

GENERAL NOTES

GENERAL NOTES

I. GENERAL

1. MATERIALS AND WORKMANSHIP TO CONFORM TO THE 2009 EDITION OF THE INTERNATIONAL BUILDING CODE AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
2. THESE GENERAL NOTES SUPPLEMENT THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS. IN CASE OF CONFLICT BETWEEN THE PLANS AND SPECIFICATIONS, CONTACT THE ENGINEER.
3. REFERENCE TO CODES, RULES, REGULATIONS, STANDARDS, MANUFACTURER'S INSTRUCTIONS OR REQUIREMENTS OF REGULATORY AGENCIES IS TO THE LATEST PRINTED EDITION OF EACH IN EFFECT AT THE DATE OF SUBMISSION OF BID UNLESS THE DOCUMENT DATE IS SHOWN.
4. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW BY THE ENGINEER.
5. DETAILS ON SHEETS TITLED 'TYPICAL DETAILS' APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. SUCH DETAILS ARE NOT NOTED AT EACH LOCATION THAT THEY OCCUR.
6. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND FOR CHECKING DIMENSIONS. NOTIFY THE OWNER'S REPRESENTATIVE OF ANY DISCREPANCIES AND RESOLVE BEFORE PROCEEDING WITH THE WORK.
7. DO NOT SCALE THE DRAWINGS.
8. PROVIDE MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES INCLUDE, BUT MAY NOT BE LIMITED TO, BRACING AND SHORING FOR LOADS DURING CONSTRUCTION. RETAIN A REGISTERED CIVIL ENGINEER WHOM IS PROPERLY QUALIFIED TO DESIGN BRACING, SHORING, ETC. VISITS TO THE SITE BY THE OWNER'S REPRESENTATIVE WILL NOT INCLUDE OBSERVATION OF THE ABOVE NOTED ITEMS.
9. INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENTS THE PRESENT KNOWLEDGE, BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE OWNER'S REPRESENTATIVE. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE OWNER'S REPRESENTATIVE.
10. REFER TO ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF FLOOR, ROOF AND WALL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS. COORDINATE THE SIZE AND LOCATION OF OPENINGS ASSOCIATED WITH, BUT NOT LIMITED TO, ELECTRICAL, MECHANICAL AND PLUMBING TRADES. SUBMIT FINAL SIZING AND LOCATION REQUIREMENTS OF OPENINGS TO THE OWNER'S REPRESENTATIVE FOR REVIEW.
11. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING A SAFE PLACE TO WORK AND MEETING THE REQUIREMENTS OF ALL APPLICABLE JURISDICTIONS. EXECUTE WORK TO ENSURE THE SAFETY OF PERSONS AND ADJACENT PROPERTY AGAINST DAMAGE BY FALLING DEBRIS AND OTHER HAZARDS IN CONNECTION WITH THIS WORK.
12. UTILIZE CURRENT TECHNOLOGY DETECTION EQUIPMENT TO LOCATE OBSTACLES (REBAR, ELEC. ETC) WITHIN CONCRETE (FLOORS, WALL, CEILINGS, ROOFS, ETC) AT EVERY LOCATION WHERE CONCRETE IS TO BE PENETRATED (DRILLING, SAWING, ETC). PROVIDE RESULTS TO THE CONTRACTING OFFICER/TECHNICAL REPRESENTATIVE (COTR) AT LEAST 48 HOURS PRIOR TO THE PENETRATION ACTION. REPAIR ANY OBSTACLES DAMAGED.
13. WHEN DRILLING NEW DOWELS INTO EXISTING CONCRETE, TAKE ALL MEASURES POSSIBLE TO AVOID DAMAGE TO EXISTING REBAR. LOCATE EXISTING REINFORCING USING X-RAYS AND PACHOMETER SCANS.

II. FOUNDATION AND SITE WORK

1. THE DESIGN OF THE FOUNDATION SYSTEM IS BASED UPON THE CRITERIA AND RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT ENTITLED "GEOTECHNICAL INVESTIGATION FOR VA - RENO CAMPUS ADMINISTRATION BUILDING SEISMIC RETROFIT (SVA261-P-0888)" BY WOOD RODGERS, RENO, NV, DATED SEPTEMBER 30, 2010. THE REPORT IS AVAILABLE FOR REVIEW.
2. LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
3. REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED.
4. NOTIFY THE OWNER'S REPRESENTATIVE IF ANY BURIED STRUCTURES NOT INDICATED, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC., ARE FOUND.
5. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
6. REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE.
7. EXCAVATIONS FOR FOUNDATIONS MUST BE ACCEPTED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING REINFORCING AND CONCRETE. NOTIFY THE GEOTECHNICAL ENGINEER WHEN EXCAVATIONS ARE READY FOR INSPECTION.
8. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE OR MASONRY HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.
9. MECHANICALLY COMPACT EXCAVATION BACKFILLS IN LAYERS. PROVIDE THE FOLLOWING MINIMUM COMPACTION IN ACCORDANCE WITH THE ASTM D1557 TEST METHOD:

MAXIMUM DRY DENSITY	LOCATIONS
90%	TRENCH AND WALL BACKFILL
90%	UPPER 6" OF SOIL BENEATH FILL
90%	FILL BENEATH SLAB ON GRADE
95%	FILL BENEATH FOOTINGS
90%	OTHER

10. FOR SPECIAL INSPECTION AND TESTING REQUIREMENTS, SEE THE GEOTECHNICAL REPORT AND SPECIFICATION SECTION 01 45 29.

III. FORMWORK

1. PROVIDE POUR POCKETS IN FORMS AND UNDER EXISTING STRUCTURAL MEMBERS AS REQUIRED TO PREVENT AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE EXISTING MEMBERS. CONCRETE CAST WITH AIR POCKETS AND/OR "HONEYCOMB" UNDER OR AROUND THE MEMBERS IS NOT ACCEPTABLE.
2. REMOVE FORMS AND SHORES IN ACCORDANCE WITH THE FOLLOWING:
- | LOCATION | REMOVE FORMS AND SHORES NO SOONER THAN |
|------------------------------------------------------------------------|----------------------------------------|
| BOTTOM FORMS AND SHORES FOR MILDLY REINFORCED SLABS, BEAMS AND GIRDERS | 7 DAYS, AND F'C = 3500 PSI MINIMUM |
| SIDE FORMS FOR BEAMS AND GIRDERS | 72 HOURS |
| COLUMNS AND WALLS | 72 HOURS |
| FOOTINGS, PILE CAPS, AND GRADE BEAMS | 48 HOURS |
3. PROVIDE CURING WHERE FORMS ARE REMOVED IN LESS THAN 7 DAYS INCLUDING BUT NOT LIMITED TO WALLS, COLUMNS, AND UNDERSIDE OF ELEVATED SLABS.

IV. REINFORCING STEEL

1. REINFORCING TO CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED:
- | LOCATION | TYPE |
|------------------------------------|-------------------|
| REINFORCING STEEL #7 AND SMALLER | ASTM A615, 60 KSI |
| REINFORCING STEEL #8 AND LARGER | ASTM A615, 60 KSI |
| AND REINFORCING STEEL TO BE WELDED | ASTM A706, 60 KSI |
| WELDED STEEL WIRE FABRIC | ASTM A185, 70 KSI |
| SMOOTH DOWELS IN SLAB ON GRADE | ASTM A36, 36 KSI |
2. ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCEMENT FROM DISPLACING DUE TO FORMWORK, CONSTRUCTION, OR CONCRETE PLACEMENT OPERATIONS. LOCATE AND SUPPORT REINFORCING BY METAL CHAIRS, RUNNERS, BOLSTERS, SPACERS, AND HANGERS AT A MAXIMUM 3-FOOT SPACING.
3. MECHANICAL COUPLERS: LENTON THREADED OR INTERLOCK COUPLERS BY ERICO, ICB0 #3967, CADWELD BY ERICO, ICB0 #3967, OR XTENDER BY HEATED REINFORCEMENT CORPORATION, ICB0 #6309. COUPLERS FOR BEAM AND SLAB BARS AT FORMED CONSTRUCTION JOINTS MAY BE LENTON FORM SAVERS BY ERICO, ICB0 #3967.
4. WELD REINFORCING STEEL IN ACCORDANCE WITH AWS D1.4 USING QUALIFIED WELDERS.
5. TERMINATE REINFORCING STEEL IN STANDARD HOOKS, UNLESS OTHERWISE SHOWN.
6. PROVIDE REINFORCING SHOWN OR NOTED CONTINUOUS IN LENGTHS AS LONG AS PRACTICABLE.

V. CAST-IN-PLACE CONCRETE

1. CONCRETE IS REINFORCED AND CAST-IN-PLACE UNLESS OTHERWISE NOTED. WHERE REINFORCING IS NOT SPECIFICALLY SHOWN OR WHERE DETAILS ARE NOT GIVEN, PROVIDE REINFORCING SIMILAR TO THAT SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE OWNER'S REPRESENTATIVE. SUBSTITUTION OF SHOTCRETE FOR CAST-IN-PLACE CONCRETE IS NOT ACCEPTABLE.
2. ROUGHEN CONCRETE SURFACES OF CONSTRUCTION JOINTS TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES. LOCATE CONSTRUCTION JOINTS AS SHOWN ON THE DRAWINGS. SUBMIT ALTERNATE JOINT LOCATIONS OR JOINTS NOT SHOWN TO THE OWNER'S REPRESENTATIVE FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH THE WORK.
3. AT LOCATIONS WHERE CONCRETE IS CAST AGAINST EXISTING CONCRETE, ROUGHEN CONTACT SURFACES TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES.
4. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF ADDITIONAL CONCRETE CURBS AND HOUSEKEEPING PADS NOT SHOWN.
5. CONCRETE CLEAR COVER TO REINFORCING BARS IS AS FOLLOWS, UNLESS OTHERWISE NOTED:

LOCATION	CLEAR COVER
CONCRETE PLACED AGAINST EARTH	3 INCHES
FORMED SURFACES EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:	
#6 BARS AND LARGER	2 INCHES
#6 BARS AND SMALLER	1 ½ INCHES
SLABS ON GRADE (TOP CLEARANCE)	1 ½ INCHES
BEAMS, GIRDERS AND COLUMNS NOT EXPOSED TO WEATHER OR EARTH	1 ½ INCHES
WALL OR SLAB SURFACES NOT EXPOSED TO WEATHER OR EARTH:	
#8 & SMALLER	¾ INCH
#6 & #7	1 INCH

CONCRETE TYPES:

CLASS	28-DAY STRENGTH	TYPE	LOCATION
A	4,000 PSI	NORMAL WEIGHT	FOUNDATIONS, MISC. CURBS, HOUSE - KEEPING PADS, ETC.
B	4,000 PSI	NORMAL WEIGHT	SLABS ON GRADE
C	4,000 PSI	NORMAL WEIGHT	WALLS AND COLUMNS
D	4,000 PSI	LIGHT WEIGHT	FILL ON METAL DECK

7. CONTINUOUSLY MOIST CURE CONCRETE SLABS-ON-GRADE FOR 7 DAYS MINIMUM. WATER FOG SPRAYS, PONDING, SATURATED ABSORPTIVE COVERS, OR MOISTURE RETAINING COVERS MAY BE USED. CURING COMPOUNDS ARE NOT ACCEPTABLE.
8. CONCRETE FILL THICKNESS SHOWN ON THE FRAMING PLANS ARE MINIMAL THICKNESSES. NO ALLOWANCES HAVE BEEN SHOWN FOR ADDITIONAL CONCRETE FILL REQUIRED TO COMPENSATE FOR FRAME, DECK, OR FORMWORK DEFLECTIONS TO MAINTAIN SURFACE TOLERANCES SPECIFIED.
9. NON-SHRINK GROUT, 7000 PSI: EUCLID CHEMICAL COMPANY'S "EUCO-NS", L&M CRYSTEX, MASTER BUILDERS "MASTERFLOW 713", OR FIVE STAR GROUT. WHERE HIGH FLUIDITY OR INCREASED PLACING TIME IS REQUIRED, USE EUCLID CHEMICAL COMPANY'S "EUCO HI-FLOW GROUT" OR MASTER BUILDERS "MASTERFLOW 928".

VI. STRUCTURAL STEEL

1. STRUCTURAL STEEL TO CONFORM TO THE FOLLOWING UNLESS OTHERWISE NOTED:

SECTIONS	TYPE
ROLLED SHAPES	
WIDE FLANGES	ASTM A992
CHANNELS, ANGLES, & OTHER	ASTM A36
PLATES	
COLUMN BASE PLATES	ASTM A572, GR 50
BRACE GUSSET PLATES	ASTM A572, GR 50
BEAM SHEAR CONNECTION PLATES	ASTM A36
COLUMN CONTINUITY PLATES	ASTM A572, GR 50
BEAM STIFFENER PLATES	ASTM A36
DECK CLOSURE PLATES	ASTM A36
OTHER	ASTM A572, GR 50
STEEL PIPE	ASTM A53 GRADE B
COLD FORMED HOLLOW STRUCTURAL SECTION (HSS)	ASTM A500 GRADE B
STAINLESS STEEL SHAPES, PLATES AND BARS	ASTM A276
BOLTS	ASTM A325X
MACHINE BOLTS	ASTM A307
ANCHOR BOLTS	ASTM F1554
ANCHOR RODS	ASTM A36
THREADED AND HANGER ROD	ASTM A572, GR50
WELDED SHEAR CONNECTORS	ASTM A 108, GRADE 1015 THROUGH 1020
WELDED THREADED STUDS	ASTM A 108, GRADE 1015 THROUGH 1020
NUTS FOR BOLTS AND MACHINE BOLTS	ASTM A563
HARDENED WASHERS	ASTM F436
UNHARDENED WASHERS	ASTM F844
PLAIN WASHERS	ANSI B18.22.1
BEVELLED WASHERS	ANSI B18.23.1

2. HOT DIP GALVANIZE IN ACCORDANCE WITH ASTM A123 AND ASTM A153 STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER. REPAIR GALVANIZING AFTER WELDING IN ACCORDANCE WITH ASTM A780.
3. STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO VIEW IN THE COMPLETED BUILDING ARE DESIGNATED ARCHITECTUALLY EXPOSED STRUCTURAL STEEL (AESS).
4. ARC-WELDING ELECTRODES/FILLER METALS TO BE LOW HYDROGEN TYPES E70XX, E70TXX OR E70XXX MINIMUM AS APPLICABLE. ELECTRODES WITH CHARPY V-NOTCH (CVN) TESTS VALUES OF A MINIMUM 20 FOOT-POUNDS AT -20 DEGREES FAHRENHEIT ARE TO BE USED AT THE FOLLOWING LOCATIONS:

- COMPLETE JOINT PENETRATION WELDS
- BEAM TO COLUMN MOMENT CONNECTIONS - INCLUDING FLANGE, WEB, AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
- BRACE CONNECTIONS - INCLUDING BRACE, GUSSET, BASE PLATES, BEAM STIFFENER PLATES, AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
- WELDS NOTED "CVN" ON THE DRAWINGS

5. WELDERS TO BE CERTIFIED BY AWS AND THE GOVERNING JURISDICTION.
6. WHERE FIELD WELDING IS NOTED, THE DESIGNATION IS GIVEN AS A SUGGESTED CONSTRUCTION PROCEDURE ONLY.
7. PROVIDE NATURAL CAMBER UP, UNLESS NOTED OTHERWISE, EXCEPT AT CANTILEVERS. AT CANTILEVERS PROVIDE CAMBER SUCH THAT TIP OF CANTILEVER IS ABOVE FINAL ELEVATION.
8. SPlice MEMBERS ONLY WHERE INDICATED.

VII. METAL DECKING

1. METAL FLOOR AND ROOF DECK TO HAVE MINIMUM SECTION PROPERTIES SHOWN ON SHEET 'TYPICAL DETAILS.'
2. ALL FLOOR AND ROOF DECK TO BE GALVANIZED IN ACCORDANCE WITH ASTM A653 COATING CLASS G60. REPAIR DAMAGED COATING.
3. WHERE POSSIBLE, LAYOUT METAL DECK TO SPAN AT LEAST THREE SPANS CONTINUOUSLY. TERMINATE ENDS OVER SUPPORTS EXCEPT AT OPENINGS OR BUILDING EDGES WHERE METAL DECKS MAY BE CANTILEVERED AS SHOWN.
4. SECURE FLOOR AND ROOF METAL DECK TO THE STEEL FRAMEWORK AND DECKED RESIDUE AS SHOWN. UNLESS OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS, MINIMUM DECK ATTACHMENT IS AS FOLLOWS:

- TYPE B ROOF DECK ATTACHMENT: (7) ¾" EFFECTIVE DIAMETER PUDDLE WELDS PER 36" WIDE SECTION AT TRANSVERSE SUPPORTS. ½" EFFECTIVE DIAMETER PUDDLE WELDS AT 12" O.C. AT PERIMETER AND LONGITUDINAL SUPPORTS. 1-1/2" TOP SEAM WELD AT 12" O.C. AT SIDE LAP CONNECTIONS.

VIII. MECHANICAL ANCHORS

1. EXPANSION OR WEDGE ANCHORS INTO CONCRETE: HILTI KB-TZ (ICC ESR-1917), ITW REDHEAD TRIBULOT (ICC ESR-2427), POWERS POWER-STUD (ICC ESR-2502), OR SIMPSON STRONG BOLT (ICC ESR-1771). INSTALL ANCHORS IN ACCORDANCE WITH ICC REPORT.
2. PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.
3. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.
4. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.
5. MINIMUM EMBEDMENT OF ANCHORS, UNLESS OTHERWISE NOTED:
- | ANCHOR DIA. | WEDGE EMBEDMENT |
|-------------|-----------------|
| ½" | 2" |
| 5/16" | - |
| 3/8" | 2 ½" |
| ½" | 3 ½" |
| 5/8" | 4" |
| ¾" | 4 ¾" |
| 1" | 6" |
6. ANCHORS WILL BE PROOF-TESTED BY OWNER'S TESTING AND INSPECTION AGENCY.

7. IF ANY ANCHOR FAILS TESTING, REPLACE ANCHOR AND TEST ADDITIONAL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME INITIAL TESTING FREQUENCY.
8. APPLY TEST LOAD BY ANY METHOD THAT WILL EFFECTIVELY MEASURE THE TENSION ON THE ANCHOR SUCH AS DIRECT PULL WITH A HYDRAULIC JACK, TORQUE WRENCH, OR CALIBRATED SPRING-LOADING DEVICES, ETC.
9. TEST ANCHORS NO SOONER THAN 24 HOURS AFTER INSTALLATION.
10. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWAL BY A BASEPLATE OR OTHER FIXTURE. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE THE FIXTURE PRIOR TO TESTING.
11. TEST 50% WEDGE OR SLEEVE ANCHORS PER ONE OF THE FOLLOWING METHODS:
- A. HYDRAULIC RAM METHOD: APPLY PROOF TEST LOAD WITHOUT REMOVING THE NUT. IF IT IS NOT POSSIBLE TO TEST WITH THE NUT INSTALLED, REPLACE THE NUT WITH A THREADED COUPLER TO THE SAME TORQUE MEASURED WITH A TORQUE WRENCH, AND THEN APPLY THE LOAD. ANCHOR IS ACCEPTABLE IF NO MOVEMENT IS OBSERVED AT THE TEST LOAD. MOVEMENT MAY BE DETERMINED WHEN THE WASHER UNDER THE NUT BECOMES LOOSE.
- B. TORQUE WRENCH METHOD: TEST ANCHORS TO THE TORQUE LOAD INDICATED IN THE TABLE BELOW WITHIN THE FOLLOWING LIMITS:
1. FOR 3/8" SLEEVE ANCHORS, ONE-QUARTER TURN OF THE NUT. FOR OTHER SLEEVE ANCHORS, ONE-HALF TURN OF THE NUT.
2. FOR WEDGE ANCHORS, ONE-HALF TURN OF THE NUT.

IX. ADHESIVE ANCHORS AND DOWELS

1. ANCHORS AND DOWELS INSTALLED INTO CONCRETE: HIT RE-500 SD BY HILTI (ICC ESR-2322), SET-XP BY SIMPSON STRONG-TIE (ICC ESR-2508), POWERS PE1000-BY POWERS FASTENERS (ICC ESR-2583). EMBEDMENT DEPTH FOR ANCHORS AND DOWELS IS AS FOLLOWS, UNLESS OTHERWISE NOTED. THE TESTING LABORATORY WILL PERFORM TENSION TESTS ON 25% OF ANCHORS AND DOWELS TO THE FOLLOWING TEST LOADS:

ROD DIA OR BAR SIZE	EMBEDMENT	TEST LOAD	BASE MATERIAL
3/8"	4"	18000#	CONCRETE
1/2"	5"	22000#	CONCRETE
5/8"	6"	50000#	CONCRETE
3/4"	7"	71000#	CONCRETE
7/8"	9"	97000#	CONCRETE
1"	11"	128000#	CONCRETE

#3	5"	30000#	CONCRETE
#4	6-1/2"	54000#	CONCRETE
#5	8"	84000#	CONCRETE
#6	10"	119000#	CONCRETE
#7	12"	169000#	CONCRETE
#8	14"	213000#	CONCRETE

2. ANCHORS: ASTM A36 THREADED RODS WITH ASTM A 563 GRADE A NUTS AND ANSI B 18.22.1 TYPE A WASHERS, UNLESS OTHERWISE NOTED. ANCHORS DESIGNATED AS ASTM A193 GRADE B7 THREADED RODS TO USE ASTM A 563 GRADE D8 HEAVY HEX NUTS AND ASTM F 436 WASHERS.
3. DOWELS: ASTM A615 GRADE 60 REINFORCING STEEL.
4. REMOVE GREASE, OIL, RUST, AND OTHER LAITANCE FROM RODS AND DOWELS PRIOR TO INSTALLATION.
5. REPLACE ANCHORS AND DOWELS THAT FAIL DURING TESTING AND RETEST. IF MORE THAN 10% OF THE TESTED DOWELS AND ANCHORS FAIL TO ACHIEVE THE SPECIFIED TEST LOAD, TEST 100% OF THE DOWELS AND ANCHORS INSTALLED IN THE LAST 2 DAYS OF ANCHOR INSTALLATION.
6. THE DIAMETER OF THE HOLES IS PER THE MANUFACTURER'S INSTRUCTIONS. DRILL HOLES FOR ANCHORS AND DOWELS IN UNREINFORCED BRICK MASONRY WITH A NON-IMPACT ROTARY DRILL. PRIOR TO INSTALLING ANCHORS OR DOWELS, WIRE BRUSH HOLES TO REMOVE RESIDUE, BLOW OUT WITH OIL-FREE COMPRESSED AIR, AND ALLOW HOLE TO DRY.
7. PLACE ADHESIVE WITH THE MANUFACTURER'S RECOMMENDED APPLICATION TOOL TO A DEPTH AS SPECIFIED BY THE MANUFACTURER AND TO MINIMIZE THE AMOUNT OF ADHESIVE THAT WILL OVERFLOW OUT OF THE HOLE WHEN THE BAR IS INSERTED. REMOVE EXCESS ADHESIVE ON THE ADJACENT SURFACES.
8. INSERT THE ANCHOR OR DOWEL IN THE HOLE WITH A TWISTING MOTION TO THE REQUIRED EMBEDMENT DEPTH. DO NOT PUMP THE ANCHOR OR DOWEL IN AND OUT OF THE HOLE.
9. WEDGE BARS TIGHT AND CENTERED IN THE HOLE WITH WOODEN WEDGES (GOLF TEES) TO HOLD IT IN PLACE UNTIL THE ADHESIVE SETS.
10. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.

11. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH ADHESIVE ANCHORS.
- X. STRUCTURAL TESTS, INSPECTIONS, AND OBSERVATIONS
1. AN INDEPENDENT TESTING AGENCY AND SPECIAL INSPECTORS WILL BE RETAINED AND PAID FOR BY THE CONTRACTOR TO PERFORM THE FOLLOWING TESTS AND INSPECTIONS AND PROVIDE ACCESS AND FURNISH SAMPLES TO THE AGENCY AS REQUIRED BY THE CONTRACT DOCUMENTS.
2. IF INITIAL TESTS OR INSPECTIONS MADE BY THE CONTRACTOR'S TESTING AGENCY REVEAL THAT ANY PORTION OF THE STRUCTURE DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS WILL BE MADE AT THE CONTRACTOR'S EXPENSE.
3. THE FOLLOWING ITEMS REQUIRE TESTS AND INSPECTIONS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CHAPTER "STRUCTURAL TESTS AND INSPECTIONS" OF THE CODE OF THE GOVERNING JURISDICTION AS NOTED IN THE GENERAL SECTION OF THESE GENERAL NOTES. ADDITIONAL ITEMS AND REQUIREMENTS FOR TESTS AND INSPECTIONS ARE IDENTIFIED IN THE SPECIFICATIONS.

STRUCTURAL STEEL			
✓	REVIEW MILL CERTIFICATE & TEST REPORTS		
✓	REVIEW WELDING PROCEDURE SPECIFICATION & WELDER CERTIFICATION		
✓	SAMPLE & TEST SECTIONS	✓	AS SPECIFIED
✓	SHOP MATERIAL IDENTIFICATION	✓	AS REQUIRED
✓	FIELD ERECTION INSPECTION		
✓	FABRICATION INSPECTION	✓	SHOP
✓	WELDING INSPECTION	✓	FIELD
✓	NON-DESTRUCTIVE WELD TEST	✓	SHOP
✓	BOLTING INSPECTION	✓	FIELD
✓	COMPOSITE STUD INSPECTION & TESTING		
✓	STEEL JOIST LOAD TEST		
METAL DECK			
✓	REVIEW MILL CERTIFICATES & TEST REPORTS		
✓	PLACEMENT INSPECTION		
✓	WELDING & FASTENING INSPECTION		
REINFORCING STEEL			
✓	REVIEW MILL CERTIFICATES & TEST REPORTS		
✓	SAMPLE & TEST	✓	REINFORCING BARS
✓	PLACEMENT INSPECTION		
✓	WELDING INSPECTION		
✓	TEST EXISTING REINFORCING FOR WELDABILITY		
STRUCTURAL LUMBER			
✓	REVIEW CERTIFICATES & TEST REPORTS		
✓	SAMPLE & TEST TIMBER CONNECTORS		
✓	FABRICATION	✓	GLU-LAM
✓	INSPECTION		
✓	FIELD ERECTION INSPECTION		
✓	TRUSSES		
✓	OPEN WEB JOIST		
EARTHWORK/FOUNDATION - FOR REQUIREMENTS, SEE GEOTECHNICAL REPORT AND SPECIFICATION SECTION 01 45 29			
✓	PREPWORK - FOR REQUIREMENTS, SEE SPECIFICATION SECTION 01 45 29		
MISCELLANEOUS			
✓	DRILLED-IN CONCRETE ANCHORS		
✓	EPOXY OR GROUTED DOWELS		
✓	BOLTS CAST IN CONCRETE		
✓	GROUTING UNDER BASE PLATES		
✓	COLD FORM METAL FRAMING WELDING		
CONCRETE & GROUT			
✓	CONCRETE		
✓	GROUT		
✓	AGGREGATE TESTS		
✓	CEMENT TEST		
✓	MIX DESIGN REVIEW		
✓	CONTINUOUS BATCH PLANT INSPECTION		
✓	BATCH PLANT INSPECTIONS		
✓	CAST, PICK-UP, AND COMPRESSION TEST SAMPLES		
✓	SUMP, ENTRAINED AIR, & TEMPERATURE TEST		
✓	UNIT WEIGHT TEST		
✓	DRY:		
	145 LBS/FT³ (NORMAL-WEIGHT, CLASS A,B,&C)		
	120 LBS/FT³ (LIGHT-WEIGHT, CLASS D)		
	SEE GEN NOTES SEC V.5 FOR CLASS REFERENCES		
✓	SHRINKAGE TEST		
✓	PLACEMENT INSPECTION		
✓	CORE & TEST		

4. IN ADDITION TO THE REQUIREMENTS NOTED IN THE TABLE ABOVE, PERFORM TESTING AND INSPECTIONS AS REQUIRED IN SPECIFICATION SECTION 01 45 29.
5. NOTIFY THE ENGINEER AT SIGNIFICANT CONSTRUCTION STAGES 72 HOURS IN ADVANCE AND PROVIDE ACCESS FOR THE FOLLOWING STRUCTURAL OBSERVATIONS:

A. FOUNDATIONS

1. REINFORCEMENT

B. STEEL FRAMING

1. GENERAL
2. MOMENT FRAMES
3. BRACED FRAMES
4. METAL DECKING AND SHEAR CONNECTORS

C. WOOD FRAMING

1. GENERAL
2. SHEARWALLS AND HOLD-DOWNS
3. DIAPHRAGMS AND COLLECTORS

D. CONCRETE

1. WALL REINFORCEMENT AND BOUNDARY ELEMENTS
2. FRAME REINFORCEMENT
3. POST-TENSIONING REINFORCEMENT
4. SLABS AND SLABS-ON-GRADE

E. MASONRY

1. ALL REINFORCEMENT AND BOUNDARY ELEMENTS
2. MEMBER CONNECTIONS TO MASONRY

XI. NONSTRUCTURAL UPGRADES

PER NON-STRUCTURAL REQUIREMENTS OF ASCE 31-03, CONTRACTOR TO PROVIDE NON-STRUCTURAL ANCHORAGE/BRACING, INCLUDING THE FOLLOWING ITEMS:

1. ALL LIGHTS, INCLUDING EMERGENCY LIGHTING, SHALL BE ANCHORED OR BRACED TO PREVENT FALLING DURING AN EARTHQUAKE.
2. LIGHT FIXTURES IN SUSPENDED GRID CEILINGS SHALL BE SUPPORTED INDEPENDENTLY OF THE CEILING SUSPENSION SYSTEM BY A MINIMUM OF TWO WIRES AT DIAGONALLY OPPOSITE CORNERS OF THE FIXTURES.
3. CANOPIES LOCATED AT BUILDING EXITS SHALL BE ANCHORED AT A SPACING OF 6 FEET OR LESS. CONTRACTOR TO CHECK (E) CANOPY ANCHORAGE CONDITIONS.
4. CABINETS, SHELVING UNITS, AND OTHER SLENDER CONTENTS SHALL BE ANCHORED TO THE FLOOR SLAB OR ADJACENT STRUCTURAL WALLS.
5. THE EMERGENCY POWER SYSTEM SHALL BE MOUNTED SUCH THAT IT IS OPERATIONAL AFTER AN EARTHQUAKE.
6. ALL SYSTEMS USING OR CONTAINING HAZARDOUS MATERIALS SHALL BE BRACED SUCH THAT THE HAZARDOUS WASTE IS NOT RELEASED DUE TO AN EARTHQUAKE. SUPPLY LINES SHOULD BE PROTECTED TO PREVENT DAMAGE IN AN EARTHQUAKE.
7. EQUIPMENT WEIGHING OVER 20 LB. THAT IS ATTACHED TO CEILINGS, WALLS, OR OTHER SUPPORTS 4 FT. ABOVE THE FLOOR LEVEL SHALL BE BRACED.
8. FIRE SUPPRESSION PIPING SHALL BE ANCHORED AND BRACED IN ACCORDANCE WITH NFPA-13. FLUID, GAS, AND FIRE SUPPRESSION PIPING SHALL USE FLEXIBLE COUPLINGS.
9. LAY-IN TILES USED IN CEILING PANELS LOCATED AT EXITS AND CORRIDORS SHALL BE SECURED WITH CLIPS.
10. INTEGRATED SUSPENDED CEILINGS AT EXITS AND CORRIDORS SHALL BE BRACED AT A MAXIMUM OF EVERY 12 FT.
11. PARTIAL-HEIGHT PARTITIONS SHALL BE BRACED AT LESS THAN OR EQUAL TO 6 FT.
12. UNREINFORCED MASONRY OR HOLLOW CLAY TILE PARTITIONS SHALL BE BRACED AT A SPACING OF 6 FEET MAXIMUM, AS SHOWN IN SHEET S5331
13. GLAZING REQUIREMENTS:

- A. FOR GLAZING PANES MORE THAN 16 SQ. FT. IN AREA, LESS THAN OR EQUAL TO 10 FEET ABOVE THE GROUND, REPLACE WITH SAFETY GLAZING.
- B. FOR GLAZING PANES MORE THAN 16 SQ. FT. IN AREA, MORE THAN 10 FEET ABOVE THE GROUND, REPLACE WITH LAMINATED SAFETY GLASS.

XII. DESIGN CRITERIA

1. APPLICABLE CODE: 2009 INTERNATIONAL BUILDING CODE WITH DEPARTMENT OF VETERANS AFFAIRS H-18-8 AMENDMENTS (JULY 2010).
2. FOUNDATIONS HAVE BEEN DESIGNED WITH THE FOLLOWING CRITERIA:

- ALL FOOTINGS: ALLOWABLE NET SOIL PRESSURE FOR DL + LL = 3,000 PSF
ALLOWABLE NET SOIL PRESSURE FOR DL + LL + EQ = 4,40