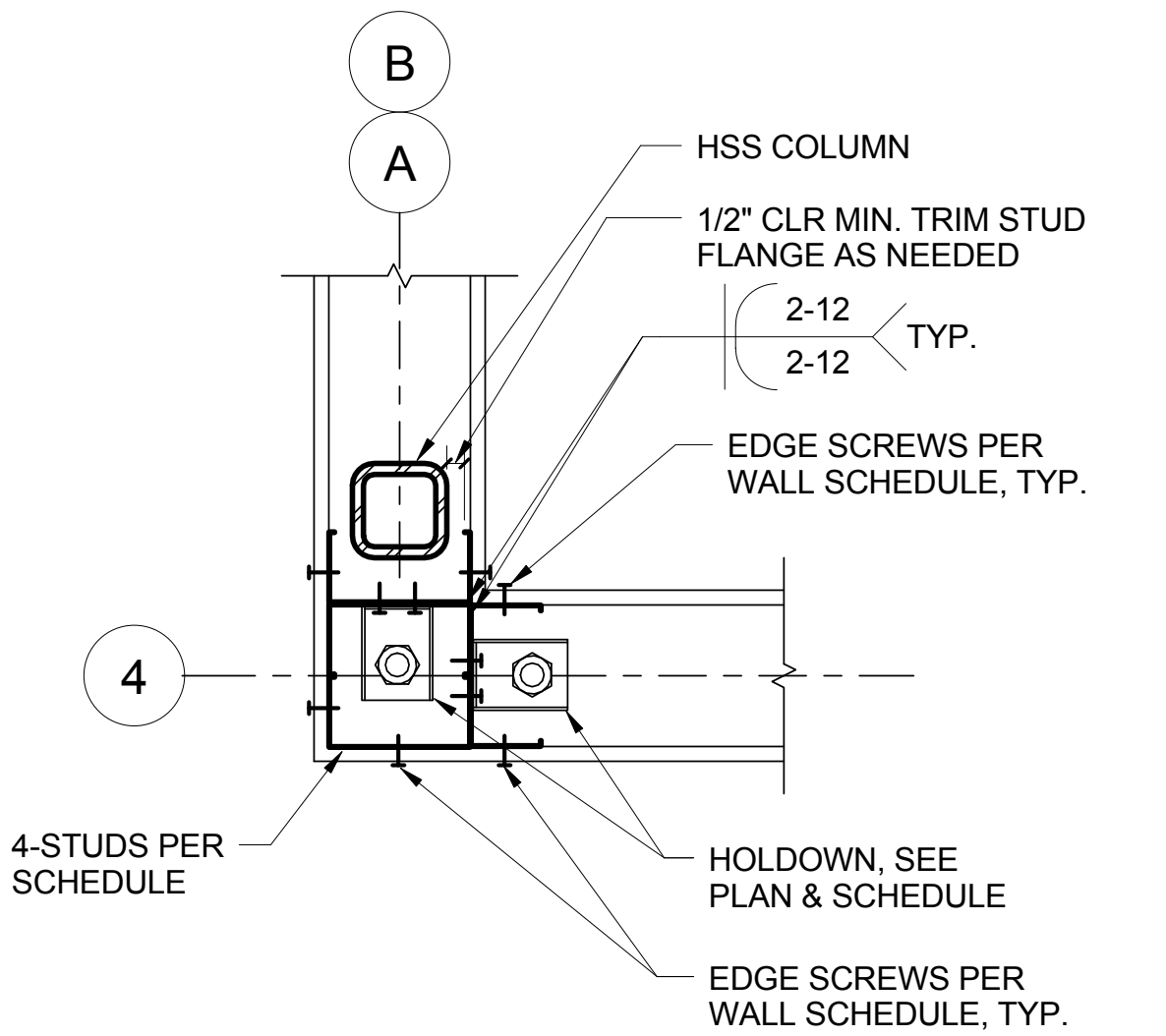
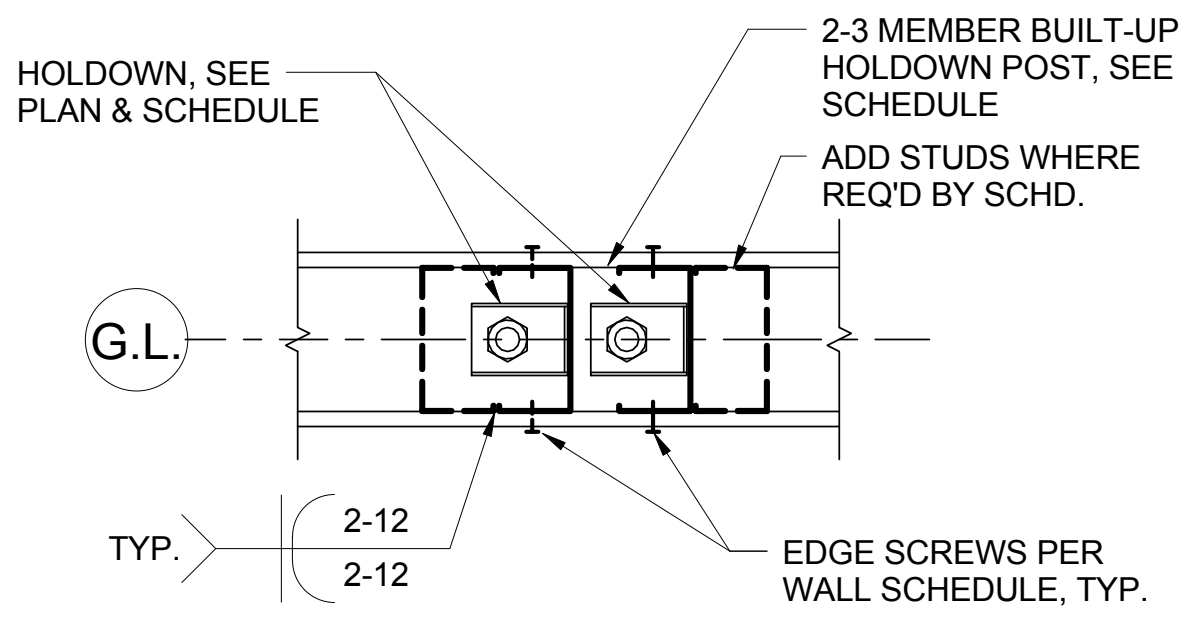
12 BOX HEADER SCHEDULE
N.T.S.

BOX HEADER SCHEDULE		
WALL	OPENING SIZE	
	≤ 8'-0"	≤ 12'-0"
6"	(2) 600S200-54	(2) 1000S200-54

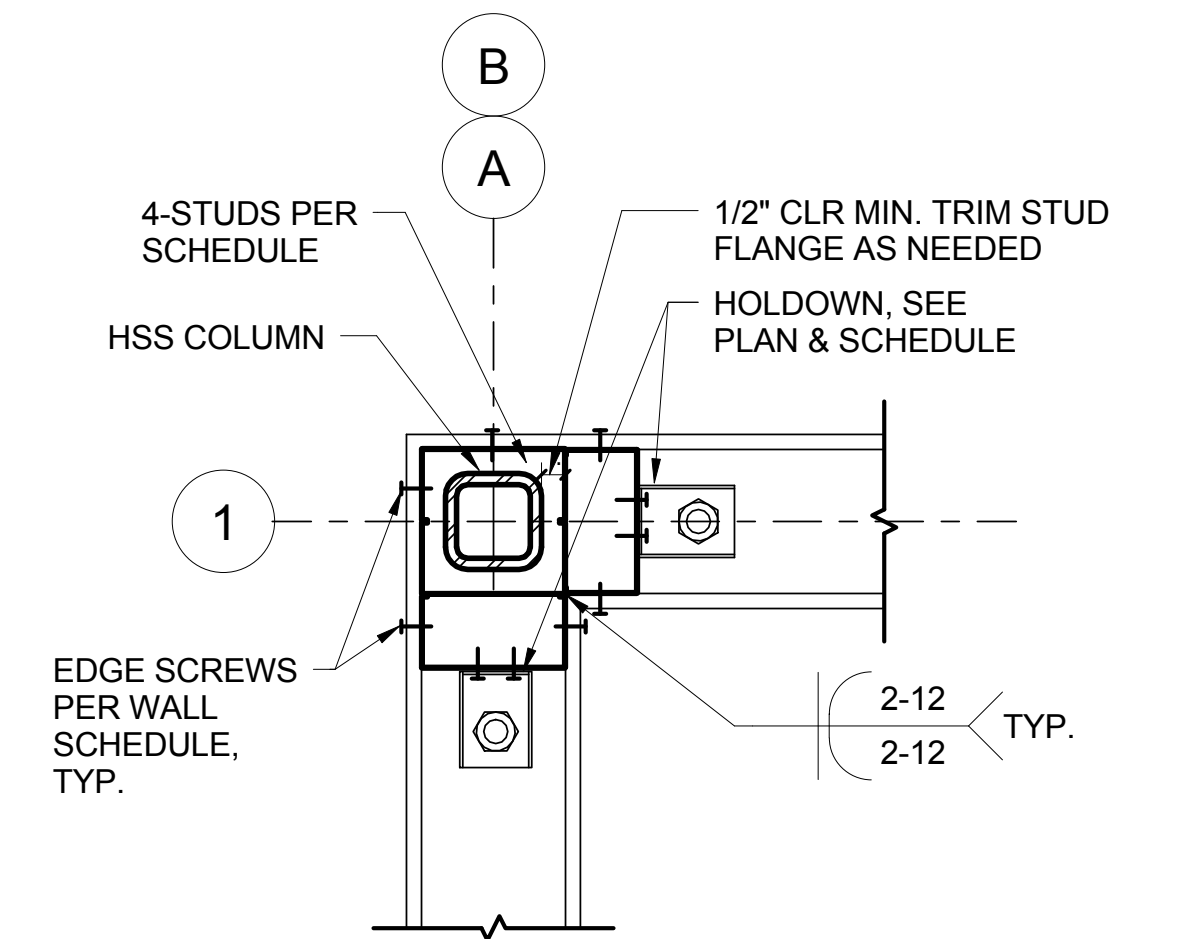
KING STUD SCHEDULE				
	OPENING SIZE			
	3'-0"	5'-0"	7'-0"	9'-0"
NUMBER OF FIELD STUDS	2	3	4	5

ARCHITECT

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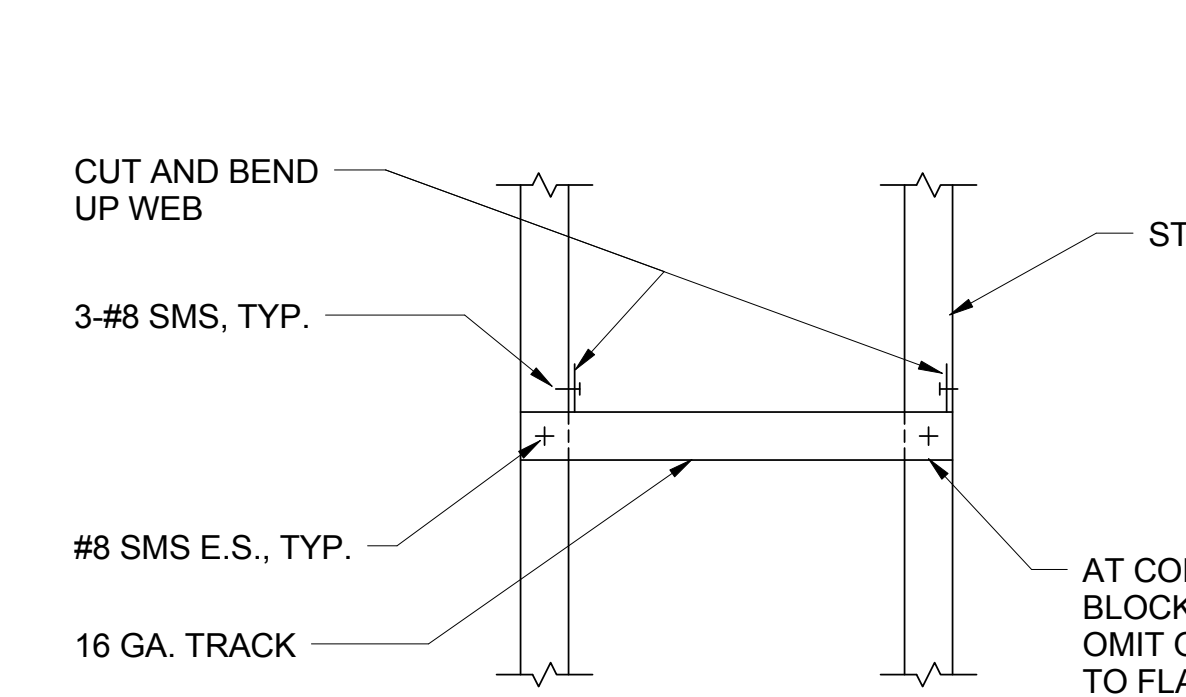


DETAIL B

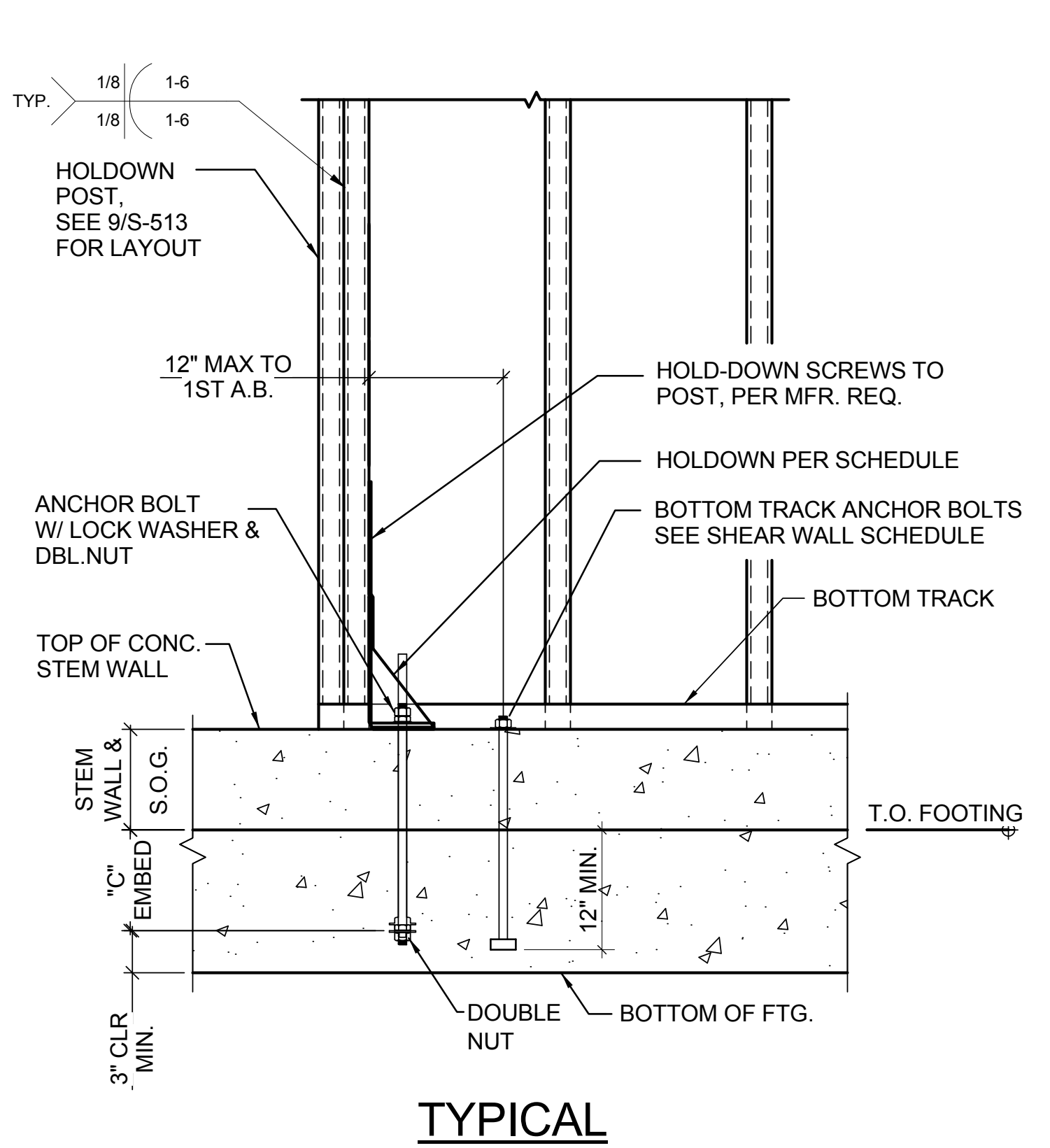
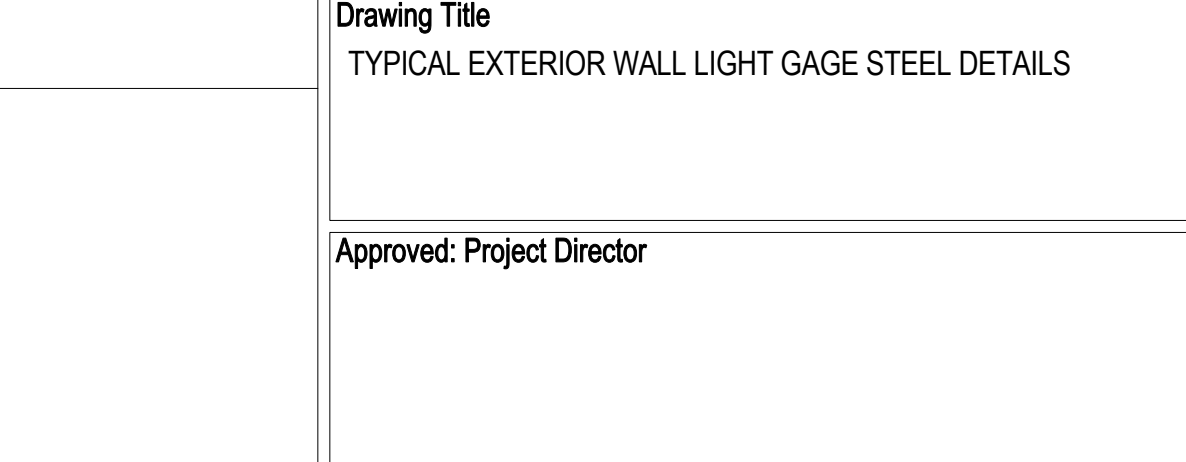


DETAIL C

7 HOLD DOWNS
N.T.S.



8 TYPICAL BLOCKING
1 1/2" = 1'-0"



HOLDOWN SCHEDULE					
HD #	TYPE	BOLT DIA.	HOLDOWN POST	"C" EMBED	PLAN DETAIL
1	2-S/HD10S	7/8"	4-600S300-54 (5/8" lip)	16"	DETAIL A
2	2-S/HD8S	7/8"	2-600S300-54 (5/8" lip)	16"	DETAIL A
3	2-S/HD10S	7/8"	4-600S300-54 (5/8" lip)	16"	DETAIL B
4	2-S/HD10S	7/8"	4-600S300-54 (5/8" lip)	16"	DETAIL C
5	2-S/HD8S	7/8"	4-600S300-54 (5/8" lip)	16"	DETAIL C
6	15/HD10S	7/8"	2-600S200-54 (5/8" lip)	16"	TYP.

100% CD / BID SUBMISSION
FEBRUARY 2, 2016

Drawing Title
TYPICAL EXTERIOR WALL LIGHT GAGE STEEL DETAILS

Approved: Project Director

Project Title
POST TRAUMATIC STRESS DIAGNOSIS (PTSD)
EXPANSION & RENOVATION

Location
795 WILLOW ROAD, MENLO PARK CA

Date
11/03/2014

Checked
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Drawn
ED

Project Number
640-235003

Building Number

Drawing Number

360-J-S801

Office of
Construction and Facilities
Management

