GENERAL STRUCTURAL NOTES

BUILDING CODES USED FOR DESIGN

1. INTERNATIONAL BUILDING CODE, 2009 EDITION

DESIGN LOADS

1. DESIGN LIVE LOADS:

PUBLIC AREAS, 1ST FLOOR CORRIDORS & STAIRS	100 PSF
CORRIDORS ABOVE 1ST FLOOR	80 PSF
OFFICE AREAS	50 PSF + 20 PSF*
STORAGE	125 PSF
MECHANICAL	125 PSF**
ROOFS	20 PSF

2. ROOF SNOW:

GROUND SNOW LOAD	25 PSF
FLAT-ROOF SNOW LOAD	30 PSF***
SNOW EXPOSURE FACTOR	1.0
SNOW IMPORTANCE FACTOR	1.0
THERMAL FACTOR	1.0

3. WIND:

BASIC WIND SPEED	90 MPH
WIND IMPORTANCE FACTOR	1.15
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	±0.18
COMPONENTS & CLADDING	ANALYSIS PER ASCE 7-05 CH. 6

4. SEISMIC:

SEISMIC IMPORTANCE FACTOR	1.0
USE GROUP	III
S_S	0.114
S_1	0.054
SITE CLASS	D
S_{DS}	EXISTT
S_{D1}	0.15
* PARTITION LOAD	0.084

** OR EQUIPMENT WEIGHT IF HEAVIER

*** PLUS DRIFTING AND/OR SLIDING SNOW

NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION

1. THE CONTRACTOR SHALL VERIFY, BY FIELD CHECK, ALL SIZES, DIMENSIONS, ELEVATIONS, LOCATIONS, ETC. OF ELEMENTS OF THE EXISTING CONSTRUCTION WHICH ARE RELATIVE TO THE NEW CONSTRUCTION.
2. ALL DIMENSIONS INVOLVING NEW WORK TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION SHALL BE FIELD CHECKED BY THE CONTRACTOR AND FURNISHED TO THE SUBCONTRACTOR, PRIOR TO FABRICATION OF ANY WORK. THE VERIFIED DIMENSIONS SHALL APPEAR AND BE NOTED AS SUCH ON THE FIRST SHOP DRAWING SUBMITTED.
3. THE ENGINEER HAS MADE ASSUMPTIONS CONCERNING THE SOUNDNESS OF THE EXISTING BUILDING AND THESE ASSUMPTIONS ARE THAT THIS BUILDING WAS DESIGNED AND CONSTRUCTED IN CONFORMITY WITH GOOD DESIGN AND CONSTRUCTION PRACTICES. THE CONTRACTOR SHALL TAKE EXTRAORDINARY PRECAUTIONS CONCERNING PRESERVATION OF THE BUILDING DURING DEMOLITION AND NEW CONSTRUCTION WORK. FURTHER, HE SHALL AGREE TO ASSUME ALL RESPONSIBILITY FOR THE PRESERVATION OF THIS PROPERTY.
4. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN CONSTRUCTION DOCUMENTS AND ACTUAL FIELD CONDITIONS.
5. ALL HOLES THROUGH EXISTING CONSTRUCTION SHALL BE CORE DRILLED OR SAW CUT.

NEW CONSTRUCTION

1. THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS.
2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE CONSTRUED TO APPLY AT ANY SIMILAR CONDITION ELSEWHERE ON THE JOB EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO CONSTRUCTION.
4. THE STEEL FRAMING COMPONENTS SHOWN RELY ON BUILDING COMPONENTS OTHER THAN STRUCTURAL STEEL FOR FINAL STRUCTURAL STABILITY (PREVIOUSLY REFERRED TO AS A NON-SELF-SUPPORTING STEEL FRAME BY THE AISC "CODE OF STANDARD PRACTICE"). THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PROVISION OF ANY AND ALL TEMPORARY BRACING AND SHORING AGAINST WIND, ERECTION AND ALL CONSTRUCTION LOADS UNTIL ALL ELEMENTS, MEMBERS AND CONNECTIONS (FLOORS, ROOF, SHEAR WALLS, ETC.), AS SHOWN ON THE CONTRACT DOCUMENTS ARE COMPLETELY INSTALLED. THE STRUCTURAL MEMBERS SHOWN ON THE CONTRACT DOCUMENTS ARE DESIGNED FOR THE ANTICIPATED LOADS THAT THE STRUCTURE WILL BE SUBJECTED TO ONLY AFTER ALL STRUCTURAL ELEMENTS ARE IN PLACE AND FINAL CONNECTIONS ARE COMPLETE.
5. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.

EXCAVATION AND EARTHWORK

1. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED, PRIOR TO CONCRETE PLACEMENT, BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.
2. NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.
3. WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS SHALL BEAR ON VIRGIN SOIL OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.
4. FOUNDATION DESIGN DOES NOT ACCOUNT FOR WINTER CONSTRUCTION. ANY UNENCLOSED/UNHEATED SPACES SHALL BE ADEQUATELY PROTECTED AGAINST FROST DURING WINTER CONSTRUCTION BY CONTRACTOR.
5. THE FOUNDATION DESIGN IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURE(S):

SPREAD FOOTINGS	2000 PSF
WALL FOOTINGS	2000 PSF

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS SHALL BE IN ACCORDANCE WITH THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AS INDICATED IN THE NINTH EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE.
2. ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL BE DESIGNED FOR THE SERVICE-LOAD END REACTIONS INDICATED ON PLANS, OR IF NOT INDICATED, SHALL BE DESIGNED TO DEVELOP 60% OF THE ALLOWABLE UNIFORM LOAD TABULATED IN THE NINTH EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN, UNLESS NOTED OTHERWISE. NUMBER OF BOLTS MUST SATISFY MINIMUM, REQUIREMENTS AS FOLLOWS: 2 BOLTS PER CONNECTION FOR 8" AND 10" DEEP MEMBERS, 3 FOR 12" AND 14" DEEP MEMBERS, 4 FOR 16" AND 18" DEEP MEMBERS, 5 FOR 21" AND 24" DEEP MEMBERS, AND 6 FOR 27" AND DEEPER MEMBERS. SINGLE SHEAR PLATE CONNECTIONS SHALL BE DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF APPENDIX C OF THE AISC CONNECTIONS MANUAL.
3. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

STEEL WIDE FLANGES	ASTM A992
STEEL ANGLES, CHANNELS AND PLATES	ASTM A36
STEEL PIPE	ASTM A53, GRADE B OR ASTM A501
STEEL TUBE	ASTM A500, GRADE B

4. ALL FILLER METAL USED IN WELDING SHALL BE 70 KSI YIELD, LOW-HYDROGEN.
5. ALL WELDING SHALL BE BY CERTIFIED WELDERS AND SHALL CONFORM TO THE "STRUCTURAL WELDING CODE", AWS D1.1 AND MEET AISC MINIMUM REQUIREMENTS FOR WELD SIZE.
6. FIELD CONNECTIONS ARE TO BE BOLTED AND INSTALLED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS." USE MIN 3/4" DIAMETER HIGH STRENGTH BOLTS AND NUTS, A325-TYPE 1, UNLESS NOTED OTHERWISE. CONNECTIONS SHALL BE ST TYPE EXCEPT WHERE PT OR SC TYPE CONNECTIONS ARE SPECIFICALLY NOTED. WHERE SC TYPE CONNECTIONS ARE NOTED PROVIDED CLASS A FAYING SURFACES, WITH SLIP CHECKED AT SERVICE LEVEL.
7. STEEL COLUMN BASE PLATES SHALL BE SIZE SHOWN ON PLAN WITH 3/4" DIAMETER ANCHOR RODS (F1554, GR 36) UNO AND 3/4" NON-METALLIC, NON-SHRINK GROUT FOR UNIFORM BEARING.
8. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SUPPLIER IS TO FURNISH L5x3 1/2 x 5/16 LLV SHOP WELDED ANGLE FRAMES AT ALL FLOOR AND ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH MECHANICAL (M/E/P/FP) CONTRACTOR. NOT ALL OPENINGS REQUIRING FRAMES SHOWN ON DRAWINGS. NO EXTRAS FOR ADDITIONAL FRAMES WILL BE APPROVED DUE TO FAILURE OF GC TO COORDINATE BETWEEN TRADES.
9. ALL PLATES USED IN BOLTED CONNECTIONS SHALL HAVE ROLLED OR GAS CUT EDGES.
10. ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS SHALL BE PRIME PAINTED WITH ONE COAT OF FABRICATOR'S STANDARD RUST-INHIBITIVE PRIMER OR AS INDICATED IN THE PROJECT SPECIFICATION. TOUCH UP ALL DISTURBED AREAS AFTER ERECTION. STEEL TO BE FIREPROOFED SHALL RECEIVE PAINT/FINISH PROCESS COMPATIBLE WITH FIREPROOFING.
11. ADJUSTABLE MASONRY TIES SHALL BE FURNISHED AT 16" OC VERTICALLY AND 24" OC HORIZONTALLY ON ALL STEEL MEMBERS ENCASED IN OR ADJACENT TO MASONRY WALLS WHETHER OR NOT SUCH ANCHORS ARE SHOWN ON THE DRAWINGS. TIES SHALL BE CAPABLE OF TRANSMITTING FORCES PERPENDICULAR TO THE PLANE OF THE WALL.

STEEL FORM DECK

1. STEEL FORM DECK SHALL BE FABRICATED FROM GALVANIZED (G60) STEEL CONFORMING TO ASTM A653, GRADE 80.
2. ATTACH DECK USING WELDERS EXPERIENCED IN THIN METAL WELDING. TEST PRACTICE WELDS USING PRY-OFF TEST.
3. WELD DECK SHEETS TO SUPPORTING MEMBER IMMEDIATELY AFTER ALIGNMENT USING A WELD WASHER AT EACH SHEET EDGE PLUS TWO INTERMEDIATE WELDS (4 WELDS PER SHEET).
4. WELDING WASHERS SHALL BE USED ON ALL DECKING LESS THAN 22 GAUGE.
5. DECK ATTACHMENT SHALL MEET STEEL DECK INSTITUTE REQUIREMENTS AS A MINIMUM, BUT IN NO CASE LESS THAN THE FOLLOWING:
   A. FASTEN EACH END LAP USING A WELD WASHER AT EACH SHEET EDGE PLUS TWO INTERMEDIATE WELDS (4 WELDS PER SHEET).
   B. SIDELAP CONNECTIONS SHALL CONSIST OF ONE NO. 10 SIDELAP SCREW AT EACH INTERMEDIATE SUPPORT.
6. FURNISH AND INSTALL SHEET METAL CLOSURES, JOINT COVERS, CONCRETE STOPS AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE INSTALLATION.

HELICAL PILE

1. CONTRACTOR SHALL SUBMIT SPECIFICATION DATA, SHOP DWGS, CERTIFIED TEST REPORTS, INSTALLATION PROCEDURES, AND CALCULATIONS SIGNED & SEALED BY AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED TO ENGINEER OF RECORD FOR APPROVAL.
2. SHOP DRAWINGS SHALL INCLUDE SIZES & LOCATIONS OF THE PILES. DRAWINGS SHALL INCLUDE THE SHAFT SIZE AND HELICAL BEARING PLATES. CALCULATIONS SHALL INCLUDE THE DESIGN OF THE SHAFT AND HELICAL BEARING PLATES BASED ON THE GEOTECHNICAL REPORT USING GENERALLY ACCEPTED ENGINEERING JUDGEMENT AND METHODS.
3. PILES SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
4. MANUFACTURER SHALL PROVIDE HELICAL PILES, AND BRACKETS IF REQUIRED, FOR SERVICE LOADS INDICATED ON THE DRAWINGS AND APPLY A MINIMUM FACTOR OF SAFETY = 2.0.
5. WHERE INSTALLATION OCCURS ADJACENT TO EXISTING BUILDING INSTALLER SHALL TAKE CARE CONCERNING THE PRESERVATION OF THE EXISTING BUILDING. NO NEW LOADS SHALL BE APPLIED TO THE EXISTING BUILDING UNLESS APPROVED BY THE ENGINEER.
6. PILES SHALL BE PLACED AS INDICATED ON PLAN. CONTRACTOR SHALL NOT DEVIATE FROM PLAN UNLESS APPROVED BY THE ENGINEER.
7. A WORKING LOAD TEST SHALL BE PROVIDED ON THE FIRST INSTALLED PILE. THE TESTS SHALL BE IN CONFORMANCE WITH ASTM D-1143-81 STANDARD TEST FOR PILE UNDER STATIC AXIAL COMPRESSIVE LOAD AND/OR ASTM D3689 STANDARD TEST METHOD FOR PILE UNDER STATIC AXIAL TENSION LOAD.
8. THE ACCEPTED CRITERIA FOR HELICAL PILES IS 1/2 OF THE APPLIED TEST LOAD CAUSING A NET SETTLEMENT (GROSS SETTLEMENT LESS THE ELASTIC COMPRESSION) NOT TO EXCEED 0.08 TIMES THE DIAMETER OF THE LARGEST HELICAL PLATE.

CONCRETE

1. ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE," ACI 318 AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE," ACI 301.
2. ALL CONCRETE SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH f'c = 3000 PSI FOR FOUNDATIONS AND SLABS AT 28 DAYS.
3. CONCRETE SHALL BE VIBRATED INTO FORMS WHILE PLACING, WITHOUT OVER-VIBRATING. REINFORCING SHALL BE PROTECTED BY CONCRETE OF THICKNESS AS FOLLOWS:

FOOTINGS	3"
PIERS	1 1/2" TO TIES
OTHER FOUNDATION	2" BOTH FACES
EXPOSED EXTERIOR CONCRETE	2"
GRADE BEAMS	2" SIDES, 3" BOTTOM
PILASTERS	1 1/2" TO TIES
SLAB ON GRADE	t/3 FROM TOP

4. SLABS-ON-GRADE SHALL BE PLACED IN CONTINUOUS STRIPS PER ACI RECOMMENDATIONS.
5. COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.
6. ALL ANCHOR RODS SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE.
7. ALL EXTERIOR EXPOSED CONCRETE TO HAVE 6%, PLUS 1% OR MINUS 1.5%, ENTRAINDED AIR.

REINFORCING STEEL

1. THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE PLANS AND/OR DETAILS.
2. REINFORCING STEEL SHALL BE ASTM A615, GRADE 60.
3. WELDED WIRE REINFORCING SHALL BE SUPPLIED IN FLAT SHEETS AND SHALL CONFORM TO ASTM A185.
4. CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO CRSI "RECOMMENDED PRACTICE FOR PLACING REINFORCEMENT BARS."
5. ALL REINFORCEMENT SPLICES SHALL BE LAPPED 48 BAR DIAMETERS MINIMUM UNLESS OTHERWISE NOTED. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCEMENT AT CORNERS AND INTERSECTIONS.
6. TOP BARS SHALL BE HOOKED AT END SPANS.
7. CONTRACTOR MAY SUBSTITUTE BLENDED FIBER SECONDARY REINFORCING, PROVIDED THE BLEND CONSISTS OF MULTIFILAMENT POLYPROPYLENE FIBERS AND COLD DRAWN STEEL WIRE FIBERS, NOVOMESH 850 OR APPROVED EQUIVALENT, APPLIED AT A DOSAGE RATE OF 24 lb/cy UNLESS NOTED OTHERWISE OR A DIFFERENT RECOMMENDATION IS MADE BY THE MANUFACTURER.

LINTEL NOTES

1. LOOSE LINTELS FOR NON-LOAD BEARING WALLS SHALL HAVE A MINIMUM END BEARING OF 8" AND SHALL BE THE FOLLOWING SIZES, UNLESS NOTED OTHERWISE (LINTEL SIZES GIVEN ARE FOR NEW MASONRY CONSTRUCTION ONLY; SPANS NOTED ARE CLEAR SPANS):
   6" WALLS:
   UP TO 4'-0" SPAN — 2-L3 1/2"x2 1/2"x5/16 LLV
   4'-0" TO 6'-4" SPAN — 2-L3 1/2"x2 1/2"x3/8 LLV
   TYPICAL WALLS (1-L FOR EACH 4" OF WALL THICKNESS):
   UP TO 4'-0" SPAN — L3 1/2"x3 1/2"x5/16
   OVER 4'-0" TO 6'-0" — L5x3 1/2"x3/8 LLV
   OVER 6'-0" TO 8'-0" — L6x3 1/2"x3/8 LLV
   OVER 8'-0" TO 12'-0" — W8x18 + 5/16 PL ON BOTTOM (PL TO BE 1/2" LESS THAN WALL WIDTH)
2. PLATES ON LINTELS SHALL BE SHOP WELDED TO MEMBER W/ 1/4"x2" FILLET WELDS AT 12" O.C. STAGGERED.
3. WELD TOGETHER ALL BACK-TO-BACK ANGLES.
4. CONTRACTOR SHALL COORDINATE ALL REQUIRED OPENINGS IN NON-LOAD BEARING MASONRY WALLS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

STEEL ROOF DECK

1. STEEL ROOF DECK SHALL BE PAINTED OR GALVANIZED AS INDICATED IN THE PROJECT SPECIFICATIONS.
2. END JOINTS OF ROOF DECK SHALL HAVE 2" MINIMUM END LAPS.
3. ROOF DECK SHALL BE 1 1/2" x 22 GA. WIDE RIB DECK, TYPE B, UNLESS NOTED OTHERWISE ON DRAWINGS.
4. ROOF DECK SHALL BE WELDED AT 12" O.C. OVER ALL INTERIOR SUPPORTS AND 6" O.C. OVER ALL EXTERIOR SUPPORTS, UNLESS NOTED OTHERWISE ON DRAWINGS.
5. WELDS TO HAVE A NOMINAL DIAMETER OF 5/8".
6. PROVIDE ONE SIDELAP SCREW PER SPAN, MINIMUM, UNLESS NOTED OTHERWISE ON DRAWINGS OR REQUIRED FOR MINIMUM SIDELAPS BY THE STEEL DECK INSTITUTE.
7. MAKE JOINTS OVER SUPPORTING MEMBER ONLY AND USE MINIMUM TRIPLE SPANS.
8. MECHANICAL FASTENERS MAY BE USED WITH THE APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.

PIER SCHEDULE

MARK	PIER SIZE B x N	VERTICAL REIN	REIN DETAIL	PIER TIES	ELEV OF TOP OF PIER	REMARKS
P1	12"x18"	6 #6	2/4-SS2.00	(3) #3 @ 2" OC REST @ 12" OC	-0'-8"	SEE 3/4-SS 2.01

PILE CAP SCHEDULE

f'c = 3000 psi

MARK	SIZE			REINFORCING, TOP + BOTTOM		REMARKS
	L	S	D	LONG BARS	SHORT BARS	
PC5.0	5'-0"	5'-0"	2'-0"	6 #5	6 #5	SEE 1/4-SS 0.00
PC6.0	6'-0"	6'-0"	2'-0"	7 #5	7 #5	SEE 1/4-SS 0.00

COLUMN SCHEDULE

MARK	COLUMN SIZE	BASE PL t x B x N	ELEV OF BOT OF BASE PL	ANCHOR BOLTS	CAP PL t	BASE PL DETAIL	REMARKS
C1	HSS6x6x3/8	1"x12"x1'-0"	1" ABOVE T/PILE CAP	(4) 3/4"ø ANCHOR BOLTS W/ 9" EMBEDMENT	3/4"	3/4-SS0.00	
C2	ROUND HSS5.0x3/8	1"x12"x1'-0"	1" ABOVE T/PILE CAP	(4) 3/4"ø ANCHOR BOLTS W/ 9" EMBEDMENT	3/4"	3/4-SS0.00	
C3	HSS4x4x3/8	3/4"x7"x1'-0"	1" ABOVE T/PIER	(4) 3/4"ø ANCHOR BOLTS W/ 9" EMBEDMENT	3/4"	3/4-SS0.00	
C4	HSS4x4x3/8	1/2"x8"x0'-10"	SEE 2BA2C/4-SS 0.00	SEE 2BA2C/4-SS 0.00	1/2"	3/4-SS0.00	

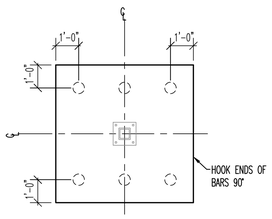
LINTEL SCHEDULE

MARK	SIZE	END BEARING	SHAPE	REMARKS
L1	HSS8x6x1/2	-		CONNECT TO NEW CANOPY BEAM BOTH ENDS, UNO
L2	W8x24 W/ 5/16" PL	8"		BEARING PL 7"x3/8"x0'-8", CONNECT WEST END TO EXISTING COLUMN. SEE 4/4-SS 0.00
L3	W8x24 W/ 5/16" PL	8"		BRG PL 7"x3/8"x0'-8"

NOTE: PLATES ON LINTELS SHALL BE WELDED TO MEMBER WITH 1/4"x2" FILLET WELDS AT 12" O.C. STAGGERED EACH SIDE.
FIELD MEASURE EXISTING WALLS TO DETERMINE PLATE WIDTHS.

STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLT	FOC	FACE OF CONCRETE	PCF	POUNDS PER CUBIC FOOT
ADDL	ADDITIONAL	FOW	FACE OF WALL	PL	PLATE
ADJ	ADJACENT	FP	FIREPROOFING	PLF	POUNDS PER LINEAR FOOT
AFF	ABOVE FINISH FLOOR	FS	FAR SIDE	PLYWD	PLYWOOD
AHU	AIR HANDLING UNIT	FSTNR	FASTENER	PREFAB	PREFABRICATED
ALUM	ALUMINUM	FT, '	FEET OR FOOT	PRELIM	PRELIMINARY
ALT	ALTERNATE	FTG	FOOTING	PSF	POUND PER SQUARE FOOT
APPROX	APPROXIMATE(LY)	GA	GAUGE	PSI	POUND PER SQUARE INCH
ARCH	ARCHITECT/ARCHITECTURAL	GALV	GALVANIZED	PT	POST TENSIONED
B/B	BACK TO BACK	GR BM	GRADE BEAM	QTY	QUANTITY
BL	BRICK LEDGE	GC	GENERAL CONTRACTOR	R	RADIUS
BLDG	BUILDING	GEN	GENERAL	RD	ROOF DRAIN
BM	BEAM	GL	GLUE LAMINATE TIMBER	REBAR	REINFORCING STEEL BAR
BOS	BOTTOM OF STEEL	GR	GRADE	REF	REFERENCE
BOT	BOTTOM	GRTG	GRATING	REINF	REINFORCE (D) (MENT)
BRG	BEARING	GYP	GYPSUM	REQ	REQUIRE (D)
BRDG	BRIDGING	HORIZ	HORIZONTAL	REV	REVISED/REVISION
BSMT	BASEMENT	HK	HOOK	RH	RIGHT HAND
BS	BOTH SIDES	HS	HEADED STUD	RTU	ROOF TOP UNIT
C	COMPRESSION	HT	HEIGHT	SCHED	SCHEDULE
CA	CAISSON	I	MOMENT OF INERTIA	SECT	SECTION
CH	CHANNEL	IBC	INTERNATIONAL BUILDING CODE	SF	SQUARE FOOT
CIP	CAST-IN-PLACE CONCRETE	ID	INSIDE DIAMETER	SHT	SHEET
CJ	CONTROL/CONSTRUCTION JOINT	IN, "	INCH OR INCHES	SIM	SIMILAR
CL	CENTER LINE	INCL	INCLUDE	SL	SNOW LOAD
CLG	CEILING	INFO	INFORMATION	SOG	SLAB ON GRADE
CLR	CLEAR	IN-LB	INCH POUND	SPA	SPACES
CM	CONSTRUCTION MANAGER	INSUL	INSULATION	SPEC	SPECIFICATION
CMU	CONCRETE MASONRY UNIT	INT	INTERIOR	SST	STAINLESS STEEL
COL	COLUMN	JF	JOINT FILLER	STD	STANDARD
CONC	CONCRETE	JST	JOIST	STL	STEEL
CONF	CONFERENCE	JT	JOINT	STRUCT	STRUCTURAL
CONN	CONNECTION	K	KIP (OR 1000 POUNDS)	T&B	TOP AND BOTTOM
CONST	CONSTRUCTION	KLF	KIPS PER LINEAR FOOT	T&G	TONGUE AND GROVE
CONT	CONTINUE (D) (OUS)	L	ANGLE	TOC	TOP OF CONCRETE
CONTR	CONTRACTOR	LATL	LATERAL	TOF	TOP OF FOOTING
COORD	COORDINATE	LB, #	POUNDS	TOFDTN	TOP OF FOUNDATION
CTR	CENTER	LF	LINEAR FOOT	TOP	TOP OF PLANK
DBA	DEFORMED BAR ANCHOR	LH	LEFT HAND	TOS	TOP OF SLAB, TOP OF STEEL
DBL	DOUBLE	LL	LIVE LOAD	T	TENSION
DEMO	DEMOLITION	LLH	LONG LEG HORIZONTAL	TCX	TOP CHORD EXTENSION
DET	DETAIL	LLV	LONG LEG VERTICAL	TD	TRENCH DRAIN
DIA	DIAMETER	LT GA	LIGHT GAUGE	THK	THICK
DIAG	DIAGONAL	LT WT	LIGHT WEIGHT	THRU	THROUGH
DIM	DIMENSION (S)	LVL	LAMINATED VENEER LUMBER	TYP	TYPICAL
DL	DEAD LOAD	M	MOMENT	UNO	UNLESS NOTED OTHERWISE
DP	DRILLED PIER	MATL	MATERIAL	V	SHEAR
DWG	DRAWING	MAU	MAKE-UP AIR UNIT	VAR	VARIES
DWL	DOWEL	MAX	MAXIMUM	VERT	VERTICAL
E	MODULUS OF ELASTICITY	MECH	MECHANICAL	VIF	VERIFY IN FIELD
EA	EACH	MEP	MECHANICAL ELECTRICAL PLUMBING	W/	WITH
ECC	ECCENTRICITY	MEZZ	MEZZANINE	W/O	WITHOUT
EF	EACH FACE	MFR	MANUFACTURER	WD	WOOD
EJ	EXPANSION JOINT	MFR REC	MANUFACTURER'S RECOMMENDATIONS	WP	WORK POINT
EL	ELEVATION	MIN	MINIMUM	WS	WATERSTOP
EOR	ENGINEER OF RECORD	MISC	MISCELLANEOUS	WT	WEIGHT
EOS	EDGE OF SLAB	MO	MASONRY OPENING	WWR	WELDED WIRE REINFORCEMENT
EQ	EQUAL	MP	METAL PANEL		
EQUIV	EQUIVALENT	MTL	METAL		
EQUIP	EQUIPMENT	NA	NOT APPLICABLE		
EW	EACH WAY	NIC	NOT IN CONTRACT		
EWEF	EACH WAY EACH FACE	NO, #	NUMBER		
EXC	EXCAVATE	NOM	NOMINAL		
EXIST	EXISTING	NS	NEAR SIDE		
EXP	EXPANSION	NTS	NOT TO SCALE		
EXP BT	EXPANSION BOLT	NWC	NORMAL WEIGHT CONCRETE		
EXT	EXTERIOR	OC	ON CENTER		
F/F	FACE TO FACE	OD	OUTSIDE DIAMETER		
FAB	FABRICATOR	OH	OPPOSITE HAND		
FD	FLOOR DRAIN	OPNG	OPENING		
FDTN	FOUNDATION	OPP	OPPOSITE		
FIN FLR	FINISHED FLOOR	PC	PRECAST CONCRETE		
FLR	FLOOR	PCC	PVC		



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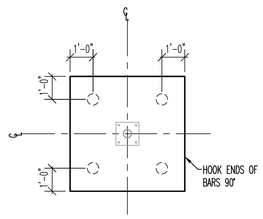

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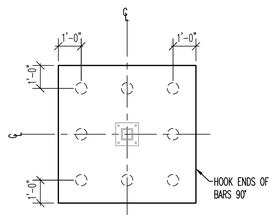
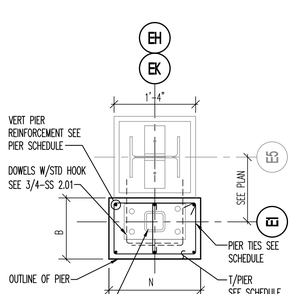
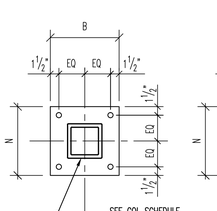


PILE CAP PC 6.0 WITH 8 PILES

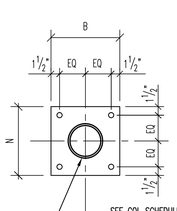


2 4-SS 0.00 DETAIL PIER P1/C3
1" = 1'-0"

1 4-SS 0.00 PILE CAP DETAILS
3/8" = 1'-0"

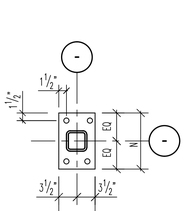


COL C1

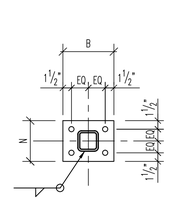


COL C2

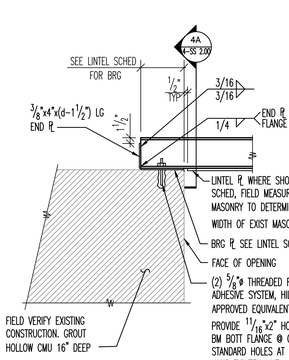
3 4-SS 2.00 BASE PLATE DETAILS
NO SCALE



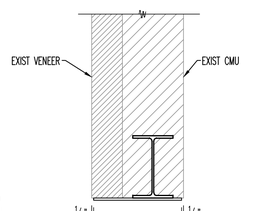
COLUMN C3



COLUMN C4



4 4-SS 0.00 TYP WF LINTEL BEARING DETAIL ON EXISTING MASONRY WALL
3/4" = 1'-0"



4A 4-SS 0.00 SECTION
1 1/2" = 1'-0"

Revisions:	Date

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Drawing Title: GENERAL NOTES AND SCHEDULES

Approved: Project Director

Project Title: FY2012 - BUILDING 4

Location: 3001 North Green Bay RD, North Chicago, IL 60064

Date: 03-30-2012 | Checked: CF | Drawn: VC

Project Number: 211097.00

Building Number: 4

Drawing Number: 4-SS 0.00

Dwg. of -

Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08-6231