GENERAL NOTES

1. SHELTER DESIGN
   - A. THIS SHELTER HAS BEEN DESIGNED AS AN OPEN STRUCTURE. THE ADDITION OF ANY ENCLOSURE SUCH AS WALLS, INSECT MESH, OR SHADE SCREENS SHALL BE PROHIBITED AS INCREASED WIND FORCES MAY RESULT.
2. FOUNDATION
   - A. THE FOUNDATION SHALL REST ON SOUND SOIL THAT IS FREE OF ORGANIC AND DELETERIOUS MATERIALS AND CAPABLE OF SUPPORTING 1000 PSF VERTICAL BEARING PRESSURE.
   - B. FOUNDATION DESIGN SHOWN IS A RECOMMENDATION ONLY. OWNER SHALL VERIFY ACTUAL SOIL CONDITIONS AT EACH JOB SITE AND ANY REQUIRED ADJUSTMENTS TO THE FOOTING DESIGN SHALL BE DESIGNED BY OTHERS.
3. CONCRETE
   - A. COMPRESSION STRENGTH OF ALL REINFORCED CONCRETE SHALL NOT BE LESS THAN 2500 PSI AT 28 DAYS.
   - B. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO THE REQUIREMENTS OF MINIMUM ASTM A615 GRADE 40 FOR #4 AND SMALLER BARS AND GRADE 60 FOR BARS LARGER THAN #4.
   - C. MINIMUM CONCRETE CLEAR COVER FOR REINFORCING BARS SHALL BE 3".
   - D. ANCHOR BOLTS IN CONCRETE SHALL BE ASTM F1554, GRADE 36 HEX HEAD BOLTS.
4. STRUCTURAL STEEL
   - A. STEEL PLATE SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36
   - B. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500, GRADE B.
   - C. HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A325.
   - D. WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY'S SPECIFICATION FOR THE MATERIAL BEING WELDED.
   - E. WELDING ELECTRODES SHALL BE E70XX.
   - F. STEEL FRAMING SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TGIC POWDER COAT FINISH MEETING AAMA 2604-02 SPECIFICATION.
5. STEEL ROOF DECK
   - A. INTERLOCKING SEAL GALVALUME ROOF DECK SHALL BE ROLL FORMED FROM ASTM A792, GRADE 50 STEEL AND SHALL CONFORM TO THE DECK PROFILE SHOWN ON THE DRAWINGS.
   - B. ROOF DECK SHALL BE COATED WITH HEAT REFLECTIVE BASF ULTRA-COOL COATING OR APPROVED EQUAL.
6. ALUMINUM
   - A. EXTRUDED ALUMINUM RIDGE CAP, AND GUTTER SHALL BE FABRICATED FROM ALUMINUM ALLOY 6105-T5 AND SHALL CONFORM TO THE REQUIREMENTS SHOWN ON THE DRAWING.
   - B. EXTRUDED ALUMINUM RIDGE CAP, AND GUTTER SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TGIC POWDER COAT FINISH MEETING AAMA 2604-02.
7. SCREWS
   - A. SCREWS ATTACHING TO STEEL SHALL BE 12-24 HEX WASHER HEAD #5 POINT SELF DRILLING SCREWS WITH BOND SEAL WASHERS BY SFS OR APPROVED EQUAL.
   - B. SCREWS ATTACHING TO ALUMINUM SHALL BE 8-18 HEX WASHER HEAD #2 POINT SELF DRILLING SCREWS BY ATLAS OR APPROVED EQUAL.
   - C. ALL SCREWS SHALL BE STAINLESS STEEL OR COATED WITH ZINC.
8. SHOP FABRICATION AND FIELD ASSEMBLY
   - A. ALL STRUCTURAL STEEL AND ALUMINUM COMPONENTS SHALL BE SHOP FABRICATED SO THAT FIELD ASSEMBLY OF CONNECTIONS CAN BE PERFORMED USING ONLY BOLTING AND SCREW PLACEMENT.

BASIS OF DESIGN

DESIGN PARAMETERS

ROOF SNOW LOAD	30 PSF
DESIGN ROOF DEAD LOAD	5 PSF
WIND CRITERIA / EXPOSURE C	100 MPH
SOIL DESIGN BEARING STRENGTH	1000 PSF
CODES:	2006 IBC ASCE 7-05

BUILDING DATA

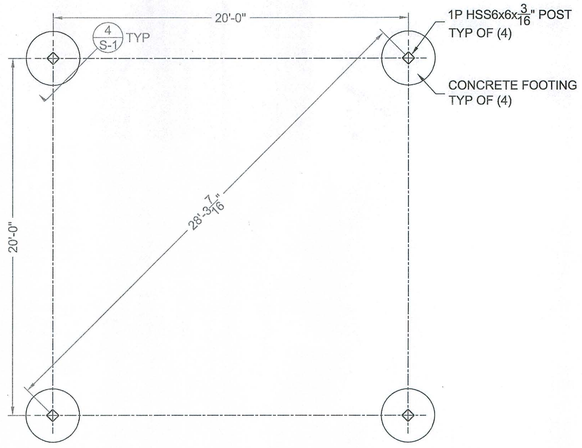
OCCUPANCY CLASSIFICATION	A-3 (NON-SEPERATED USE)
CONSTRUCTION TYPE	TYPE II-B
FLOOR AREA	576 SQ. FT.
OCCUPANCY LOAD	7 SQ. FT. / OCCUPANT= 82 OCCUPANTS / SHELTER

DESIGN CRITERIA

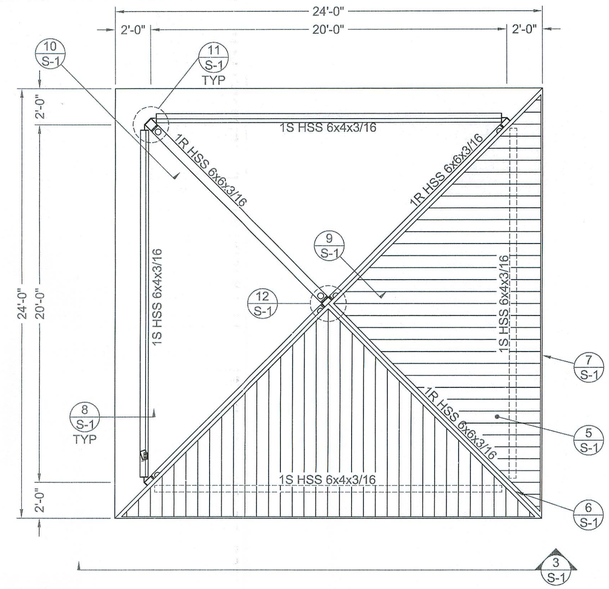
WIND DESIGN CRITERIA	
BASIC WIND SPEED	100 MPH
WIND IMPORTANCE FACTOR	Iw = 1.0
OCCUPANCY CATEGORY	I
WIND EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	N/A (OPEN STRUCTURE)
DESIGN WIND PRESSURE (COMPONENTS & CLADDING)	SEE CALCS.
SEISMIC DESIGN CRITERIA	
SEISMIC IMPORTANCE FACTOR	I_e = 1.0
OCCUPANCY CATAGORY	I
SPECTRAL RESPONSE ACCELERATION, 0.2, SECOND	S_s = 0.225
SPECTRAL RESPONSE ACCELERATION, 1.0, SECOND	S_1 =0.089
SITE CLASS	D
SPECTRAL RESPONSE COEFFICIENT, S_{DS}	0.240
SPECTRAL RESPONSE COEFFICIENT, S_{D1}	0.143
SEISMIC DESIGN CATEGORY	C
BASIC SEISMIC FORCE RESISTING SYSTEM	ORDINARY STEEL MOMENT FRAME
DESIGN BASE SHEAR	V = C_sW
SEISMIC RESPONSE COEFFICIENT	C_s = 0.069
RESPONSE MODIFICATION FACTOR	R = 3.5
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE

FOOTING REACTIONS	
⇩	5.548 KIPS
⇧	1.644 KIPS
⇨	0.525 KIPS

NOTE: THE FOOTING CONFIGURATION IS A RECOMMENDATION ONLY. ACTUAL SOIL CONDITIONS AT SHELTER LOCATION SHALL BE VERIFIED AND ANY REQUIRED ADJUSTMENTS TO THE CONFIGURATION SHALL BE DESIGNED BY OTHERS. SOIL CONDITIONS USED IN THIS RECOMMENDED FOOTING CONFIGURATION WERE ONLY ASSUMED. NO GEOTECHNICAL DATA WAS PROVIDED

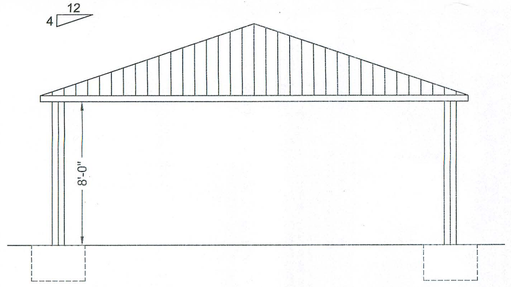


24'x24' NAVAJO SHELTER FOUNDATION PLAN — 1/4" = 1'-0" — 1 S-1

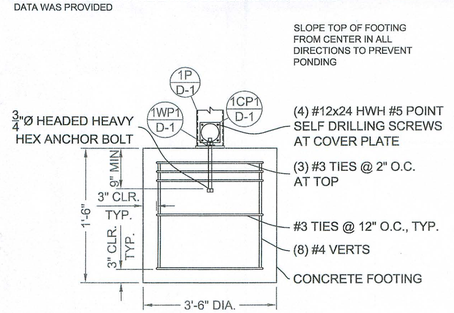


24'x24' NAVAJO SHELTER ROOF PLAN — 1/4" = 1'-0" — 2 S-1

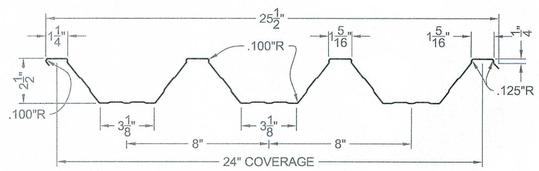
BOTTOM OF TIE BLOCK ELEVATION 11'-2 15/16"



24'x24' NAVAJO ELEVATION — 1/4" = 1'-0" — 3 S-1

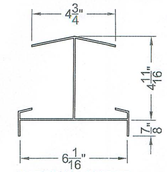


FOUNDATION SECTION — 3/4" = 1'-0" — 4 S-1



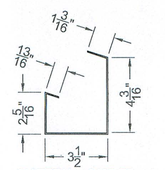
GALVALUME t = 0.024

INTERLOCKING ROOF DECK PROFILE — 3" = 1'-0" — 5 S-1



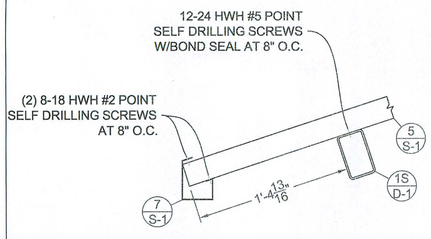
MATERIAL: 6105-T5 ALUMINUM .090" THICK

RIDGE CAP — 3" = 1'-0" — 6 S-1

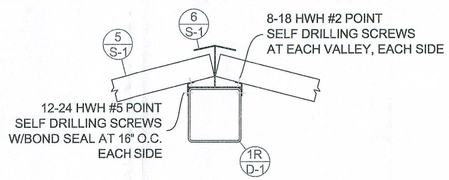


MATERIAL: 6105-T5 ALUMINUM .062" THICK

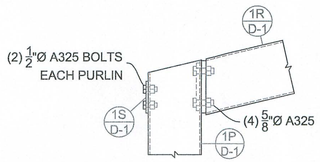
GUTTER — 3" = 1'-0" — 7 S-1



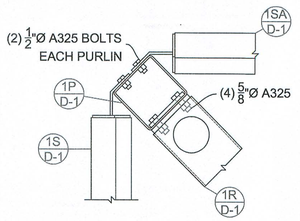
ROOF DECK TO PURLIN CONNECTION — 1-1/2" = 1'-0" — 8 S-1



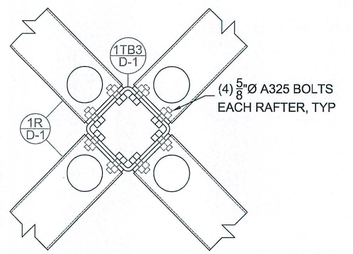
ROOF DECK TO RAFTER & RIDGE BEAM CONNECTION — 1-1/2" = 1'-0" — 9 S-1



RAFTER TO POST CONNECTION — 1-1/2" = 1'-0" — 10 S-1



PURLIN TO POST CONNECTION — 1-1/2" = 1'-0" — 11 S-1



RAFTER TO RIDGE BEAM CONNECTION — 1-1/2" = 1'-0" — 12 S-1

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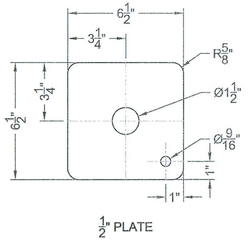
MAY 09 2011

SHELTER

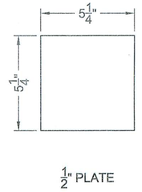
24'x24' NAVAJO SHELTER

DRAWN BY	RJM
CHECKED BY	BW, TN
SCALE	SEE DETAILS
DATE	3-31-11
REVISE DATE	5-3-11
P.O. NO.	21404
JOB NO.	139860
DRAWING NO.	A-2333
SHEET	S-1
	OF 2 SHEETS



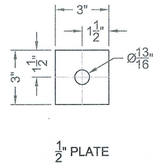
1/2" PLATE

(4) BASE PLATE 1BP7



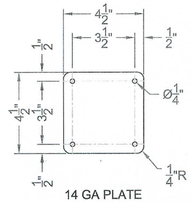
1/2" PLATE

(4) POST PLATE 1PL7



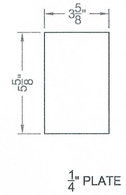
1/2" PLATE

(4) WASHER 1WP1



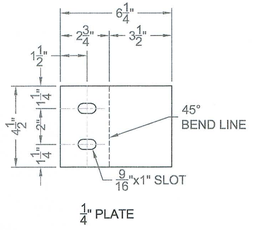
14 GA PLATE

(4) COVER PLATE 1CP1



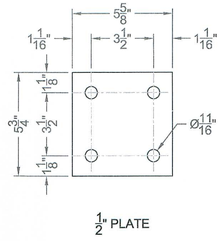
1/4" PLATE

(8) PURLIN PLATE 1Sa



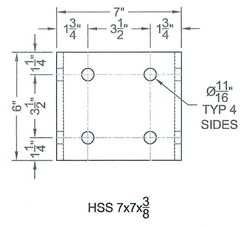
1/4" PLATE

(8) PURLIN MOUNT 1Sb



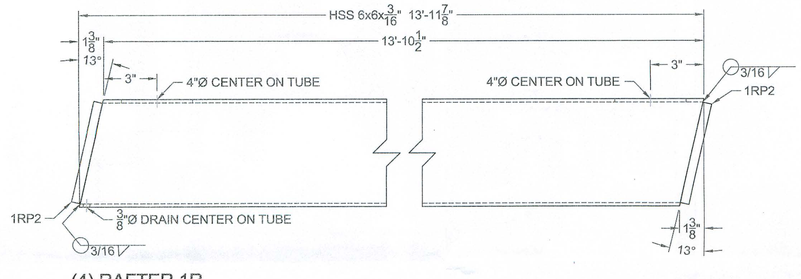
1/2" PLATE

(8) RAFTER CAP 1RP2



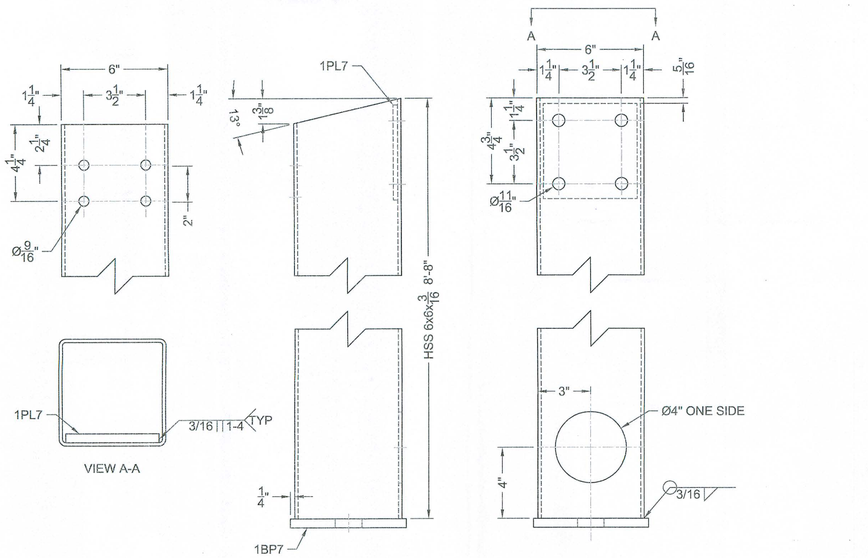
HSS 7x7x3/8

(1) TIE BLOCK 1TB3



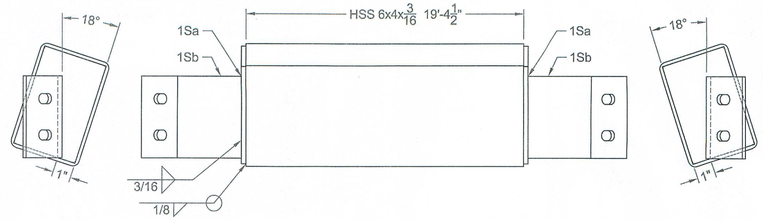
(4) RAFTER 1R

3" = 1'-0"



(4) POST 1P

3" = 1'-0"



(4) PURLIN 1S

3" = 1'-0"

NOTE: DRILL 1/8"Ø VENT HOLE IN TOP OF PURLIN

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SEAL

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