GENERAL NOTES:

I. GENERAL

1. MATERIALS AND WORKMANSHIP TO CONFORM WITH THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE, WITH TITLE 24 AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
2. THESE GENERAL NOTES SUPPLEMENT THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS. IN CASE OF CONFLICT BETWEEN THE PLANS AND SPECIFICATIONS, CONTACT THE RESIDENT ENGINEER.
3. VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE CONDITIONS BEFORE STARTING WORK. NOTIFY RESIDENT ENGINEER OF DISCREPANCIES.
4. REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR SITE SLABS AND WALLS.
5. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW BY THE RESIDENT ENGINEER.
6. DETAILS ON SHEETS TITLED "TYPICAL DETAILS" APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. SUCH DETAILS ARE NOT NOTED AT EACH LOCATION THAT THEY OCCUR.
7. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND FOR CHECKING DIMENSIONS. NOTIFY THE RESIDENT ENGINEER OF ANY DISCREPANCIES AND RESOLVE BEFORE PROCEEDING WITH THE WORK.
8. DO NOT SCALE THE DRAWINGS.
9. INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENTS THE PRESENT KNOWLEDGE, BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE ARCHITECT. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE RESIDENT ENGINEER.
10. REFER TO ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS OF FLOOR AND ROOF OPENINGS. COORDINATE THE SIZE AND LOCATION OF OPENINGS ASSOCIATED WITH, BUT NOT LIMITED TO, ELECTRICAL, MECHANICAL AND PLUMBING TRADES. SUBMIT FINAL SIZING AND LOCATION REQUIREMENTS OF OPENINGS TO THE ARCHITECT FOR REVIEW.
11. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING A SAFE PLACE TO WORK AND MEETING THE REQUIREMENTS OF ALL APPLICABLE JURISDICTIONS. EXECUTE WORK TO ENSURE THE SAFETY OF PERSONS AND ADJACENT PROPERTY AGAINST DAMAGE BY HAZARDS INCONNECTION WITH THIS WORK.
12. SEE ARCHITECTURAL DRAWING FOR SEISMIC JOINT COVERS. SEE DETAIL 8/S1402 AND PLANS FOR REQUIRED JOINT MOVEMENT.
13. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS.

II. CONSTRUCTION MEANS AND METHODS ENGINEERING (SHORING)

1. CONTRACTOR TO PROVIDE MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES INCLUDE BUT ARE NOT LIMITED TO BRACING, UNDERPINNING, AND SHORING FOR LOADS DURING CONSTRUCTION. RETAIN CALIFORNIA REGISTERED CIVIL ENGINEER WHOM IS PROPERLY QUALIFIED TO DESIGN BRACING, UNDERPINNING, SHORING, ETC.
2. PRIOR TO CONSTRUCTION SUBMIT SHORING DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A CALIFORNIA REGISTERED CIVIL ENGINEER. IN ADDITION, SUBMIT A MONITORING PLAN, SEQUENCING, AND ANY LOADS TO BE IMPOSED ON THE STRUCTURE.
3. MAINTAIN EXISTING GRADE A MINIMUM OF 20 FEET FROM THE EDGE OF EXISTING BUILDING FOUNDATIONS BEFORE STARTING EXCAVATIONS. EXCAVATION TO HAVE A MAX SLOPE OF 2H:1V.
4. VISITS TO THE SITE BY THE STRUCTURAL ENGINEER OF RECORD WILL NOT INCLUDE OBSERVATION OF THE CONSTRUCTION MEANS AND METHODS SHORING.

III. FOUNDATION AND SITE WORK

1. THE DESIGN OF THE FOUNDATION SYSTEM IS BASED UPON THE CRITERIA AND RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT ENTITLED "GEOLOGICAL AND GEOTECHNICAL INVESTIGATION COMMUNITY LIVING CENTER EXPANSION VETERANS AFFAIR MEDICAL CENTER FRESNO CA. AND DATED MARCH 4, 2013"
2. GROUNDWATER ELEVATION IS ESTIMATED IN THE GEOTECHNICAL REPORT. PROVIDE SITE DE-WATERING AS REQUIRED.
3. LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
4. REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED.
5. NOTIFY THE RESIDENT ENGINEER IF ANY BURIED STRUCTURES NOT INDICATED ARE FOUND.
6. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
7. REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE.
8. EXCAVATIONS FOR FOUNDATIONS MUST BE ACCEPTED BY THE VA'S GEOTECHNICAL ENGINEER PRIOR TO PLACING REINFORCING AND CONCRETE. NOTIFY THE VA'S GEOTECHNICAL ENGINEER WHEN EXCAVATIONS ARE READY FOR INSPECTION.
9. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.
10. MECHANICALLY COMPACT EXCAVATION BACKFILLS IN LAYERS PER THE SPECIFICATIONS AND THE GEOTECHNICAL REPORT.

IV. FORMWORK

1. PROVIDE POUR POCKETS IN FORMS AND UNDER EXISTING STRUCTURAL MEMBERS AS REQUIRED TO PREVENT AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE EXISTING MEMBERS. CONCRETE CAST WITH AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE MEMBERS IS NOT ACCEPTABLE.
2. REMOVE FORMS AND SHORES IN ACCORDANCE WITH THE FOLLOWING:

LOCATION	REMOVE FORMS AND SHORES NO SOONER THAN
BOTTOM FORMS AND SHORES FOR MILDLY REINFORCED SLABS, BEAMS AND GIRDERS	3 DAYS AND MIN. 75% F'c
SIDE FORM FOR BEAMS AND GIRDERS	24 HOURS
COLUMNS AND WALLS	24 HOURS
FOOTINGS, PILE CAPS, AND GRADE BEAMS	24 HOURS

3. PROVIDE CURING WHERE FORMS ARE REMOVED IN LESS THAN 7 DAYS, INCLUDING BUT NOT LIMITED TO WALLS, COLUMNS, AND UNDERSIDE OF ELEVATED SLABS.

V. REINFORCING STEEL

1. REINFORCING TO CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED:

LOCATION	TYPE
ALL REINFORCING STEEL IN SUSPENDED SLAB, WALLS, FOUNDATIONS, AND ALL REINFORCEMENT. REINFORCING STEEL #8 AND LARGER.	ASTM A706, 60 KSI
REINFORCING STEEL #7 AND SMALLER OR IN SLABS ON GRADE, HOUSEKEEPING PADS OR CURBS.	ASTM A615, 60 KSI
WELDED STEEL WIRE FABRIC	ASTM A185, 70 KSI
SMOOTH DOWELS IN SLAB ON GRADE	ASTM A36, 36 KSI

2. ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCEMENT FROM DISPLACING DUE TO FORMWORK, CONSTRUCTION, OR CONCRETE PLACEMENT OPERATIONS. LOCATE AND SUPPORT REINFORCING BY METAL CHAIRS, RUNNERS, BOLSTERS, SPACERS, OR HANGERS
3. MECHANICALCOUPLERS: TYPE 2, SEE SPECS FOR ADDITIONAL INFORMATION.
4. WELD REINFORCING STEEL ONLY WHERE NOTED AND IN ACCORDANCE WITH AWS D1.4 USING QUALIFIED WELDERS.
5. TERMINATE REINFORCING STEEL IN STANDARD HOOKS, UNLESS OTHERWISE SHOWN.
6. PROVIDE REINFORCING SHOWN OR NOTED CONTINUOUS IN LENGTHS AS LONG AS PRACTICABLE.
7. PROVIDE RENFORCING BAR DOWELS IN FOOTINGS AND OTHER SUPPORTING MEMBERS OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCING, U.O.N. LAP SPLICE AS INDICATED ON TYPCAL DETAILS.

VI. CAST-IN-PLACE CONCRETE

1. CONCRETE IS REINFORCED AND CAST-IN-PLACE UNLESS OTHERWISE NOTED. WHERE REINFORCING IS NOT SPECIFICALLY SHOWN OR WHERE DETAILS ARE NOT GIVEN, PROVIDE REINFORCING SIMILAR TO THAT SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE RESIDENT ENGINEER.
2. ROUGHEN CONCRETE SURFACES OF CONSTRUCTION JOINTS TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES. SUBMIT JOINT LOCATIONS OR JOINTS NOT SHOWN TO THE RESIDENT ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH THE WORK.
3. AT LOCATIONS WHERE CONCRETE IS CAST AGAINST EXISTING CONCRETE, ROUGHEN CONTACT SURFACES TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES.
4. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF ADDITIONAL CONCRETE CURBS AND HOUSEKEEPING PADS NOT SHOWN.
5. CONCRETE CLEAR COVER TO REINFORCING BARS IS AS FOLLOWS, UNLESS OTHERWISE NOTED:

LOCATION	CLEAR COVER
CONCRETE PLACED AGAINST EARTH	3 INCHES
FORMED SURFACES EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: #6 BARS AND LARGER #5 BARS AND SMALLER	 2 INCHES 1 ½ INCHES
SLABS ON GRADE (TOP CLEARANCE)	1 ½ INCHES
BEAMS, GIRDERS AND COLUMNS NOT EXPOSED TO WEATHER OR EARTH	1 ½ INCHES
WALL OR SLAB SURFACES NOT EXPOSED TO WEATHER OR EARTH: #5 & SMALLER #6 & #7 #8,#9, #10 & #11 #14 & #18	 ¾ INCH 1 INCH 1 ½ INCHES 2 ½ INCHES

6. CONCRETE TYPES:

CLASS	28-DAY STRENGTH	TYPE	LOCATION	CEMENT	MAX W/C	MAX AGGR
A	4000 PSI	NWC	GRADE BEAMS, FOUNDATIONS,	I-II	0.45	SIZE 467
B	4000 PSI	NWC	SLABS ON GRADE, PADS, CURBS MISC.	I-II	0.45	SIZE 67
C	4000 PSI	LWC	FILL ON METAL DECK	I-II	0.55	SIZE 67

7. LIGHT WT CONC. MAX. DRY UNIT WT=115 PCF. MAX.
8. WATER REDUCING OR HIGH RANGE WATER REDUCING ADD MIXTURES SATISFYING ASTM C494 ARE ANTICIPATED.
9. NO AIR ENTRAINMENT IS ANTICIPATED.
10. CONTINUOUSLY MOIST CURE ARCHITECTURALLY EXPOSED CONCRETE SLABS IN PUBLIC SPACES FOR 7 DAYS MINIMUM. WATER FOG SPRAYS, PONDING, SATURATED ABSORPTIVE COVERS, OR MOISTURE RETAINING COVERS MAY BE USED. CURING COMPOUNDS ARE ACCEPTABLE AT NON-ARCHITECTURALLY EXPOSED SLABS.
11. CONCRETE FILL THICKNESS SHOWN ON THE FRAMING PLANS ARE MINIMAL THICKNESSES. NO ALLOWANCES HAVE BEEN SHOWN FOR ADDITIONAL CONCRETE FILL REQUIRED TO COMPENSATE FOR FRAME, DECK, OR FORMWORK DEFLECTIONS TO MAINTAIN SURFACE TOLERANCES.
12. NON-SHRINK GROUT, 7000 PSI MIN. @ 28 DAYS.
13. SEE SPECIFICATION SECTION 033000 FOR FLOOR FLATNESS REQUIREMENTS.
14. PRIOR TO CONCRETE POUR, ANCHOR BOLT ELEVATION AND LOCATION SHALL BE CERTIFIED BY LICENSED SURVEYOR PER SPEC 051200.

VII. UNIT MASONRY

1. MINIMUM COMPRESSIVE STRENGTH OF MASONRY, F'M, EQUAL TO 1500 PSI AT 28 DAYS.
2. MASONRY UNITS: ASTM C-90, GRADE N, TYPE 1, MEDIUM WEIGHT, HOLLOW, LOAD BEARING UNITS. MINIMUM COMPRESSIVE STRENGTH: 1900 PSI.
3. MORTAR: ASTM C-270, TYPE S.
4. GROUT: ASTM C-476, MINIMUM COMPRESSIVE STRENGTH: 2000 PSI.
5. REINFORCING STEEL:

LOCATION	TYPE
REINFORCING TO BE WELDED	ASTM A706, GRADE 60
ALL OTHER REINFORCING	ASTM A615, GRADE 60
JOINT REINFORCING	UBC STD 21-10

6. CENTER VERTICAL REINFORCING IN WALL UNLESS NOTED OTHERWISE.
7. LAY UNITS IN RUNNING BOND AND MAINTAIN VERTICAL CONTINUITY OF CORES OR CELL CAVITIES. USE OPEN END UNITS AT LOCATIONS OF VERTICAL REINFORCING. PLACE HORIZONTAL REINFORCING IN BOND BEAM UNITS.
8. FILL ALL CELLS SOLIDLY WITH GROUT. CLEAN CELLS AND BOND BEAMS OF MORTAR PROTRUSIONS AND DEBRIS BEFORE GROUTING.
9. DOWELS FROM THE FOUNDATION TO MATCH SIZE AND LOCATION OF VERTICAL REINFORCING IN MASONRY, UNLESS NOTED OTHERWISE.
10. PROVIDE VERTICAL CONTROL JOINTS IN THE WALLS AT LOCATIONS SHOWN ON THE DRAWINGS.
11. LOW-LIFT GROUTING: LAY UNITS A MAXIMUM OF 2 FEET HIGH BEFORE GROUTING. PLACE GROUT IN A CONTINUOUS PLACEMENT IN GROUT LIFTS NOT EXCEEDING 2 FEET. COMPLETELY GROUT THE FULL HEIGHT OF THE WALL SECTION IN ONE DAY WITH NO INTERRUPTIONS GREATER THAN ONE HOUR
12. HIGH-LIFT GROUTING: LAY UNITS A MAXIMUM OF 12 FEET HIGH BEFORE GROUTING. PLACE GROUT IN A CONTINUOUS PLACEMENT IN GROUT LIFTS NOT EXCEEDING 6 FEET. COMPLETELY GROUT THE FULL HEIGHT OF THE WALL SECTION IN ONE DAY WITH NO INTERRUPTIONS GREATER THAN ONE HOUR. PROVIDE CLEANOUTS AT EVERY CELL ALONG THE BOTTOM COURSE OF EACH GROUT LIFT. USE SIKA GROUT AID TYPE II OR E-Z MIX ADMIXTURE IN THE GROUT TO REDUCE EARLY WATER LOSS AND PRODUCE AN EXPANSIVE ACTION.

VIII. STRUCTURAL STEEL

1. STRUCTURAL STEEL TO CONFORM TO THE FOLLOWING UNLESS OTHERWISE NOTED:

SECTIONS	TYPE
ROLLED SHAPES WIDE FLANGES CHANNELS, ANGLES, & OTHER	 ASTM A992, GR 50 ASTM A36
PLATES COLUMN BASE PLATES BEAM COVERSIDE PLATES BEAM SHEAR PLATES COLUMN CONTINUITY PLATES BEAM STIFFENER PLATES OTHER, U.O.N.	 ASTM A572, GR 50 ASTM A36 ASTM A36 ASTM A572, GR 50 ASTM A36 ASTM A572, GR 50
STEEL PIPE	ASTM A53 GRADE B
COLD FORMED HOLLOW STRUCTURAL SECTION (HSS)	ASTM A500 GRADE B
STAINLESS STEEL SHAPES, PLATES AND BARS	ASTM A276
BOLTS	ASTM 325X
MACHINE BOLTS	ASTM A307
ANCHOR BOLTS AND RODS	ASTM F1554 GR 55 U.O.N.
THREADED AND HANGER ROD	ASTM A307 OR A36
WELDED SHEAR CONNECTORS	ASTM A108, GRADE 1015 THROUGH 1020
NUTS FOR BOLTS AND MACHINE BOLTS	ASTM A563
HARDENED WASHERS	ASTM F436
UNHARDENED WASHERS	ASTM F844
PLAIN WASHERS	ANSI B18.22.1
BEVELED WASHERS	ANSI B18.23.1

2. HOT DIP GALVANIZE IN ACCORDANCE WITH ASTM A123 AND ASTM A153 STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER. REPAIR GALVANIZING AFTER WELDING IN ACCORDANCE WITH ASTM A780.
3. ARC-WELDING ELECTRODES/FILLER METALS TO BE LOW HYDROGEN TYPES E7XTX, E7XTXX OR E70XXX MINIMUM AS APPLICABLE.
4. WELDERS TO BE CERTIFIED BY AWS AND THE GOVERNING JURISDICTION.
5. WHERE FIELD WELDING IS NOTED, THE DESIGNATION IS GIVEN AS A SUGGESTED CONSTRUCTION PROCEDURE ONLY.
6. PROVIDE NATURAL CAMBER UP, UNLESS NOTED OTHERWISE, EXCEPT AT CANTILEVERS. AT CANTILEVERS PROVIDE CAMBER SUCH THAT TIP OF CANTILEVER IS ABOVE FINAL ELEVATION.
7. SPLICE MEMBERS ONLY WHERE INDICATED.
8. DETAIL, FABRICATE AND ERECT IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS".

IX. METAL DECKING

1. METAL FLOOR AND ROOF DECK TO HAVE MINIMUM SECTION PROPERTIES SHOWN ON SHEET "TYPICAL METAL DECK DETAILS."
2. METAL DECK TO MEET ASTM A653
3. FLOOR AND ROOF DECK TO BE GALVANIZED IN ACCORDANCE WITH ASTM A653 COATING CLASS G60. REPAIR DAMAGED COATING.
4. WHERE POSSIBLE, LAYOUT METAL DECK TO SPAN AT LEAST THREE SPANS CONTINUOUSLY. TERMINATE ENDS OVER SUPPORTS EXCEPT AT OPENINGS OR BUILDING EDGES WHERE METAL DECKS MAY BE CANTILEVERED AS SHOWN.
5. PROVIDE MINIMUM STUDS SHOWN IN 11/S601 U.O.N.
6. SEE SPECIFICATION SECTION 033000 FOR FLOOR FLATNESS REQUIREMENTS.

X. MECHANICAL AND ADHESIVE ANCHORS

1. ALL MECHANICAL ANCHORS ARE TO HAVE ICC APPROVAL FOR USE IN CRACKED CONCRETE UNDER SEISMIC LOADS.
2. EXPANSION ANCHORS: HILTI KB-TZ (ICC ESR-1917), ITW REDHEAD TRUBOLT+ (ICC ESR-2427), POWERS POWER-STUD+ (ICC ESR-2502), OR SIMPSON STRONG BOLT 2 (ICC ESR-3037).
3. CONCRETE SCREW ANCHORS: POWERS WEDGE-BOLT (ICC ESR-2526), OR SIMPSON TITEN HD (ICC ESR-2713).
4. ADHESIVE ANCHORS: HILTI HIT RE-500-SD (ICC ESR-2322), POWERS PE1000+ (ICC ESR-2583), OR SIMPSON SET-XP (ICC ESR-2508)
5. INSTALL AND TEST ANCHORS IN ACCORDANCE WITH THE LATEST ICC-ESR REPORT.
6. PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN PERMANENTLY EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.
7. ADHESIVE ANCHORS SHALL BE ASTM A36 THREADED RODS. REINFORCING DOWELS SHALL BE ASTM A615 GRADE 60 REINFORCING STEEL. UNLESS OTHERWISE NOTED.
8. EXPANSION AND SCREW ANCHOR MINIMUM EMBEDMENTS UNLESS OTHERWISE NOTED:

ANCHOR DIA. (SIZE)	MIN EMBED (INCH)
1/4"	2"
3/8"	2 ½"
1/2" *	2 ½"
5/8" *	3 ¼"
3/4"	4 ¼"

* MINIMUM EMBEDMENT MUST BE INCREASED FOR SIMPSON TITEN HD ANCHOR. USE 3 1/4" FOR 1/2" DIAMETER AND 5 1/2" FOR 5/8" DIAMETER SCREWS.

9. ADHESIVE ANCHOR MINIMUM EMBEDMENT DEPTH AND TEST LOADS IN CONCRETE, UNLESS OTHERWISE NOTED:

ANCHOR/BAR SIZE	MIN. EMBEDMENT	TENSION LOAD
3/8"	4"	3,000 LBS.
1/2"	5"	6,000 LBS.
5/8"	6"	8,000 LBS.
3/4"	7"	12,000 LBS.
7/8"	9"	15,000 LBS.
NO. 3	4"	3,000 LBS.
NO. 4	5"	6,000 LBS.
NO. 5	6"	8,000 LBS.
NO. 6	7"	12,000 LBS.
NO. 7	9"	15,000 LBS.

10. TEST ANCHORS IN THE PRESENCE OF THE SPECIAL INSPECTOR.
11. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
12. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING.
13. TEST QUANTITY OF EXPANSION AND ADHESIVE ANCHORS FOR EACH SIZE AND TYPE AS NOTED BELOW:

APPLICATION	QUANTITY
EXPANSION AND SCREW ANCHORS	50%
EXPANSION ANCHORS AFTER TEST FAILURE OF ONE ANCHOR	100% OF 20 PREVIOUS
ADHESIVE ANCHORS	25%
ADHESIVE ANCHORS W/ GREATER THAN 10% FAILURE AFTER TESTING OF THE FIRST 100 ANCHORS	100%
ADHESIVE ANCHORS W/ LESS THAN 5% FAILURE AFTER TESTING OF THE FIRST 100 ANCHORS	10%

14. APPLY THE TEST LOAD TO EXPANSION ANCHORS PER EITHER THE HYDRAULIC RAM METHOD OR THE TORQUE WRENCH METHOD. APPLY THE TEST LOAD TO SCREW ANCHORS PER THE TORQUE WRENCH METHOD APPLY THE TEST LOAD TO ADHESIVE ANCHORS PER THE HYDRAULIC RAM METHOD.
15. THE FOLLOWING CRITERIA APPLY FOR THE TESTING AND ACCEPTANCE OF INSTALLED ANCHORS:
    A. HYDRAULIC RAM METHOD: MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST, E.G.AS EVIDENCED BY LOOSENING OF THE WASHER UNDER THE NUT.
    B. TORQUE WRENCH METHOD: ATTAIN THE SPECIFIED TORQUE WITHIN 1/2 TURN OF THE NUT.

XI. STRUCTURAL TESTS, INSPECTIONS, AND OBSERVATIONS

1. AN INDEPENDENT TESTING AGENCY AND SPECIAL INSPECTORS WILL BE PAID BY THE CONTRACTOR AND APPROVED BY THE VA TO PERFORM THE TESTS AND INSPECTION REQUIRED BY SPEC 014529. PROVIDE ACCESS AND FURNISH SAMPLES TO THE AGENCY AS REQUIRED BY THE CONTRACT DOCUMENTS.
2. IF INITIAL TESTS OR INSPECTIONS MADE BY THE TESTING AGENCY REVEAL THAT ANY PORTION OF THE WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS WILL BE MADE AT THE CONTRACTOR'S EXPENSE.
3. TESTING LAB SHALL BE PAID BY CONTRACTOR.

XII. DESIGN CRITERIA

1. APPLICABLE CODES / DESIGN STANDARDS:

   VA STRUCTURAL DESIGN MANUAL FOR HOSPITAL PROJECTS
   VA PROGRAM GUIDE PG-18-1, MASTER CONSTRUCTION SPECIFICATIONS
   VA PROGRAM GUIDE PG-18-3, DESIGN AND CONSTRUCTION PROCEDURES
   VA PROGRAM GUIDE PG-18-15, A/E/ SUBMISSION INSTRUCTIONS, VOLUME B
   VA HANDBOOK H-18-8, SEISMIC DESIGN REQUIREMENTS, FEBRUARY 2011
   DEPARTMENT OF DEFENSE 2010 UNIFIED FACILITIES CRITERIA (UFC)
   2015 IBC
   ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
   2010 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION STEEL BUILDINGS
   2010 AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SEISMIC PROVISIONS
   2014 AMERICAN CONCRETE INSTITUTE (ACI) 318

2. FOUNDATIONS HAVE BEEN DESIGNED WITH THE FOLLOWING CRITERIA:

   SPREAD FOOTINGS:
   ALLOWABLE NET SOIL PRESSURE FOR DL + LL = 2000 PSF
   ALLOWABLE NET SOIL PRESSURE FOR DL + LL + EQ = 2600 PSF

3. GRAVITY LOADS:

   A. DEAD LOADS - VARY BASED ON ACTUAL BUILDING AND EQUIPMENT OPERATING WEIGHTS
   B. LIVE LOADS:
      FLOOR 100 PSF
      ROOF 20 PSF

4. SNOW LOADS = 0 PSF
5. FLOOD LOADS = 0 LBS
6. SEISMIC DESIGN: RESPONSE SPECTRUM

   WHERE:R = 5.5 FOR SPECIAL CMU WALL.

   Ω = 2.5
   I = 1.25
   SS = 0.634
   S1 = 0.257
   SDS = 0.551
   SD1 = 0.264
   SITE CLASS = D
   OCCUPANCY CATEGORY = III
   SEISMIC DESIGN CATEGORY = D

   INELASTIC INTERSTORY DRIFT LIMIT = 0.01 X STORY HT.

7. WIND DESIGN:
   BASIC WIND SPEED = 115 MPH
   WIND EXPOSURE = B
8. EQUIPMENT AND UTILITY LINES SUPPORTED BY STRUCTURAL FRAMING ARE TO BE CONCENTRICALLY CONNECTED TO THE FRAMING MEMBERS. BRACING TO THE BOTTOM 2/3 OF BEAMS IS PROHIBITED.
9. EQUIPMENT, ARCHITECTURAL ELEMENTS, AND DISTRIBUTION SYSTEMS ARE TO BE ANCHORED AND BRACED PER SPECIFICATION 130541.
10. DESIGN TEAM

JAMES O MALLEY	SENIOR PRINCIPAL
ROBERT GRAFF	PROJECT MANAGER
ANDREW MA	DESIGNER
JUN SANCHEZ	SENIOR CAD SPECIALIST
MIGUEL MARASIGAN	SENIOR CAD SPECIALIST

100% CONSTRUCTION DOCUMENTS
NOVEMBER 19, 2015

Revisions: Date

CONSULTANTS:

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ARCHITECT

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Drawing Title: GENERAL NOTES	Project Title: EXPAND COMMUNITY LIVING CENTER		Project Number: 570-218
			Building Number: 31
Approved: Project Director	Location: 2615 EAST CLINTON AVE FRESNO,CA 93703		Drawing Number: SS001
	Date: 09/01/14	Checked: RG	Drawn: JQS

Office of Construction and Facilities Management
Department of Veterans Affairs

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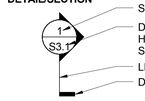
REFERENCE SYMBOLS

DETAIL/SECTION



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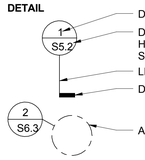
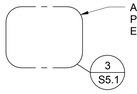
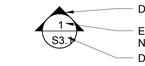


DETAIL PLAN OR ELEVATION



SINGLE ELEVATION



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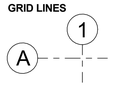
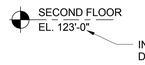
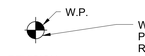


LEVEL LINE



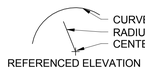
WORK POINT



REVISION

KEY NOTE

CENTER POINT OF CURVE



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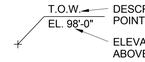
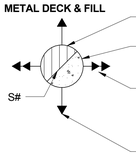
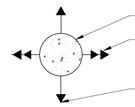


PLAN SYMBOLS

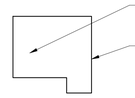
METAL DECK & FILL



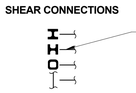
CONCRETE FILL



FLOOR DEPRESSIONS

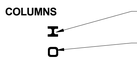


SHEAR CONNECTIONS

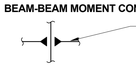


MOMENT CONNECTIONS

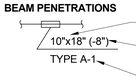
COLUMNS



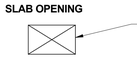
BEAM-BEAM MOMENT CONNECTION



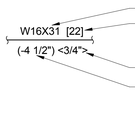
BEAM PENETRATIONS



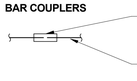
SLAB OPENING



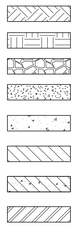
BEAM DESIGNATIONS



BAR COUPLERS



MATERIAL SYMBOLS



ABBREVIATIONS

(E)	EXISTING
#	NUMBER
&	AND
@	AT
ø	DIAMETER
ℓd	DEVELOPMENT LENGTH
ℓdh	HOOK DEVELOPMENT LENGTH
ℓs	LAP SPLICE LENGTH
AA	ADHESIVE ANCHOR
ABV	ABOVE
ADD'L	ADDITIONAL
ADJ	ADJACENT
AGGR	AGGREGATE
ALUM	ALUMINUM
ALT	ALTERNATE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APPROX	APPROXIMATE
AR	ANCHOR ROD
ARCH	ARCHITECTURAL / ARCHITECT
ASTM	AMERICAN SOCIETY for TESTING and MATERIALS
ASPH	ASPHALT
AC	ASPHALT CONCRETE
AWG	AMERICAN WIRE GAUGE
BF	BOTH FACES
BLDG	BUILDING
BLK, BLKG	BLOCK or BLOCKING
BM, BMS	BEAM, BEAMS
BN	BOUNDARY NAILING
BO	BOTTOM OF
BOF	BOTTOM OF FOOTING
BOT	BOTTOM
BRG	BEARING
BSMT	BASEMENT
BRBF	BUCKLING RESTRAINED BRACE FRAME
BS	BOTH SIDES
BTWN	BETWEEN
BW	BOTH WAYS
C	CHANNEL
CIP	CAST IN PLACE
CJ	CONSTRUCTION JOINT
CJP	COMPLETE JOINT PENETRATION
CLG	CEILING
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS
CSK	COUNTERSINK
CTR	CENTER
d	PENNY (NAIL SIZE)
DBL	DOUBLE
DK, DKG	DECK or DECKING
DEMO	DEMOLITION
DET, DETS	DETAIL, DETAILS
DIAG	DIAGONAL
DIM, DIMS	DIMENSION, DIMENSIONS
DIST	DISTANCE
DN	DOWN
DO	DITTO
DP	DEEP
DWL, DWLS	DOWEL, DOWELS
DWG, DWGS	DRAWING, DRAWINGS
EA	EACH
EBF	ECCENTRIC BRACE FRAME
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMBED	EMBEDMENT
EN	EDGE NAILING
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EACH SIDE
EW	EACH WAY
EXCAV	EXCAVATION
EXP	EXPANSION
EXT	EXTERIOR
FF	FAR FACE
FDN	FOUNDATION
FIN	FINISH
FLG	FLANGE
FLR, FLRS	FLOOR, FLOORS
FN	FIELD NAILING
FO	FACE OF
FOC	FACE OF CONCRETE
FOS	FACE OF STUDS
FP	FIREPROOFING
FS	FAR SIDE
FT	FOOT OR FEET
FTG, FTGS	FOOTING, FOOTINGS
GA	GAGE
GALV	GALVANIZED
GLB	GLU-LAM BEAM
GRND	GROUND
GR	GRADE
HDG	HOT DIPPED GALVANIZED
HDR	HEADER
HP	HIGH POINT
HSB	HIGH STRENGTH BOLTS
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
HK, HKS	HOOKS
HORIZ, (H)	HORIZONTAL
ID	INSIDE DIAMETER/DIMENSION
INFO	INFORMATION
JST, JSTS	JOIST, JOISTS
JT	JOINT
KO	KNOCK-OUT
L	ANGLE
LP	LOW POINT
LEV	LEVEL
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LOC	LOCATION
LONGIT	LONGITUDINAL
LT	LIGHT
LWC	LIGHTWEIGHT CONCRETE
MAX	MAXIMUM
MB	MACHINE BOLT
MEZZ	MEZZANINE
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, PLUMBING DOCUMENTS
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MTL	METAL
MTD	MOUNTED
NF	NEAR FACE
NIC	NOT IN CONTRACT
NOM	NOMINAL (DIAMETER)
NS	NEAR SIDE
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OC	ON CENTER
OD	OUTSIDE DIAMETER/DIMENSION
OPH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
PC, PCS	PIECE, PIECES
PCC	PRECAST CONCRETE
PERP	PERPENDICULAR
PJP	PARTIAL JOINT PENETRATION
PL	PLATE
PLYWD	PLYWOOD
PTN	PARTITION
RO	ROUGH OPENING
R	RADIUS
REBAR	REINFORCING BAR
REF	REFERENCE
REINF	REINFORCED or REINFORCING
REQ'D	REQUIRED
REV	REVISION
RFG	ROOFING
RSJ	ROLLED STEEL JOIST
SAD	SEE ARCHITECTURAL DOCUMENTS/DRAWINGS
SCHED	SCHEDULE
SECT	SECTION
SHT	SHEET
SHTG	SHEATHING
SIM	SIMILAR
SL	SLOPE
SMF	SPECIAL MOMENT FRAME
SMS	SHEET METAL SCREW
SOG	SLAB ON GRADE
SPEC, SPECS	SPECIFICATION, SPECIFICATIONS
SPSW	SPECIAL PLATE SHEAR WALL
SQ	SQUARE
SS	STAINLESS STEEL
STAG	STAGGER or STAGGERED
STD	STANDARD
STIF	STIFFENER
STIR	STIRRUP or STIRRUPS
STL	STEEL
STRUCT	STRUCTURAL
SUB	SUBSTITUTE
SUSP	SUSPENDED
SYMM	SYMMETRICAL
T&B	TOP and BOTTOM
T&G	TONGUE and GROOVE
THK	THICK
THRD	THREADED
THRU	THROUGH
TO	TOP OF
TOC	TOP OF CONCRETE
TOS	TOP OF STEEL
TR	TREAD
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
URM	UNREINFORCED MASONRY
VENT	VENTILATE
VERT, (V)	VERTICAL
VIF	VERIFY IN FIELD
W or WF	WIDE FLANGE
W/	WITH
W/O	WITHOUT
WD	WOOD
WP	WORK POINT
WT	WEIGHT/TEE SECTION
WWM	WELDED WIRE MESH
X HVY	EXTRA HEAVY
XX HVY	DOUBLE EXTRA HEAVY
X STR	EXTRA STRONG
XX STR	DOUBLE EXTRA STRONG

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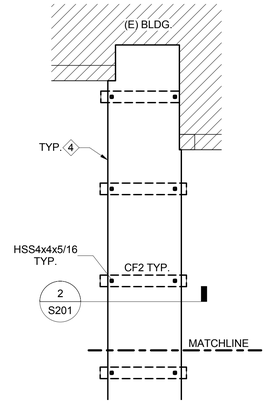
Drawing Title: SYMBOLS AND ABBREVIATIONS	Project Title: EXPAND COMMUNITY LIVING CENTER		Project Number: 570-218
			Building Number: 31
Approved: Project Director	Location: 2615 EAST CLINTON AVE FRESNO,CA 93703		Drawing Number: SS002
	Date: 09/01/14	Checked: RG	Drawn: JQS

Office of Construction and Facilities Management

Department of Veterans Affairs



2 FOUNDATION PLAN - CANOPY
1/8" = 1'-0"

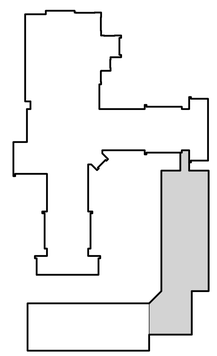
1 LEVEL 1 NORTH WING FOUNDATION PLAN
1/8" = 1'-0"

SHEET NOTES

1. FINISHED FLOOR ELEVATION, S.A.D.
2. TOP OF FOUNDATION = -1'-0" U.O.N.
3. FOUNDATION ELEVATION ON PLAN REF. FROM TYP. TOP OF FOUNDATION.

KEY NOTES

1. 5" S.O.G. W/ #4 @ 18" O.C.
2. HSS 6x6x5/16 COLUMN, TYP. U.O.N.
3. HSS 8x8x5/8 COLUMN.
4. THICKENED SLAB EDGE PER 4/S401.
5. SLAB DEPRESSION, S.A.D.



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Drawing Title	LEVEL 1 NORTH WING FOUNDATION PLAN
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date / Checked / Drawn	09/01/14 / RG / JQS
Project Number	570-218
Building Number	31
Drawing Number	SF101

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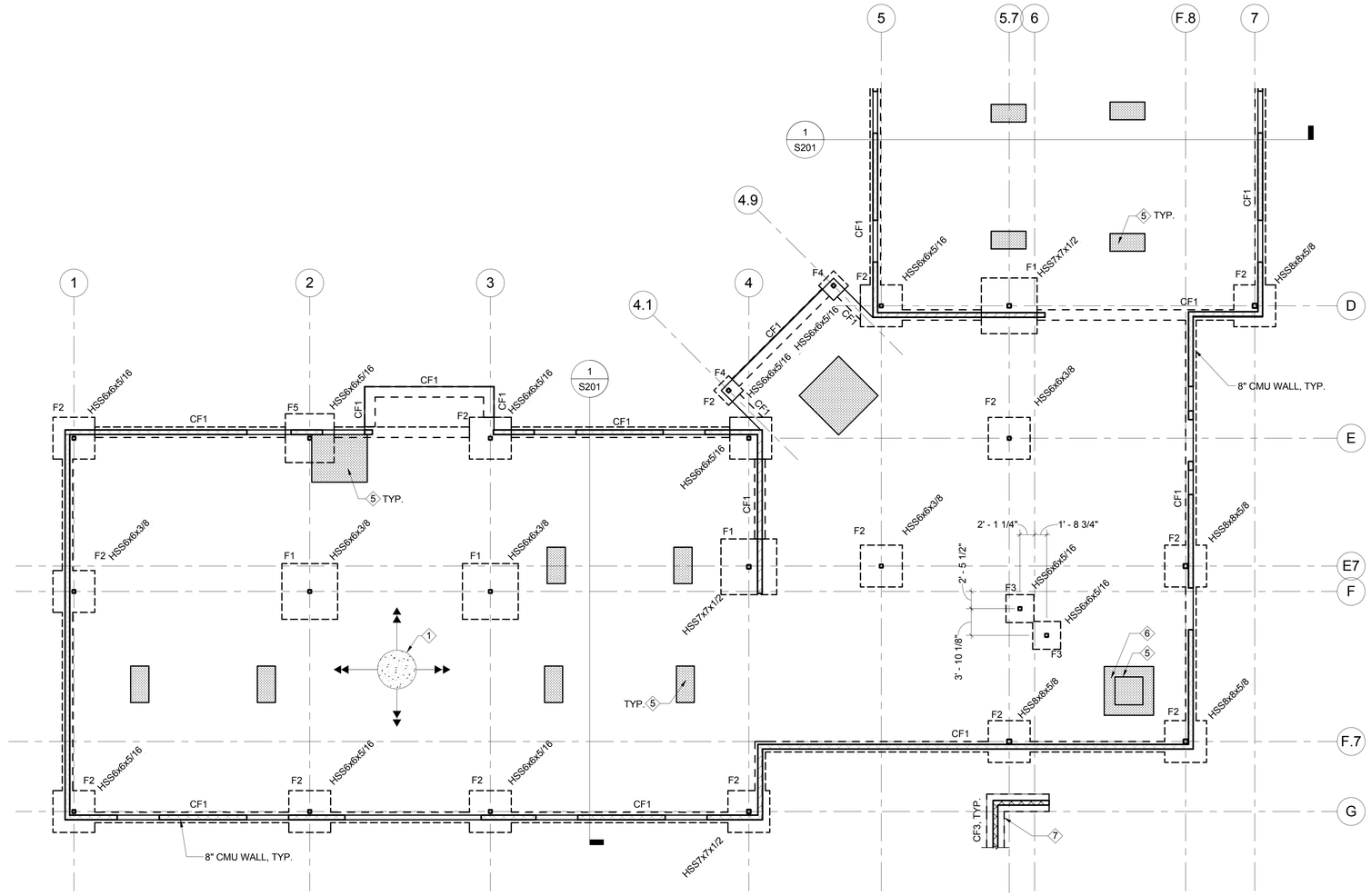


1 LEVEL 1 WEST WING FOUNDATION PLAN

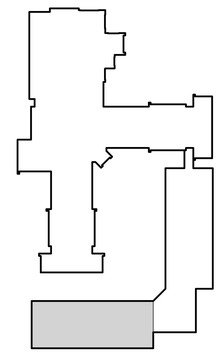
1/8" = 1'-0"

SHEET NOTES

1. FINISHED FLOOR ELEVATION, S.A.D.
2. TOP OF FOUNDATION = -1'-0" U.O.N.
3. FOUNDATION ELEVATION ON PLAN REF. FROM TYP. TOP OF FOUNDATION.

KEY NOTES

1. 5" S.O.G. W/ #4 @ 18" O.C.
2. HSS 6x6x5/16 COLUMN, TYP. U.O.N.
3. HSS 8x8x5/8 COLUMN.
4. THICKENED SLAB EDGE PER 4/S401.
5. SLAB DEPRESSION, S.A.D.
6. HOUSEKEEPING PAD S.M.D. AND 2/S401.
7. 8" CMU BLAST WALL PER 8/S701. S.A.D. FOR LOCATION. TOP OF FND. @ -1'-0" BELOW FINISHED GRADE.



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Drawing Title: LEVEL 1 WEST WING FOUNDATION PLAN	Project Title: EXPAND COMMUNITY LIVING CENTER
Approved: Project Director	Location: 2615 EAST CLINTON AVE, FRESNO,CA 93703
Date: 09/01/14	Checked: RG
Drawn: JQS	Project Number: 570-218
Building Number: 31	Drawing Number: SF102

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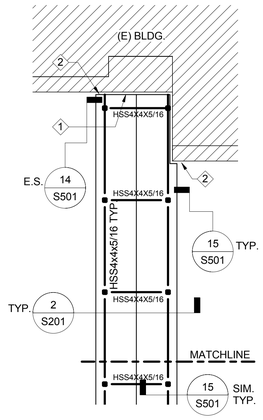


2 ROOF FRAMING PLAN - CANOPY

1/8" = 1'-0"

1 ROOF NORTH WING FRAMING PLAN

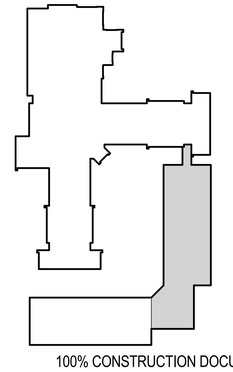
1/8" = 1'-0"

SHEET NOTES

1. TOP OF DECK = 12'-6" U.O.N.
2. TOP OF STEEL = 12'-0 1/2" U.O.N.

KEY NOTES

1. STOP HSS 4" CLR. OF FACE OF BLDG.
2. 4" GAP BETWEEN STRUCTURES. S.A.D. FOR COVER AT ROOF.
3. PROVIDE 4 - 9"Ø BEAM PENETRATIONS CENTERED IN THE BEAM HT. AND EQUALLY SPACED ALONG THE LENGTH. PROVIDE STUDS @ 6" O.C. ALONG BM. LENGTH. COORDINATE W/ MECH. SUB.
4. 3- #7 BARS. ALIGN W/ LOW FLUTES OF DECK. 2 MAX. PER FLUTE.
5. 2- #7 BARS. ALIGN W/ LOW FLUTES OF DECK. 2 MAX. PER FLUTE.
6. FRAME AROUND OPENING PER 17B/S601, SIM.



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Drawing Title	LOW ROOF NORTH WING FRAMING PLAN
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date	09/01/14
Checked	RG
Drawn	JQS
Project Number	570-218
Building Number	31
Drawing Number	SF103

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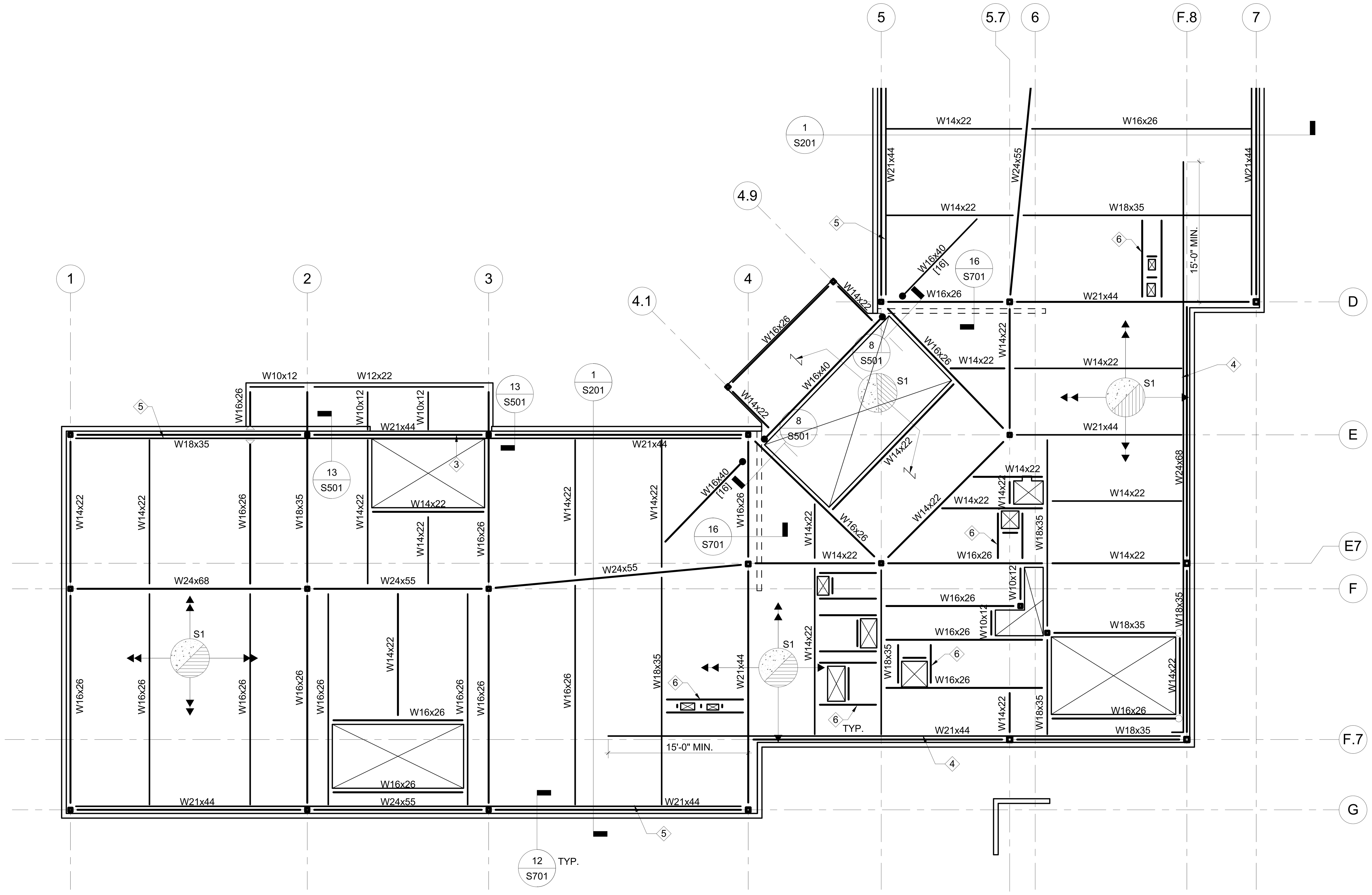


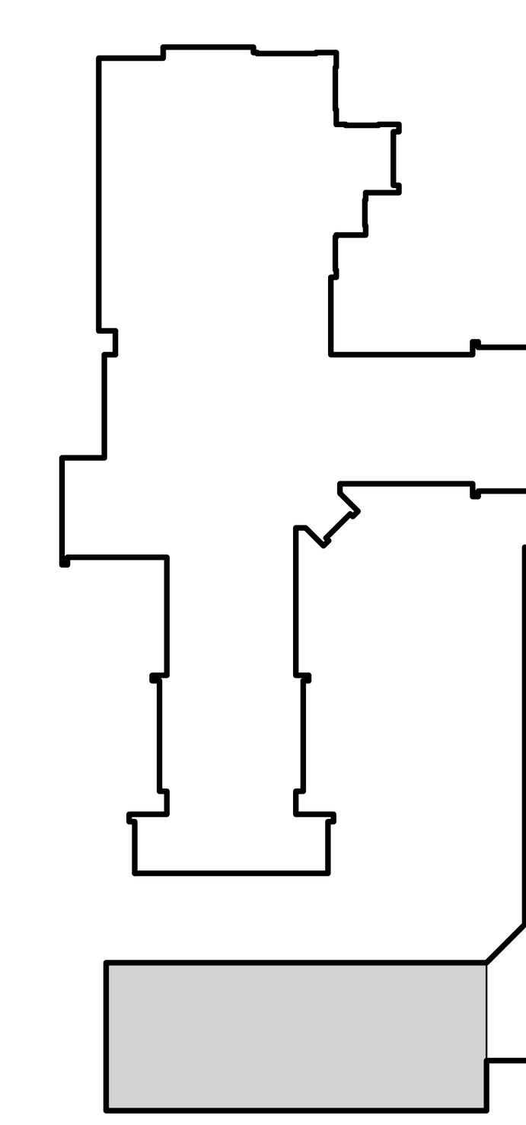
1 ROOF WEST WING FRAMING PLAN
1/8" = 1'-0"

SHEET NOTES

1. TOP OF DECK = 12'-6" U.O.N.
2. TOP OF STEEL = 12'-0 1/2" U.O.N.

KEY NOTES

1. NOT USED.
2. NOT USED.
3. PROVIDE 4 - 9"Ø BEAM PENETRATIONS CENTERED IN THE BEAM HT. AND EQUALLY SPACED ALONG THE LENGTH. PROVIDE STUDS @ 6" O.C. ALONG BM. LENGTH.COORDINATE W/ MECH. SUB.
4. 3- #7 BARS. ALIGN W/ LOW FLUTES OF DECK. 2 MAX. PER FLUTE.
5. 2- #7 BARS. ALIGN W/ LOW FLUTES OF DECK. 2 MAX. PER FLUTE.
6. FRAME AROUND OPENING PER 17B/S601, SIM.



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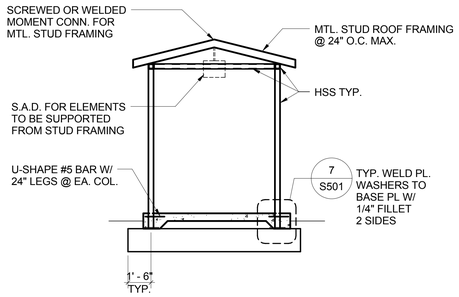
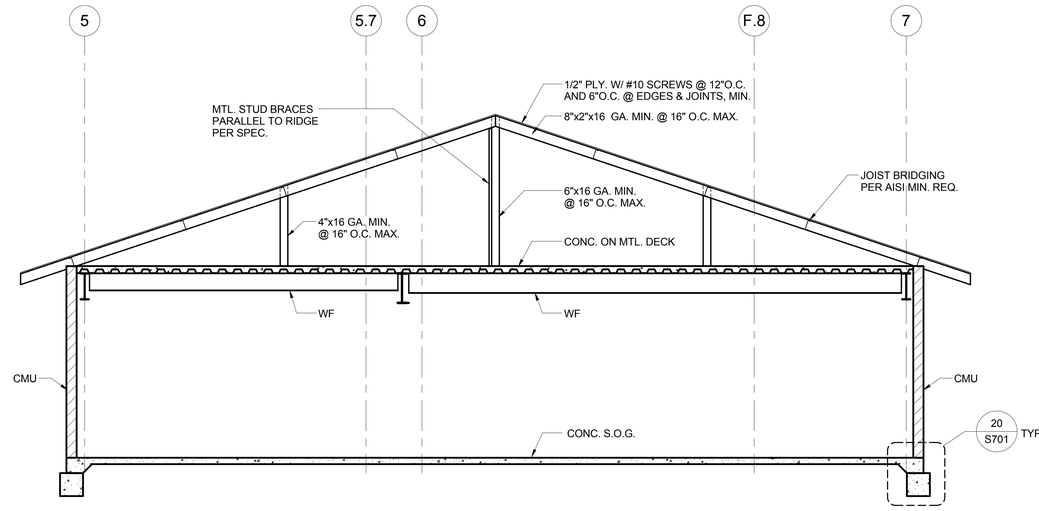
Drawing Title	Project Title	Project Number
LOW ROOF WEST WING FRAMING PLAN	EXPAND COMMUNITY LIVING CENTER	570-218
Approved: Project Director	Location: 2615 EAST CLINTON AVE FRESNO,CA 93703	Building Number: 31
	Date: 09/01/14 / Checked: RG / Drawn: JQS	Drawing Number: SF104

Office of Construction and Facilities Management
Department of Veterans Affairs



NOTE:
METAL STUD ROOF FRAMING IS DESIGN BUILD PER SPECIFICATION 54000.

2 WALKWAY CANOPY
1/4" = 1'-0"



NOTE:
METAL STUD ROOF FRAMING IS DESIGN BUILD PER SPECIFICATION 54000.

1 TYPICAL ROOF SECTION
1/4" = 1'-0"

100% CONSTRUCTION DOCUMENTS
NOVEMBER 19, 2015

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Revisions:	Date

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Exp. 6/30/17
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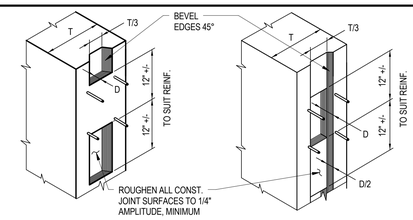
Drawing Title: BUILDING SECTIONS	Project Title: EXPAND COMMUNITY LIVING CENTER	Project Number: 570-218	Office of Construction and Facilities Management
		Building Number: 31	
Approved: Project Director	Location: 2615 EAST CLINTON AVE FRESNO,CA 93703	Drawing Number: S201	Department of Veterans Affairs
	Date: 09/01/14 / Checked: RG / Drawn: JQS		

A BAR SPACING FOR NON-SPLICED BARS

B BAR SPACING FOR BARS SPLICED WITH A NON-CONTACT LAP

9 BAR SPACING IN CONCRETE
N.T.S.



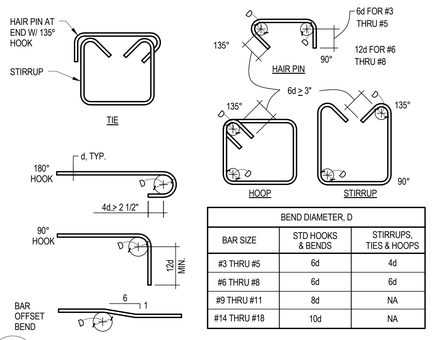
KEY TYPE #1
TYP. FOR NON-RETAINING WALLS

KEY SCHEDULE	
T	D
< 8"	3/4"
8"-16"	1 1/2"
>16"	2 1/2"

KEY TYPE #2
FOR SLABS, RETAINING WALLS & BASEMENT WALLS BELOW GRADE

NOTE:
1. DETAILS APPLY TO BOTH (H) AND (V) CONSTRUCTION JOINTS.
2. SEE 7/- SIM. FOR CONST. JT. IN WALLS THICKER THAN 18".

5 CONST. JOINTS
N.T.S.



BEND DIAMETER, D		
BAR SIZE	STD HOOKS & BENDS	STIRRUPS, TIES & HOOPS
#3 THRU #5	6d	4d
#6 THRU #8	6d	6d
#9 THRU #11	8d	NA
#14 THRU #18	10d	NA

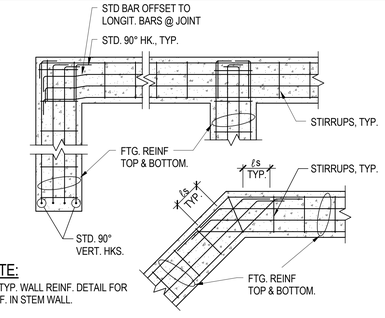
1 HOOKS & BENDS
N.T.S.

CONCRETE REINFORCING DEVELOPMENT & SPLICE LENGTHS

BAR LOCATION	CONCRETE TYPE	CONCRETE STRENGTH	BAR SIZE	#3			#4			#5			#6			#7			#8			#9			#10			#11			#14		#18	
				ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓs	ℓdh	ℓd	ℓdh	ℓd	ℓdh
CONC. WALL VERT. REINF., COLUMNS, BEAM BOT. REINF., SLAB-ON-GRADE FOOTING BOT. REINF.	NWC	f'c ≥ 3ksi		17	22	6	22	29	8	28	36	10	33	43	12	48	63	14	55	72	16	62	81	18	70	91	20	78	101	22	93	38	124	50
CONC. WALL HORIZ. REINF., FTG TOP & SIDE REINF., BEAM TOP & SIDE REINF.	NWC	f'c ≥ 3ksi		22	28	6	29	38	8	36	47	10	43	56	12	63	81	14	72	93	16	81	105	18	91	118	20	101	131	22	121	38	161	50
FILL ON METAL DECK	LWC	f'c ≥ 3ksi		22	28	9	29	38	11	36	47	14	43	56	17	63	81	20	72	93	22	81	105	25	91	118	28	101	131	31	121	38	161	50

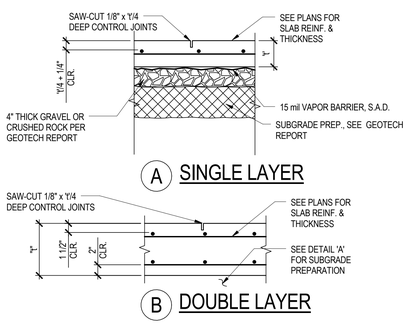
NOTES:
1. ℓd = DEVELOPMENT LENGTH
ℓs = LAP SPLICE LENGTH
ℓdh = HOOK DEVELOPMENT LENGTH
2. WHEN SPLICING BARS OF DIFFERENT SIZE, USE LAP SPLICE LENGTH OF LARGER BAR, U.O.N.
3. STAGGER EA. SPLICES AS
4. MULTIPLY THE ABOVE LENGTHS BY 1.5 FOR EPOXY COATED REINF.

18 REINFORCING DEVELOPMENT & SPLICE LENGTHS
N.T.S.



NOTE:
SEE TYP. WALL REINF. DETAIL FOR REINF. IN STEM WALL.

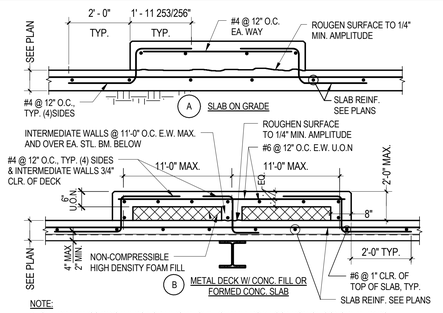
8 FOOTING REINF. CORNERS & INTERSECTIONS
N.T.S.



A SINGLE LAYER

B DOUBLE LAYER

6 SOG CONTROL JT & SUBGRADE PREP
N.T.S.



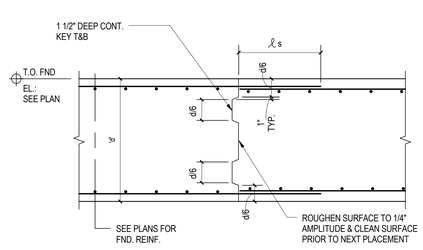
NOTE:
1. SEE DRAWINGS BY OTHERS FOR HEIGHT, SIZE, CHAMFERS & LOCATION OF CONCRETE PADS.
2. COORDINATE REINF. LOCATIONS TO AVOID INTERFERENCE WITH INSTALLATION OF POST-INSTALLED EXPANSION ANCHORS IF USED.

2 HOUSEKEEPING PAD
N.T.S.

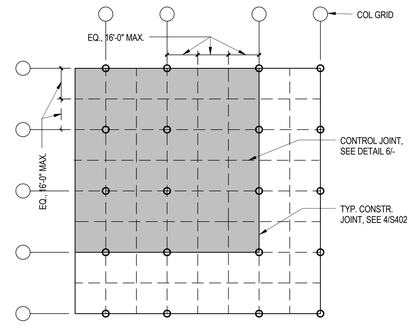
FOOTING SCHEDULE

MARK	'H' DEPTH	'B' LENGTH	'D' WIDTH	BOTTOM REINFORCING LONG.	BOTTOM REINFORCING TRANSVERSE	TOP REINFORCING LONG.	TOP REINFORCING TRANSVERSE	SLAB DOWELS	DETAIL	REMARKS
F1	2'-0"	8'-0"	8'-0"	9-#6	9-#6	-	-	4-#5	12/-	
F2	1'-6"	6'-0"	6'-0"	7-#6	7-#6	-	-	4-#5	12/-	
F3	1'-6"	4'-0"	4'-0"	5-#6	5-#6	-	-	4-#5	12/-	
F4	1'-6"	3'-0"	3'-0"	4-#6	4-#6	-	-	4-#5	12/-	
F5	1'-6"	7'-0"	7'-0"	8-#6	8-#6	-	-	4-#5	12/-	
CF1	1'-6"	N/A	1'-6"	3-#5	#6 @ 16" O.C.	-	-	-	16/-	
CF2	1'-6"	N/A	1'-6"	3-#5	#4 TIES @ 8" O.C.	3-#5	#4 TIES @ 8" O.C.	-	12/-, SIM.	
CF3	2'-0"	N/A	2'-6"	4-#5	#4 TIES @ 8" O.C.	4-#5	#4 TIES @ 8" O.C.	-	16/-, SIM.	

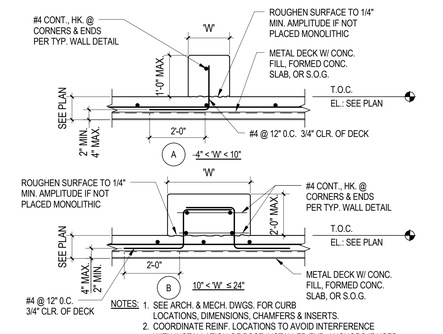
19 FOOTING SCHEDULE
N.T.S.



11 FOUNDATION CONSTRUCTION JOINT
N.T.S.

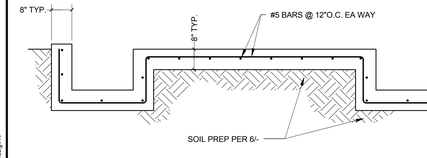


7 CONTROL JTS. IN SOG
N.T.S.



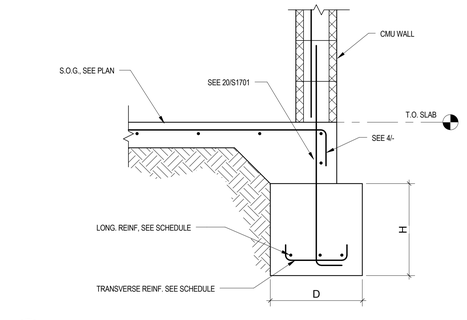
NOTES:
1. SEE ARCH. & MECH. DWGS. FOR CURB LOCATIONS, DIMENSIONS, CHAMFERS & INSERTS.
2. COORDINATE REINF. LOCATIONS TO AVOID INTERFERENCE WITH INSTALLATION OF POST-INSTALLED EXP. ANCHORS IF USED.

3 CONCRETE CURB
N.T.S.

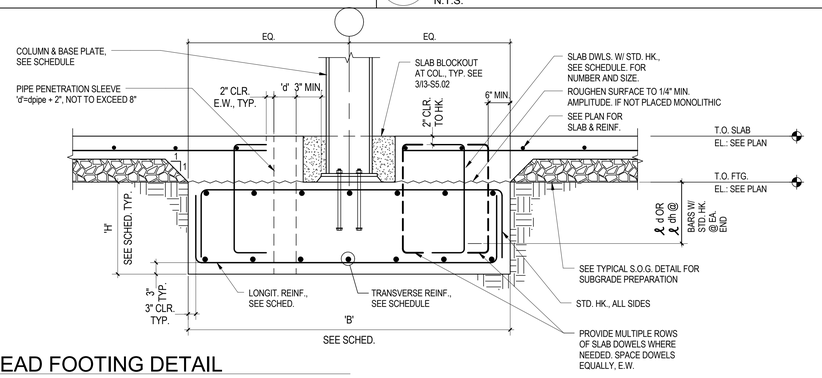


NOTE:
1. SEE DRAWINGS BY OTHERS FOR DIMS. AND LOCATION.
2. EPOXY DOWEL BARS INTO (E) ADJ. PAD 6" MIN.

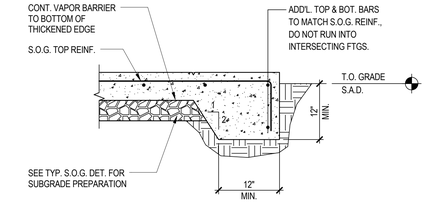
20 TRANSFORMER PAD
1/2" = 1'-0"



16 CONTINUOUS FOOTING DETAIL
1" = 1'-0"



12 SPREAD FOOTING DETAIL
N.T.S.



4 SOG FREE EDGE
N.T.S.

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NOVEMBER 19, 2015

Revisions:	Date

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Drawing Title	TYPICAL CONCRETE DETAILS
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date	09/01/14
Checked	RG
Drawn	JQS
Project Number	570-218
Building Number	31
Drawing Number	S401

Office of Construction and Facilities Management
Department of Veterans Affairs

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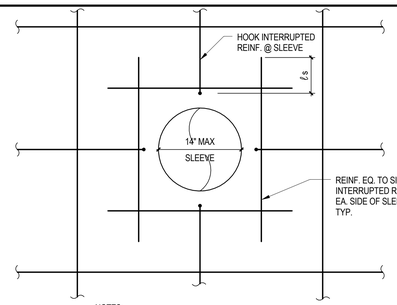
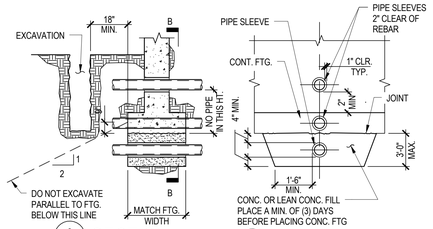


NOTES:
1. PROVIDE 2x LARGER DIAM. SLEEVE CLEAR BETWEEN SLEEVES MIN.
2. WHERE NO REINF. IS INTERRUPTED NO ADDITIONAL REINF. IS REQ.

5 SLEEVES IN WALLS AND SLABS
N.T.S.

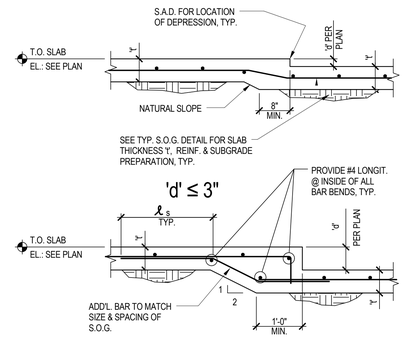


A SECTION

B ELEVATION

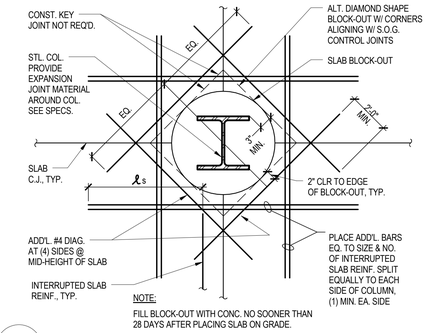
NOTES:
1. REINFORCE OPENINGS FOR SLEEVES OR GROUPS OF SLEEVES LARGER THAN 10" PER THE TYP. WALL OPENING DET. "GROUPS" OF SLEEVES INCLUDES ALL SLEEVES SPACED CLOSER THAN ONE DIAMETER CLR.
2. SEE MECH. & CIVIL DWGS. FOR ADD'L. REQUIREMENTS AT SLEEVES.
3. DEPTH OF FOOTING MAY BE DETERMINED BY LOCATION OF PIPES. CONSULT WITH MECH. CONTRACTOR TO DETERMINE EXACT DEPTH.

6 SLEEVES AT FTG.
N.T.S.



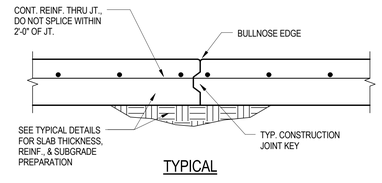
NOTE:
COORDINATE LOCATION, DEPTH, EXTENT, AND EDGE CONDITIONS OF DEPRESSIONS WITH ARCHITECTURAL DRAWINGS.

2 SOG DEPRESSION
N.T.S.



NOTE:
FILL BLOCK-OUT WITH CONC. NO SOONER THAN 28 DAYS AFTER PLACING SLAB ON GRADE.

3 SOG BLOCKOUT
N.T.S.



TYPICAL

NOTE:
CONSTRUCTION JOINTS TO BE USED ONLY WHERE SHOWN ON PLAN OR SPECIFICALLY APPROVED.

4 S.O.G. CONSTRUCTION JOINT
N.T.S.

100% CONSTRUCTION DOCUMENTS
NOVEMBER 19, 2015

Revisions: | Date

CONSULTANTS:

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Drawing Title	TYPICAL CONCRETE DETAILS
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date	09/01/14
Checked	RG
Drawn	JQS
Project Number	570-218
Building Number	31
Drawing Number	S402

Office of Construction and Facilities Management
Department of Veterans Affairs

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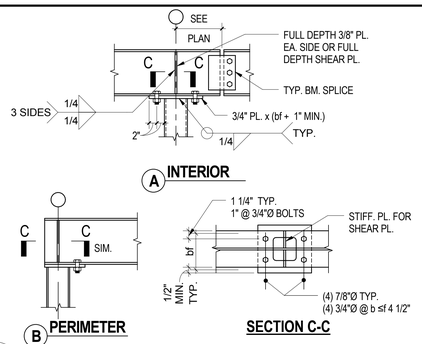
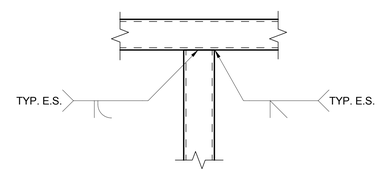


13 SEATED WF TO HSS COLUMN

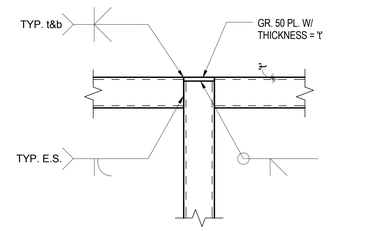
3/4" = 1'-0"



NOTE:
GRIND WELDS PER AESS

14 SEATED HSS TO HSS COLUMN

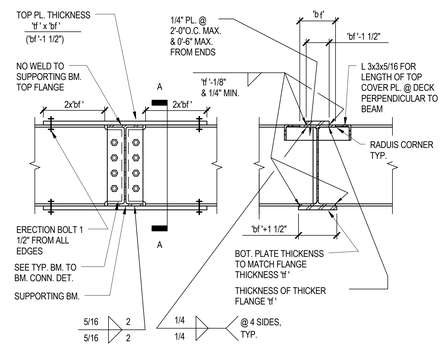
1 1/2" = 1'-0"



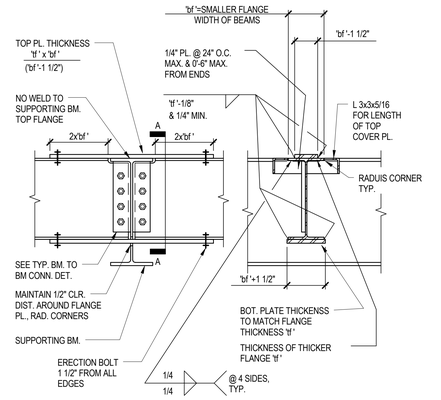
NOTE:
GRIND WELDS PER AESS

15 HSS BEAM TO HSS COLUMN

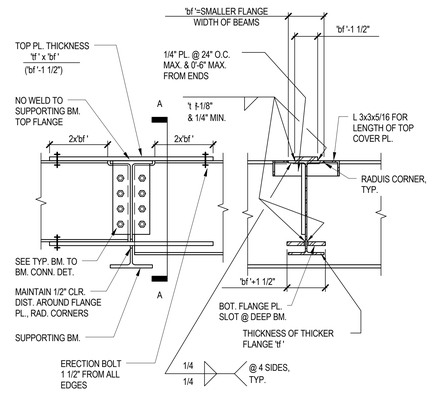
1 1/2" = 1'-0"



A BEAM WITH SAME DEPTH



B BEAM WITH VARYING DEPTH



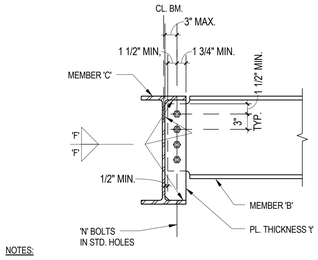
C STEPPED BEAMS

NOTES:

1. FLANGE PL. TO BE ASTM A572, GR. 50.
2. ERECTION BOLTS ALLOWED ONLY WERE INDICATED
3. NOTED ON PLAN.

12 BM TO BM MOMENT CONN.

1" = 1'-0"

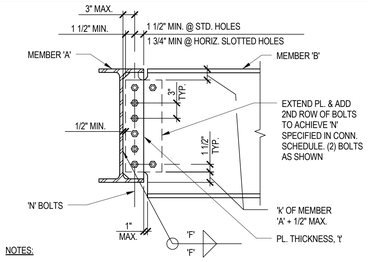


NOTES:

1. MEMBER 'B' NOTED ON PLAN.
2. FOR WELD SIZE 'F', NUMBER OF BOLTS 'N' & PLATE THICKNESS 't' . SEE BOLTED BEAM SCHEDULE.

5 FULL DEPTH BRACING CONN. SINGLE ROW MEMBER 'B' > W10

N.T.S.

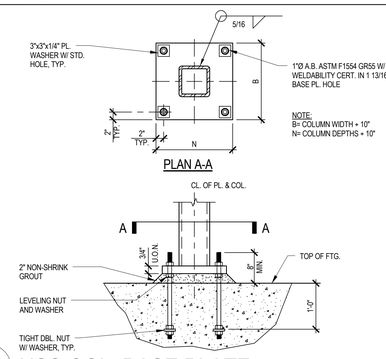


NOTES:

1. SIMILAR CONDITIONS APPLY FOR BEAMS ON BOTH SIDES OF MEMBER 'A'.
2. FOR WELD SIZE 'F' NUMBER OF BOLTS 'N' AND PLATE THICKNESS 't', SEE BOLTED BEAM TO BEAM CONNECTION SCHEDULE.
3. SHORT SLOTTED HOLES IN SHEAR PLATE MAY BE USED.
4. IF MEMBER 'B' < 8", SEE BM TO BM CONN.-MEMBER 'A' > MEMBER 'B'.
5. BASE PARAMETERS IN CONNECTION SCHEDULE ON DEPTH OF MEMBER 'B'.

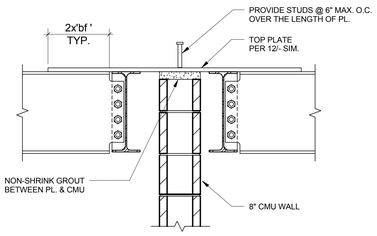
6 BM TO BM A≤B

N.T.S.



7 HSS COL. BASE PLATE

N.T.S.



NOTES:
1. NOTED ON PLAN

8 DRAG CONNECTION

1" = 1'-0"

BEAM CONNECTION SCHEDULE

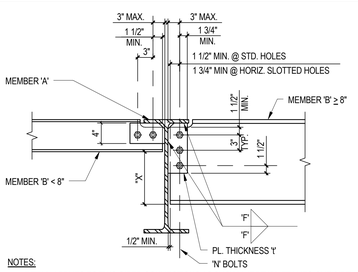
NOMINAL MEMBER DEPTH	SHEAR PL. THICKNESS (T)	FASTENERS A325-X U.O.N. (N')	WELD SIZE (F')	REMARKS
LESS THAN 12"	3/8"	(2) 7/8"Ø	5/16"	
12" THRU 14"	3/8"	(3) 7/8"Ø	5/16"	
16" THRU 18"	1/2"	(4) 7/8"Ø	3/8"	
21"	1/2"	(5) 7/8"Ø	3/8"	
24"	1/2"	(6) 7/8"Ø	3/8"	
27"	1/2"	(7) 7/8"Ø	3/8"	
30"	1/2"	(8) 7/8"Ø	3/8"	
33"	1/2"	(9) 7/8"Ø	3/8"	
36"	1/2"	(10) 7/8"Ø	3/8"	
40"	1" GR 50	(11) 7/8"Ø	5/8"	A490
44"	1" GR 50	(12) 7/8"Ø	5/8"	A490

NOTES:

1. BOLT HOLES TO BE STD. HOLES 1/16" GREATER THAN BOLT DIAMETER. SHORT SLOTTED HOLES IN SHEAR PLATE MAY BE USED ONLY WHERE NOTED.
2. BOLTS TO BE FULLY PRETENSIONED W/ HARDENED WASHERS, U.O.N.
3. SCHEDULE BASED ON NOMINAL DEPTH OF WIDE FLANGE BEAMS, CHANNELS, TUBES, & OTHER MISCELLANEOUS SHAPES.

1 BOLTED BEAM SCHEDULE DETAIL

N.T.S.

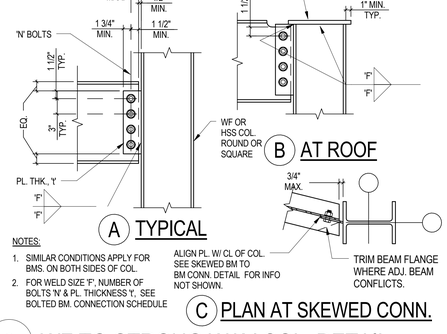


NOTES:

1. DETAIL APPLIES TO TWO SIDED CONNECTIONS AS SHOWN, IF BEAM FRAMES ONE SIDE ONLY OR X > 4", USE FULL DEPTH BEAM TO BEAM CONNECTION DETAIL.
2. FOR WELD 'F', NUMBER OF BOLTS 'N', AND PLATE THICKNESS 't', SEE BOLTED BEAM CONNECTION SCHEDULE.
3. SHORT SLOTTED HOLES IN SHEAR PLATE MAY BE USED.

2 BEAM TO BEAM CONNECTION DETAIL

N.T.S.

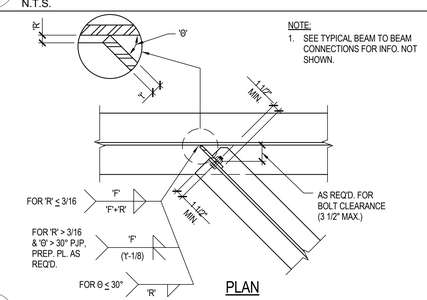


NOTES:

1. SIMILAR CONDITIONS APPLY FOR BMS. ON BOTH SIDES OF COL.
2. FOR WELD SIZE 'F', NUMBER OF BOLTS 'N' & PL. THICKNESS 't', SEE BOLTED BM. CONNECTION SCHEDULE
3. ALIGN PL. W/ CL OF COL. SEE SKEWED BM TO BM CONN. DETAIL FOR INFO NOT SHOWN.

3 WF TO STRONG WAY COL. DETAIL

N.T.S.



NOTE:
1. SEE TYPICAL BEAM TO BEAM CONNECTIONS FOR INFO. NOT SHOWN.

4 SKEWED BEAM

N.T.S.

100% CONSTRUCTION DOCUMENTS
NOVEMBER 19, 2015

Revisions:	Date

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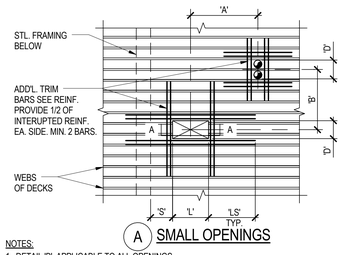
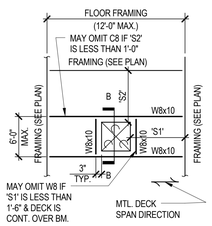
Drawing Title	TYPICAL STEEL DETAILS
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date	09/01/14
Checked	RG
Drawn	JQS
Project Number	570-218
Building Number	31
Drawing Number	S501

Office of Construction and Facilities Management

Department of Veterans Affairs

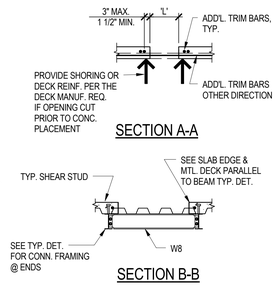


A SMALL OPENINGS



NOTE: PROVIDE TRIM BARS PER (A)

B FRAMED OPENING



SECTION A-A

SECTION B-B

NOTES:
1. DETAIL 'B' APPLICABLE TO ALL OPENINGS.
2. DETAIL 'A' APPLICABLE ONLY TO OPENINGS SATISFYING ALL OF THE FOLLOWING:
   'D' ≤ 16" (DIM. PERPENDICULAR TO DECK SPAN)
   'L' ≤ 4'-0" (DIM. PARALLEL TO DECK SPAN)
   'A' OR 'B' GREATER THAN 4D & 32", WHERE 'D' IS MAX. OF ADJACENT OPENINGS (DIMENSIONS TO ADJACENT OPENINGS).
3. HOLES UP TO 6" MAX. IN BOTH DIRECTIONS MAY OMIT TRIM REINF. PROVIDED NO MORE THAN ONE RIB IS CUT.

17 COMPOSITE DECK OPENING REINFORCING
N.T.S.

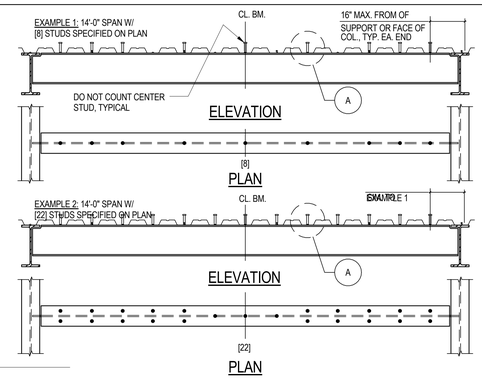
SLAB TYPE	DECK TYPE	HEIGHT	GAUGE	FACTORY VENTED	MINIMUM SECTION PROPERTIES			FILL		STUD SIZE	SLAB REINFORCING	ATTACHMENT TYPE				SIDE LAP	REMARKS
												TO PERIMETER SUPPORT		TO INTERMEDIATE SUPPORT			
					I (IN^4)	S_+ (IN^3)	S_- (IN^3)	TYPE	MINIMUM THICKNESS ABOVE TOP FLUTE			PERPENDICULAR TO DECK	PARALLEL TO DECK	PERPENDICULAR TO DECK	PARALLEL TO DECK		
S1	W	3"	18	YES	1.203	0.763	0.767	LWC	2 1/2	3/4"Ø x4 1/2"	#4 @ 12"O.C. E.W.	1/2"Ø PUDDLE WELD AT ALL DOWN FLUTES	1/2"Ø PUDDLE WELD @ 12" O.C.	1/2"Ø PUDDLE WELD AT ALL DOWN FLUTES	1/2"Ø PUDDLE WELD @ 12" O.C.	BUTTON PUNCH	-

NOTES:
1. WHENEVER POSSIBLE, DECK LAYOUTS SHALL PROVIDE SHEETS OF SUFFICIENT LENGTH TO SPAN CONTINUOUSLY ACROSS AT LEAST THREE SPANS. ENDS SHALL TERMINATE OVER A SUPPORT PERPENDICULAR TO THE DECK SPAN, EXCEPT AT OPENINGS OR BUILDING EDGES WHERE DECKS MAY BE CANTILEVERED.
2. PROVIDE A MINIMUM OF 2" BEARING AT SUPPORTING MEMBERS PERPENDICULAR TO DECK SPAN AND 1 1/2" AT MEMBERS PARALLEL TO DECK SPAN.
3. DIAMETER OF PUDDLE WELD SHOWN REPRESENTS EFFECTIVE FUSION AREA.
4. EACH PUDDLE WELD SHOWN MAY BE REPLACED WITH A SHEAR STUD WELDED THROUGH DECK. CONCRETE FILL THICKNESS SHOWN ON FRAMING PLANS AND DETAIL SHEETS ARE MINIMUM THICKNESS. PROVIDE ADDITIONAL CONCRETE FILL AS REQUIRED TO COMPENSATE FOR BEAM OR DECK DEFLECTIONS AND TO MAINTAIN SURFACE TOLERANCES SPECIFIED.

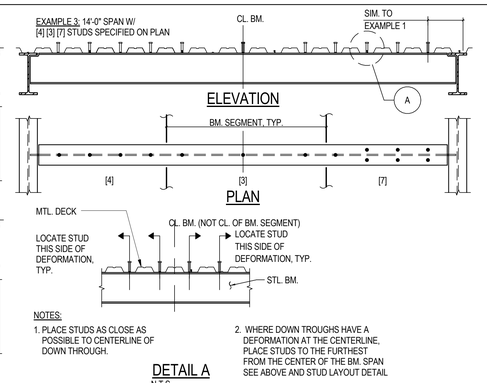
10 METAL DECK SCHEDULE
N.T.S.

NOTES:
1. MIN. NO. OF STUDS REQUIRED PER SEGMENT OF BM IS SHOWN AS [..] ON FRAMING PLANS.
2. WHERE NO INDICATION IS GIVEN, PROVIDE STUDS AT 1'-0" O.C. MAX.
3. FOR DECK PARALLEL TO BM UNIFORMLY SPACE STUDS ALONG CENTERLINE OF BM SEGMENT NO CLOSER THAN 4 1/2" O.C., SEE STUD PLACEMENT DET. FOR MORE INFO.
4. FOR DECK PERPENDECULAR OR SKEWED TO BM, PLACE REQUIRED NO. OF STUDS EQUALLY ALONG THE LENGTH OF BM SEGMENT FOR SPACING GREATER THAN 2'-0" O.C. OTHERWISE USE THE FOLLOWING METHOD:

STEP A: PLACE STUDS IN ALTERNATE THROUGHS STARTING AT EA. END.
STEP B: PLACE ONE HALF OF REMAINING STUDS AT EA. END IN THE REMAINING THROUGHS STARTING AT THE END SUPPORT.
STEP C: AFTER A STUD HAS BEEN PLACED IN EA. THROUGH, PLACE A SECOND STUD PER THROUGH STARTING AT EA. END. SIM. FOR THREE STUDS PER THROUGH UNTIL THE PLAN SPECIFICATION [..] HAS BEEN MET. SEE EXAMPLES 1, 2 & 3.

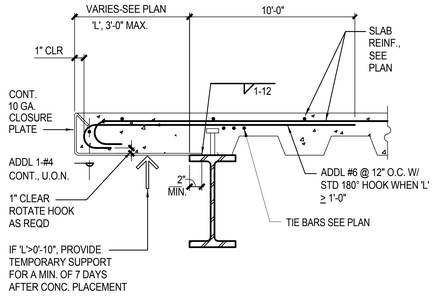


11 STUD PLACEMENT DETAIL
N.T.S.

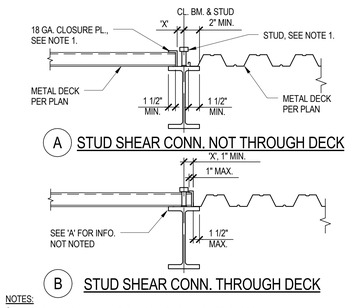


NOTES:
1. PLACE STUDS AS CLOSE AS POSSIBLE TO CENTERLINE OF DOWN THROUGH.
2. WHERE DOWN TROUGHS HAVE A DEFORMATION AT THE CENTERLINE, PLACE STUDS TO THE FURTHEST FROM THE CENTER OF THE BM. SPAN SEE ABOVE AND STUD LAYOUT DETAIL

DETAIL A
N.T.S.



16 SLAB EDGE NOT @ CMU WALL
N.T.S.

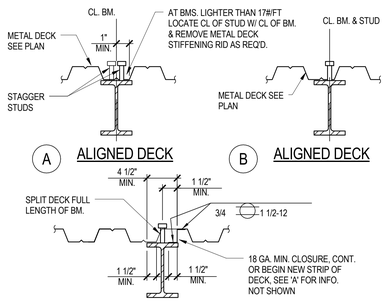


A STUD SHEAR CONN. NOT THROUGH DECK

B STUD SHEAR CONN. THROUGH DECK

NOTES:
1. AT LOCATIONS WHERE CLEARANCE 'X' TO STUD PLATE IS LESS THAN 1 1/2", PROVIDE CLOSURE BELOW HIGH FLUTES ONLY AND PLACE STUDS ADJACENT TO OPEN LOW FLUTES.

12 DECK TRANSITION
N.T.S.

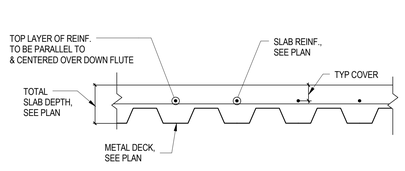


A ALIGNED DECK B ALIGNED DECK

C OFFSET DECK

NOTE:
1. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR INSERTS NOT SHOWN.

8 STUD DETAIL
N.T.S.



NOTES:
1. PROVIDE REINF. LAYERS A SHOWN U.O.N.
2. SEE S/I3-S7.00 FOR CONDITION AT SLAB EDGE.

4 METAL DECK CONC. FILL
NTS

100% CONSTRUCTION DOCUMENTS
NOVEMBER 19, 2015

Revisions:	Date

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Drawing Title	TYPICAL METAL DECK DETAILS
Approved: Project Director	
Project Title	EXPAND COMMUNITY LIVING CENTER
Location	2615 EAST CLINTON AVE FRESNO,CA 93703
Date	09/01/14
Checked	RG
Drawn	JQS
Project Number	570-218
Building Number	31
Drawing Number	S601

Office of Construction and Facilities Management
Department of Veterans Affairs

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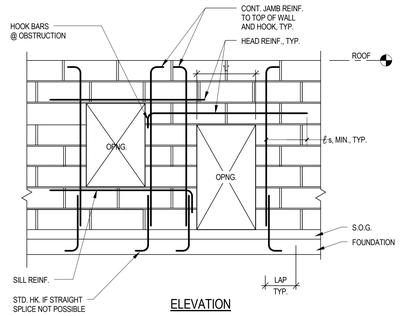
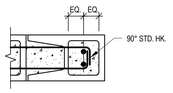
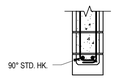


ELEVATION

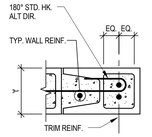


JAMB AT DBL. CURTAIN

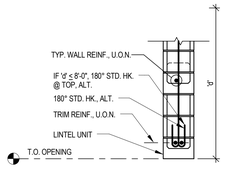


NOTE: SIM. @ SILL

HEAD & SILL AT DBL. CURTAIN



JAMB AT SINGLE CURTAIN



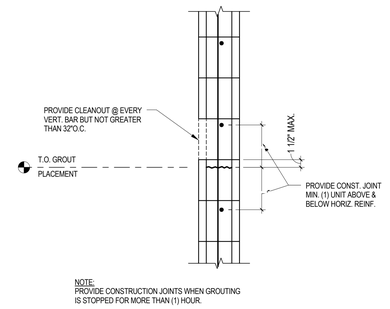
NOTE: SIM. @ SILL

HEAD & SILL AT SINGLE CURTAIN

TRIM REINFORCING SCHEDULE					
NOMINAL WALL THICKNESS 't'	MINIMUM TRIM REINF.	'L' (ft.)	JAMB REINF.	HEADER REINF.	SILL REINF.
8"	2-#5	8'	2-#5	2-#5	2-#5

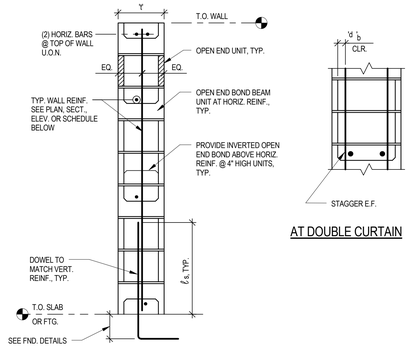
NOTES:
1. SCHED. REINF. NOT REQ'D. FOR OPNGS. SMALLER THAN 16" SQ.
2. MINIMUM TRIM REINF. TO BE LARGER OF TYP. WALL REINF. OR SIZE SHOWN IN SCHEDULE.
3. AT SERIES OF OPNGS WHERE PIER OR SPANDREL IS NARROWER THAN 120d, RUN TRIM REINF. CONTINUOUS.

11 WALL REINFORCING AT OPENINGS
N.T.S.



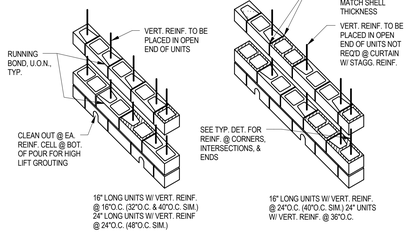
NOTE:
PROVIDE CONSTRUCTION JOINTS WHEN GROUTING IS STOPPED FOR MORE THAN (1) HOUR.

5 HORIZONTAL CONTROL JOINT
N.T.S.



SECTION

AT DOUBLE CURTAIN



ISOMETRIC (DBL. CURTAIN SIM.)

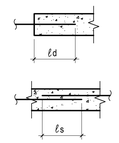
WALL REINFORCING SCHEDULE			
NOM. WALL THICKNESS 't'	VERT. REINF.	HORIZ. REINF.	REMARKS
8"	#5 @ 16" O.C.	#4 @ 24" O.C.	

NOTES:
1. ALL CELLS TO BE FULLY GROUTED.
2. SECURE REINF. AT 200 db MAX.
3. CONCAVE JOINTS, TYP. U.O.N.

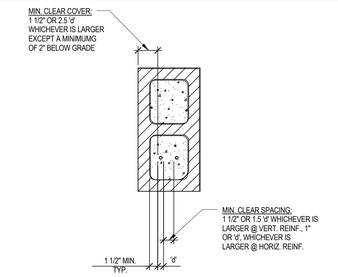
8 WALL REINFORCING
N.T.S.

CMU REINF. DEVELOPMENT & SPLICE LENGTHS																	
BAR LOCATION	CONC. MASONRY UNIT		BAR SIZE														
	TYPE	STRENGTH	#3		#4		#5		#6		#7		#8		#9		
			ℓd	ℓs	ℓd	ℓs	ℓd	ℓs	ℓd	ℓs	ℓd	ℓs	ℓd	ℓs	ℓd	ℓs	
CONC. MASONRY UNIT	-	f'm ≥1.5ksi	25	25	33	33	41	41	49	49	57	57	65	65	74	74	

NOTES:
1. ℓd = DEVELOPMENT LENGTH
2. ℓs = LAP SPLICE LENGTH



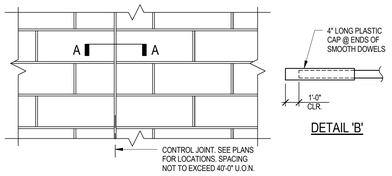
1 CMU REINF. DEVELOPMENT SCHEDULE
N.T.S.



WHERE 'd' = DIAMETER OF LARGER BAR

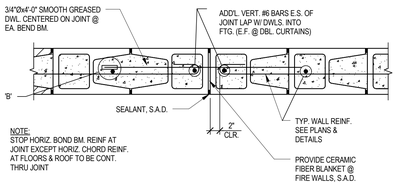
PLAN

2 BAR SPACING IN MASONRY
N.T.S.



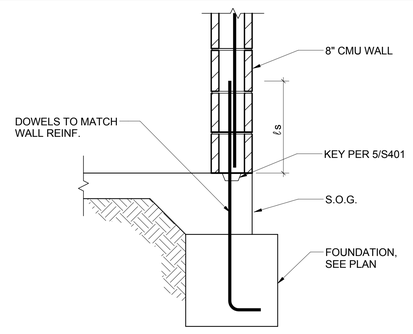
DETAIL 'B'

ELEVATION

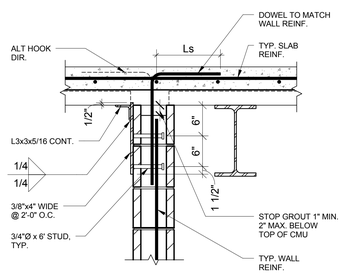


SECTION A-A

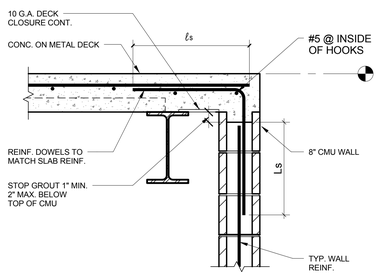
4 CONTROL JOINT
N.T.S.



20 CMU BASE CONNECTION
1" = 1'-0"



16 INTERIOR CMU CONN @ TOP
1" = 1'-0"



12 CMU TOP SUPPORT
1" = 1'-0"

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NOVEMBER 19, 2015

Revisions: | Date

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Drawing Title: TYPICAL CMU DETAILS
Approved: Project Director

Project Title: EXPAND COMMUNITY LIVING CENTER
Location: 2615 EAST CLINTON AVE FRESNO,CA 93703
Date: 09/01/14 | Checked: RG | Drawn: JQS

Project Number: 570-218
Building Number: 31
Drawing Number: S701

Office of Construction and Facilities Management
Department of Veterans Affairs

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