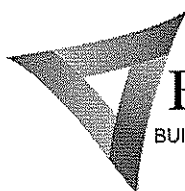


4/20/2016

# *Geotechnical Report*

*The U.S. Department of Veterans Affairs in  
Buhl, Idaho*

Prepared By: David Thibault, P.E.  
Sara Kaster, Engineer Technician



**EHM Engineers, Inc.**  
BUILDING THE FUTURE ON A FOUNDATION OF EXCELLENCE

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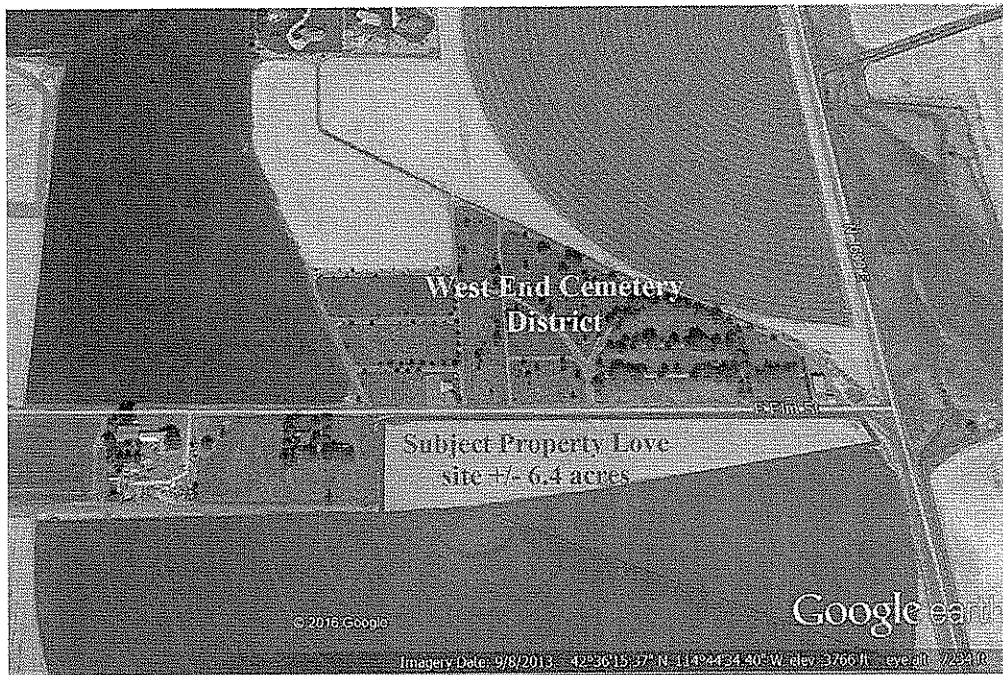
## Introduction

EHM Engineers, Inc. has performed a geotechnical investigation on the subject property located south of E. Elm Street and west of N. 1600 East, for the U.S. Department of Veterans Affairs. Refer to Figure 1 for the site vicinity map and Figure 2 for the location of the subject property in regards to the City of Buhl. The property consists of approximately six acres with access primarily from E. Elm Street. The site has historically been used for agricultural related activities.

EHM Engineers, Inc. understands that the proposed construction will consist of the establishment of pre-placed crypt gravesites, columbarium wall structures, US and POW flagpoles, and a memorial monument area.

This report will include the following:

- 1) Description of the existing subsurface materials and conditions.
- 2) Recommendations for:
  - a. Foundation design
  - b. Structural fill
  - c. Sub-grade preparation beneath slabs on grade and pavements
  - d. Cut slope limitations and excavations



**Figure 1: Site Vicinity Map**



**Figure 2: Subject property in regards to the City of Buhl**

## Site Description

This site is located directly south of the West End Cemetery District in Buhl, Idaho. Figure 1 shows the site locations.

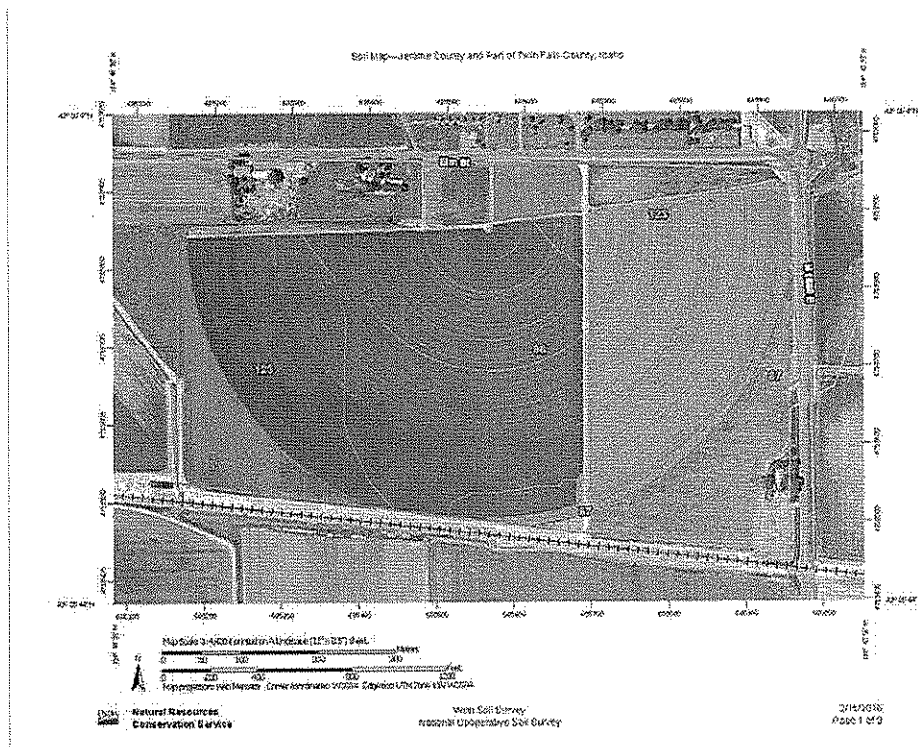
Nine test pits were dug and soil samples were taken at depths of three (3') feet and when appropriate at five to six and half feet (5'-6.5'). The test holes were generally located throughout the entire subject property, varying north and south to get diverse samples. No buildings or permanent structures have been constructed on the site. There were two underground pipes on the west side of the subject property that limited where we dug the test holes.

The subject property is related to agricultural activities in a rural setting east of the City of Buhl, Idaho. The sites naturally drain due to the Portneuf Silt Loam and Sluka Silt Loam that is present, refer to Figure 3. This site contains an average of 2.72% of organic matter, with high contents of nitrates and potassium; refer to Table 1 for a summary of soil fertility. Sample A was taken by test hole #1 and Sample B was taken by test hole #6. The pH of soil is approximately 8.15 on an average. There are no seep tiles or tunnels present and mild sloping of 0-2% and 1-4% respectively across the site, meaning the majority of storm water is absorbed on site. Depth to groundwater is described to be more than 80 inches below the ground surface (National Resource Conservation Service). No groundwater was encountered as part of the investigation.

**Table 1: Soil Fertility Summary**

	Sample A (near TH1)	Sample B (near TH6)	Range	Interpretation
Organic Content	2.89%	2.54%	N/A	Medium
Nitrates-ppm	37	28	10 – 35 ppm	High / Medium
Ammonium-ppm	4	4	+ 5 ppm	Low
Phosphorus-ppm	35	40	25 – 40 ppm	Optimum
Potassium-ppm	611	420	300 + ppm	High / Optimum
pH	8.2	8.1	N/A	Moderately Basic

Refer to Appendix C for full results of soil fertility, nutrients, and recommendations.



**Figure 3: Soil Map of the Subject Property**

## Area Geology

The City of Buhl, Idaho is at the center of the Great Rift, a 635-square mile geologic phenomenon, a series of fissures, spatter cones and lava tubes created by 60 different lava flows and over 25 volcanic eruptions. These geologic events helped create the local and adjacent features Rock Creek Canyon and Snake River Canyon. There are two predominant types of volcanic rocks that make up the Snake River Canyon near the project site: Rhyolite is from the explosive volcanic eruptions, dating back to when Magic Valley was located over the Yellowstone volcano, and basalt from the slower-moving lava. The rhyolite is 8-10 million years old and the basalt is less than 1 million years old. The canyon formed prior to the Bonneville Flood, 14,500 years ago. The canyon was much shallower and only became deepened with the high flows and extreme velocities of the water due to the flood.

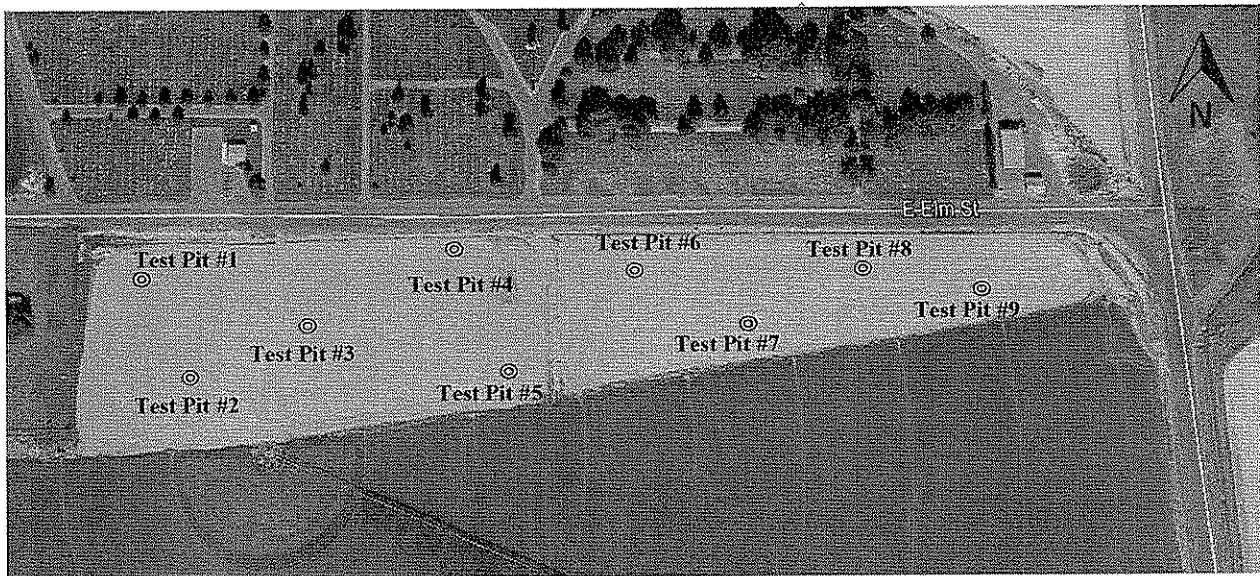
The subject property is located in the central portion of the Snake River Plain of Southern Idaho. The Snake River Plain is underlain by thousands of feet of volcanic deposits interbedded with relatively thin layers of sediment. The basalt layers are overlain in the Buhl area with a thin layer of loess/eolian (windblown) deposits of silt and sand (Eolian deposits over bedrock derived from volcanic rock). These superficial sediments generally vary between 3-15 feet in depth within the subject property. Topographic conditions appeared to range from 0% to 4% with increasing grades to the southwest.

## Site Investigation

Nine (9) test pits were excavated and soil samples were completed for each test hole at 3.0' and when appropriate 5'-6.5' on April 6, 2016. The approximate locations of the test holes as specified by EHM Engineers, Inc. and approved by Mr. Nathan Van Arsdale with Public Properties, are shown in Figure 4 and fully described in Table 2.

The soils were collected by EHM Engineers, Inc. and analyzed at their materials testing laboratory. The soils were laboratory tested and classified according to the Unified Soil Classification System (ASTM D-2487). The site investigation results are described in Table 2 and the laboratory test results are included in Appendix C.

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**Figure 4: Test Hole Locations**



**Table 2: Site Investigation Results**

Test Pit	Ground Elevation	Depth to Rock	Sample Depth	USCS	Soil Description
#1	±3793	7.0'	3.0'	ML	Sandy silt, light brown, dry, moderately dense, stiff
			5.0'-6.5'	Rock	Asphalt & Sand (Fill)-Vesicular basalt – black, very dense with caliche silt layers
#2	±3797	6.0'	3.0'	ML	Silt with sand, light brown, dry, moderately dense, stiff with thin caliche layers
#3	±3797	NA	3.0'	ML	Sandy silt, light brown, dry, moderately dense, stiff with caliche layer @ 4.0'
			10.0'-11.5'	SM	Silty sand, light brown, dense with caliche layer @ 4.0'
#4	±3796	4.0'	3.0'	SP	Poorly graded sand, light brown, dry, dense with caliche layer @ 4.0'
#5	±3798	4.0'	3.0'	SP	Poorly graded sand, light brown, dry, dense with caliche layer @ 4.0'
#6	±3796	5.0'	3.0'	SP	Poorly graded sand, light brown, dry, dense with caliche layer @ 4.0'
#7	±3795	7.0'	3.0'	SP	Poorly graded sand, light brown, dry, dense with caliche layer @ 4.0'
#8	±3789	5.0'	3.0'	SM	Silty sand with gravel, light brown, dry, dense with caliche layer @ 5.5'
#9	±3786	8.5'	3.0'	SP	Poorly graded sand, light brown, dry, dense
			5.0'-6.5'	SP	Poorly graded sand, light brown, dry, dense

## Conclusions and Recommendations

### Foundation Design

The ML and SP soils located at the site are suitable for construction of spread footing foundations with adherence to the following provisions:

- 1) All organic top soils must be removed from the footing areas. Soils containing vegetation, organic matter, debris, wastes, or frozen materials are not suitable for use as structural fills or beneath footings.
- 2) Any pavements or gravels encountered should be removed from footing areas which are in conflict.
- 3) The bottom of the footing shall be located a minimum of 30-inches below the finished grade for frost protection.
- 4) Silt soils have a moderate to high collapse potential when they become saturated under load. Therefore, site grading must be designed to direct water away from the building(s) in all directions. If this cannot be accomplished, a perimeter drainage system at or above the foundation elevation should be installed. Roof drainage systems must also divert water away from the building(s) and not discharge onto the ground near exterior walls.
- 5) For an allowable bearing pressure of 2,000 psf, it is recommended that the site be proof rolled and tested to ensure 98% compaction prior to placement of footings.

The foundation shall be cast against undisturbed and/or properly compacted soils that have been approved by a geotechnical engineer. The estimated total settlement is not expected to be greater than one-half inch. The estimated differential settlement is not expected to be greater than one-half inch.

Groundwater was not encountered during any site investigations.

### Seismic Design

The upper-most 100-feet of strata at the project site is predominantly basalt rock ( $N > 50$ ). Therefore, the site may be classified as Site Class C for seismic design. (International Code Council, Inc., 2009)

### Lateral Earth Pressure

For the ML and SP soils native to the site, the following lateral earth pressures may be used assuming a typical angle of shearing resistance ( $\phi$ ) of  $33^\circ$  and unit weight of 105 pcf:

At Rest: 47.81 psf  
Active: 30.95 psf  
Passive: 356.17 psf

(Geotechdata.info, 2011).

## **Structural Fill**

The ML and SP silts are classified as fine and course grained soils respectively. The general engineering characteristics include (Yun Zhou, 2006):

- Generally very good foundation material for supporting structures and roads.
- Generally very good embankment materials
- Generally the best backfill material for retaining walls.
- Might settle under vibratory loads or blasts
- Dewatering may be difficult in open-graded gravels due to high permeability.
- Generally not frost susceptible.

To achieve an allowable bearing pressure of 2,000 psf, it is recommended that the site be compacted, proof rolled, and tested to ensure 98% compaction prior to placement of footings. Test results shall be approved by a geotechnical engineer prior to footing placement.

Should additional structural fill material be necessary, the fill material shall have at least 90% passing a 1½ inch (38mm) sieve and no more than 8 percent passing a No. 200 (0.075mm) sieve. The material shall be uniformly graded and shall be uniform in consistency. These materials shall be free of rock or gravel larger than 3 inches (75mm) in any dimension, debris waste, frozen materials, vegetation, and/or other deleterious material.

Satisfactory soils (ASTM D 2487) are GW, GP, GM, SW, SP and SM, or a combination of these group symbols. These materials shall be free of rock or gravel larger than 3 inches (75mm) in any dimension, debris waste, frozen materials, vegetation, and/or other deleterious material.

Any imported fill placed on site shall be placed in 8 inch maximum lifts and compacted to a minimum of 95 percent of maximum density as determined by ASTM D 698 at optimum moisture.

## **Slabs on Grade**

All organic topsoil must be removed to a depth of 6"-12" from all areas in which slabs are to be placed. Slabs on grade should be placed over a minimum of 6-inches of compacted granular structural fill (native or imported structural fill as described above) compacted to 98% of maximum density as determined by ASTM D 698 at optimum moisture. Reinforced concrete, designed by a professional engineer, may then be constructed above the ballast.

## **Pavement Sections**

It is recommended that all materials used in the construction of Asphaltic Concrete Pavements meet the requirements of the State of Idaho Department of Transportation Standard Specification for Highway Construction.

The sub-grade upon which any pavement sections are to be constructed should be properly cleared and stripped to a minimum of 12 inches of depth and then compacted to 95% of maximum density as determined by ASTM D 698 at optimum moisture.

To provide for standard traffic loadings, it is recommended that a minimum of 8 inches of pit run, 2 inches of crushed  $\frac{3}{4}$ " gravel, and 2.5 inches of asphalt plant mix be provided.

To provide for heavy duty traffic loadings, it is recommended that a minimum of 12 inches of pit run, 3 inches of crushed  $\frac{3}{4}$ " gravel, and 3 inches of asphalt plant mix be provided.

The road base gravel should conform to the following gradation:

<u>Sieve Size</u>	<u>% Passing</u>
$\frac{3}{4}$ "	100
$\frac{1}{2}$ "	90-100
#4	45-70
#8	30-55
#200	2-8

### **Site Grading**

Site grading should be designed to direct surface run-off away from buildings, other structures, and roadways.

Cut slopes should be excavated at 3:1 slopes, horizontal to vertical. Final cut slopes should be 3:1 or flatter.

### **Storm Water Retention**

The City of Buhl requires that every new development capture, retain, and release their storm water within the boundaries of the subject property. This can be accomplished through several different methods including but not limited to swales, ponds, and dry wells. For this site, it is recommended that a combination of swales and basin be used.

The City of Buhl uses a storm water intensity of 1.6 inches for the 25 year – 24 hour storm event. Based on the intensity and below information, the storm water retention basin can be appropriately sized.

**Table 3: Recommended Runoff Coefficients**

Description of Runoff Area	Runoff Coefficients "C"
<b>Business</b>	
Central business areas	0.70-0.95
District and local areas	0.50-0.70
<b>Residential</b>	
Single-family	0.35-0.45
Multi-family, detached	0.40-0.60
Multi-family, attached	0.60-0.75
Residential 0.5 acre lots of larger	0.25-0.40
<b>Industrial and Commercial</b>	
Light areas	0.50-0.80
Heavy areas	0.75-0.95
<b>Parks, Cemeteries</b>	0.10-0.25
<b>Playgrounds</b>	0.20-0.35
<b>Unimproved Areas</b>	0.10-0.30
<b>Landscaped Areas</b>	0.20
<b>Streets (Asphalt, Concrete), Drives and Walks, Roofs</b>	0.90-0.95

(IDeq, Water Quality Division, September 2005)

**Table 4: Typical Soil Permeability**

Group Symbol	Coefficient of Permeability, K (cm/s)
GW	$2.5 \times 10^{-2}$
GP	$5 \times 10^{-2}$
GM	$>5 \times 10^{-7}$
GC	$>5 \times 10^{-8}$
SW	$>5 \times 10^{-4}$
SP	$>5 \times 10^{-4}$
SM	$>2.5 \times 10^{-5}$
SM-SC	$>10^{-6}$
SC	$>2.5 \times 10^{-7}$
ML	$>5 \times 10^{-6}$
ML-CL	$>2.5 \times 10^{-7}$
CL	$>5 \times 10^{-8}$
OL	---
MH	$>2.5 \times 10^{-7}$
CH	$>5 \times 10^{-8}$
OH	---

(Michael R. Lindeburg, 2011)

## Notes

The recommendations contained in this report are based upon EHM Engineers, Inc. understanding of the proposed development of the site and its evaluation of the conditions observed in the test holes. Soils conditions may vary between test holes. Variations will not appear until construction, and may require changes in the design and construction of the proposed improvements and/or developments. If any variations or undesirable conditions are encountered during construction, or if the proposed construction differs from conventional practices, a soils engineer should be notified to describe necessary supplemental recommendations.

Any fill placed onsite of different origin must be evaluated by a soils engineer. Fills placed without compaction records, or fills containing construction debris, demolition wastes, organic materials, etc. must be over-excavated and replaced with a properly compacted structural fill. Potentially hazardous material within a discovered fill is beyond the scope of this report.

This report is issued with the understanding that it is the responsibility of the owner or the representatives of the owner to ensure that the information and recommendations contained herein are called to the attention of all project Architects and Engineers, incorporated into the plans, and that the necessary steps are taken to assure that all Contractors and Sub-Contractors carry out such recommendations in the field.

This report has been prepared in accordance with generally accepted soils and foundation practices. No other warranty either expressed or implied as to professional advice provided under the terms of this agreement and included in this report is made.

Soils in the test holes are in a loose condition and are prone to settlement. Test holes located under any portion of the proposed structures, improvements, or developments should be re-excavated and backfilled with structural fill and properly compacted.

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## **Appendix A      Photographs of Adjacent Properties**

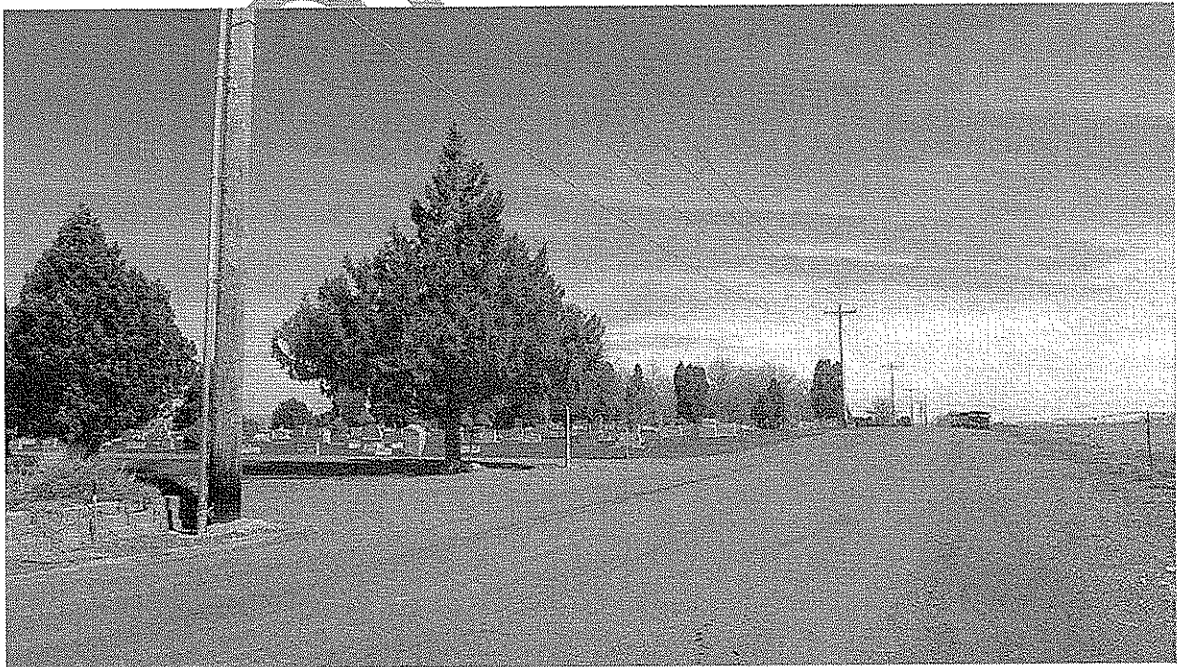
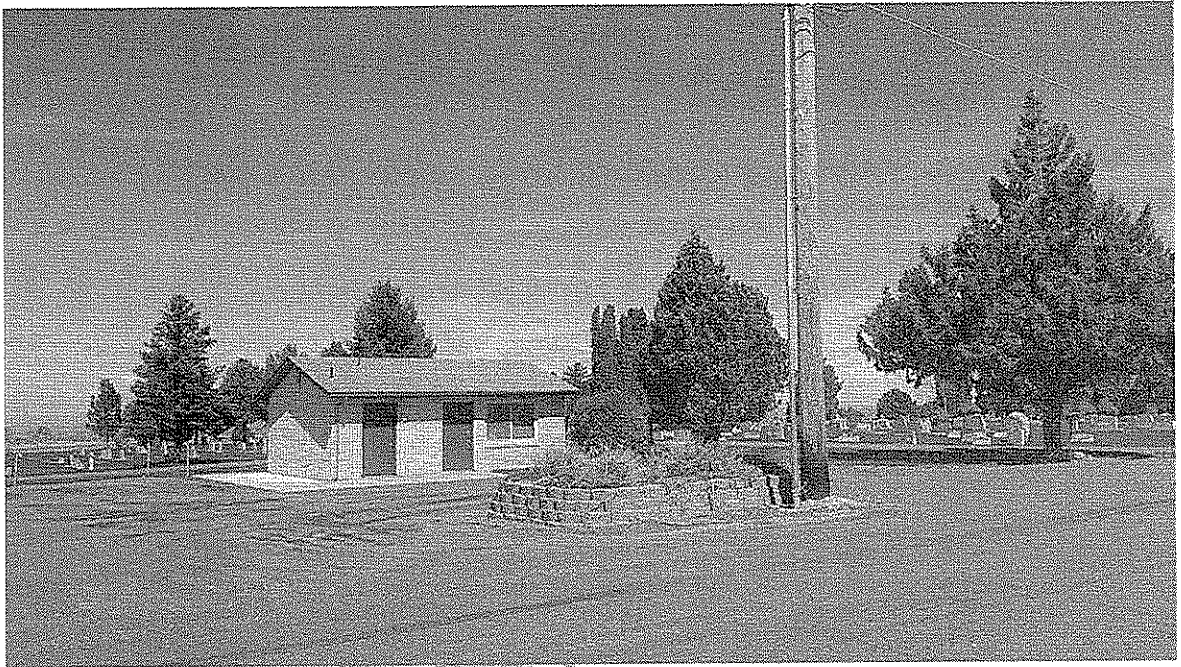
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## Appendix B      Test Pit Logs

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## Geotechnical Test Pit Log

### Test Pit No. 1

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** Northwest Corner of Site

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, brown, dry, Roots to 6.0'–12.0'	None	None
1.0' – 5.0'	(ML) Sandy silt, light brown, dry, moderately dense	None	Yes at 3'
5.0' – 6.5'	Broken up rock with calcified silt	18/20/11	Yes
6.5' – 8.0'	Hit rock at 7' Bottom of hole at 8'	None	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No.2

**Project:** Shirley Love's Property -- Buhl, Idaho

**Location:** Southwest corner of Site

Depth	Description	No. of Blow Counts	Sampled
0.0' - 1.0'	Top soil, brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' - 5.0'	(ML) Silt with sand, calcified gravel, caliche layers, light brown, dry, moderately dense	None	Yes at 3'
5.0' - 6.5'	Restrictive layer at 6.5' Bottom of hole 6.5'	4/6/21	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No

## Geotechnical Test Pit Log

### Test Pit No.3

**Project:** Shirley Loye's Property -- Buhl, Idaho

**Location:** Slightly, northwest of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, brown, soft, Roots to 6.0"–12.0"	None	None
1.0' – 5.0'	(ML) Sandy silt, light brown, dry, moderately dense with caliche layer at 4.0'	None	Yes at 3'
5.0' – 6.5'	Broken up rock with calcified silt	5/5/4	None
6.5' – 10.0'	Hit rock at 7'	None	None
10.0' – 11.5'	Used spoon (SM) silty sand, light brow, dry, dense Bottom of hole at 11.5'	16/21/50 blow for 5"	Yes

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No





## Geotechnical Test Pit Log

### Test Pit No. 4

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** Centrally located on the northwest area of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' – 4.0'	(SP) Poorly graded sand, light brown, dry, dense with caliche layer at 4.0' Bottom of hole at 4.0'	None	Yes at 3'

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No.5

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** Centrally located on the southwest area of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, dark brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' – 5.0'	(SP) Poorly graded sand, light brown, dry, dense with caliche layer at 4.0'	None	Yes at 3'
5.0' – 5.5'	Restrictive layer at 5.5'	35 for 6.0" 30 for 0.25"	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No. 6

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** Centrally located on the northeast area of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, dark brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' – 5.0'	(SP) Poorly graded sand, light brown, dry, dense with caliche layer at 4.0'	None	Yes at 3'
5.0' – 5.5'	Restrictive layer at 5.5' Broken up rock with calcified silt Bottom of hole at 5.5'	50 blows for 5.5'	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No. 7

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** Southeast of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, dark brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' – 5.0'	(SP) Poorly graded sand, light brown, dry, dense with caliche layer at 4.0'	None	Yes at 3'
5.0' – 6.5'	(SP) Poorly graded sand, light brown, dry, dense with caliche layers	8/8/13	None
6.5' – 8.5'	Rock at 7' Total depth 8.5'	None	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No.8

**Project:** Shirley Love's Property - Buhl, Idaho

**Location:** Northeast area of the site property

Depth	Description	No. of Blow Counts	Sampled
0.0' - 1.0'	Top soil, dark brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' - 5.0'	(SM) Silty sand with gravel, light brown, dry, dense with caliche layer at 5.5'	None	Yes at 3'
5.0' - 5.5'	(SM) Silty sand with gravel, light brown, dry, dense with caliche layer Restrictive layer at 5.5'	27 for 6" 50 for 1"	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No



## Geotechnical Test Pit Log

### Test Pit No.9

**Project:** Shirley Love's Property – Buhl, Idaho

**Location:** East corner of site property

Depth	Description	No. of Blow Counts	Sampled
0.0' – 1.0'	Top soil, dark brown, soft, Roots to 6.0" - 12.0"	None	None
1.0' – 5.0'	(SP) Poorly graded sand, light brown, dry, dense	None	Yes at 3'
5.0' – 6.5'	(SP) Poorly graded sand, light brown, dry, dense	11/17/21	Yes
6.5' – 9.0'	(SP) Poorly graded sand, light brown, dry, dense Restrictive layer at 8.5' Bottom of hole 9'	None	None

Observed, logged and sampled by: J. Skeen & S. Kaster  
Excavation Equipment used: Hollow Stem Logger  
Samples taken at 3.0 feet  
Ground Water Encountered: No

## Appendix C      Sieve Analysis and Soil Classification

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## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 1,3 - 3'

Sample	Total Wt
Sample/Tare	480.7
Tare /Ratio	0
Sample Wt.	480.7

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

### Sieve Analysis

ASTM C136 -C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	0.0	0.0	100
3/4" / 19.0 MM	0.0	0.0	100
1/2" / 12.5 MM	0.0	0.0	100
3/8" / 9.5 MM	8.5	1.8	98
No. 4 / 4.75 MM	19.8	4.1	96
No. 8 / 2.36 MM	31.7	6.6	93
No. 16 / 1.18 MM	40.8	8.5	92
No. 30 / 0.600 MM	56.8	11.8	88
No. 50 / 0.300 MM	77.4	16.1	84
No. 100 / 0.150 MM	105.9	22.0	78
No. 200 / 0.075 MM	148.9	31.0	69

### Atterburg Limits

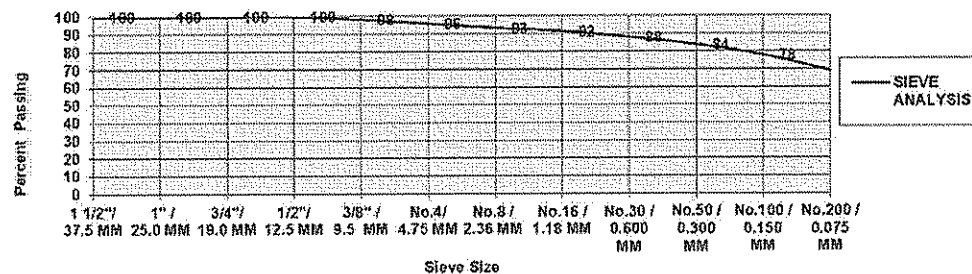
ASTM D4318

Liquid Limit: NP

Plastic Limit: NP

Plasticity Index: NP

Gradation Chart



### Classification (UCS)

ASTM D2487 Group Name:

Sandy Silt

Group Symbol:

ML

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.



## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 1 - 5'-6.5'

Sample	Total Wt
Sample/Tare	544.9
Tare /Ratio	0
Sample Wt.	544.9

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

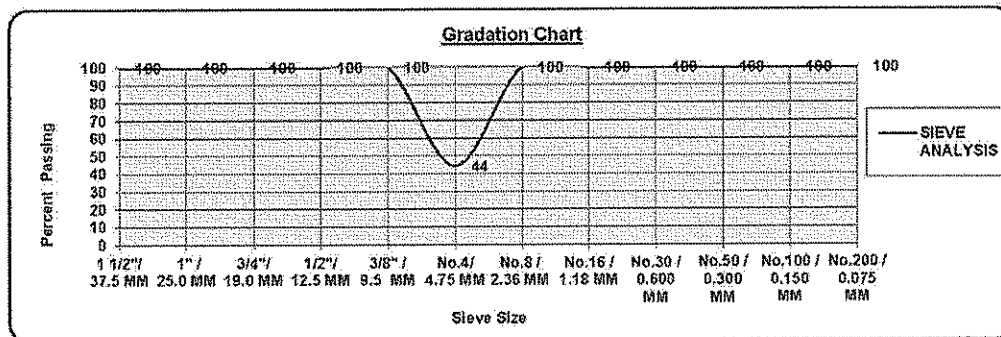
### Sieve Analysis

ASTM C136 -C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	0.0	0.0	100
3/4" / 19.0 MM	0.0	0.0	100
1/2" / 12.5 MM	0.0	0.0	100
3/8" / 9.5 MM	0.0	0.0	100
No. 4 / 4.75 MM	303.6	55.7	44
No. 8 / 2.36 MM	0.0	0.0	100
No. 16 / 1.18 MM	0.0	0.0	100
No. 30 / 0.600 MM	0.0	0.0	100
No. 50 / 0.300 MM	0.0	0.0	100
No. 100 / 0.150 MM	0.0	0.0	100
No. 200 / 0.075 MM	0.0	0.0	100

### Atterburg Limits

ASTM D4318

Liquid Limit: NP
Plastic Limit: NP
Plasticity Index: NP



### Classification (UCS)

ASTM D2487 Group Name:

**ASPHALT & SAND (FILL)**

Group Symbol: -

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.

## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 2 - 3'

Sample	Total Wt
Sample/Tare	473.3
Tare /Ratio	0
Sample Wt.	473.3

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

### Sieve Analysis

ASTM C136-C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	0.0	0.0	100
3/4" / 19.0 MM	0.0	0.0	100
1/2" / 12.5 MM	0.0	0.0	100
3/8" / 9.5 MM	0.0	0.0	100
No. 4 / 4.75 MM	0.0	0.0	100
No. 8 / 2.36 MM	5.0	1.1	99
No. 16 / 1.18 MM	11.9	2.5	97
No. 30 / 0.600 MM	19.4	4.1	96
No. 50 / 0.300 MM	30.5	6.4	94
No. 100 / 0.150 MM	45.3	9.6	90
No. 200 / 0.075 MM	79.5	16.8	83

### Atterburg Limits

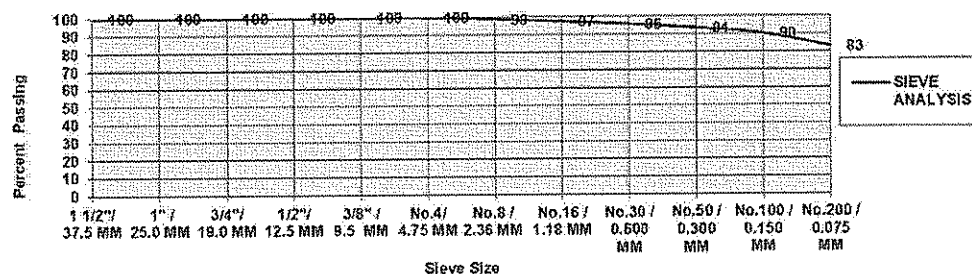
ASTM D4318

Liquid Limit: NP

Plastic Limit: NP

Plasticity Index: NP

Gradation Chart



### Classification (UCS)

ASTM D2487 Group Name:

Silt with Sand

Group Symbol:

ML

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.

## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 3 - 10' - 11.5'

Sample	Total Wt
Sample/Tare	460.4
Tare /Ratio	0
Sample Wt.	460.4

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

### Sieve Analysis

ASTM C136-C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	0.0	0.0	100
3/4" / 19.0 MM	0.0	0.0	100
1/2" / 12.5 MM	13.2	2.9	97
3/8" / 9.5 MM	14.6	3.2	97
No. 4 / 4.75 MM	22.5	4.9	95
No. 8 / 2.36 MM	35.4	7.7	92
No. 16 / 1.18 MM	72.3	15.7	84
No. 30 / 0.600 MM	170.7	37.1	63
No. 50 / 0.300 MM	262.3	57.0	43
No. 100 / 0.150 MM	324.4	70.5	30
No. 200 / 0.075 MM	365.1	79.3	21

### Atterburg Limits

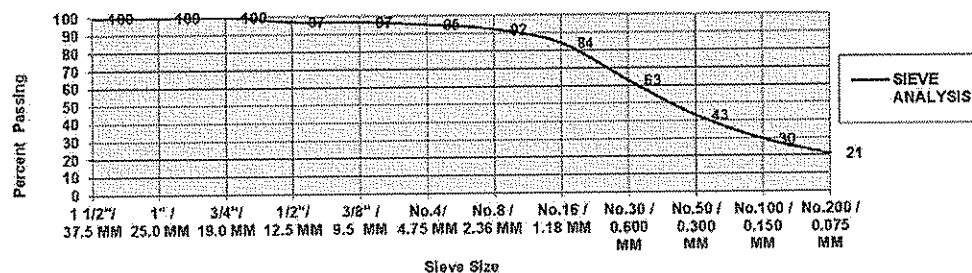
ASTM D4318

Liquid Limit: NP

Plastic Limit: NP

Plasticity Index: NP

Gradation Chart



### Classification (UCS)

ASTM D2487 Group Name:

Silty Sand

Group Symbol:

SM

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.

## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 4,5,6,7,9 - 3'

Sample	Total Wt
Sample/Tare	343.9
Tare /Ratio	0
Sample Wt.	343.9

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

### Sieve Analysis

ASTM C136 -C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	46.3	13.5	87
3/4" / 19.0 MM	51.8	15.1	85
1/2" / 12.5 MM	73.8	21.5	79
3/8" / 9.5 MM	103.9	30.2	70
No. 4 / 4.75 MM	132.5	38.5	61
No. 8 / 2.36 MM	152.2	44.3	56
No. 16 / 1.18 MM	173.9	50.6	49
No. 30 / 0.600 MM	217.3	63.2	37
No. 50 / 0.300 MM	257.3	74.8	25
No. 100 / 0.150 MM	294.8	85.7	14
No. 200 / 0.075 MM	331.7	96.5	4

### Atterburg Limits

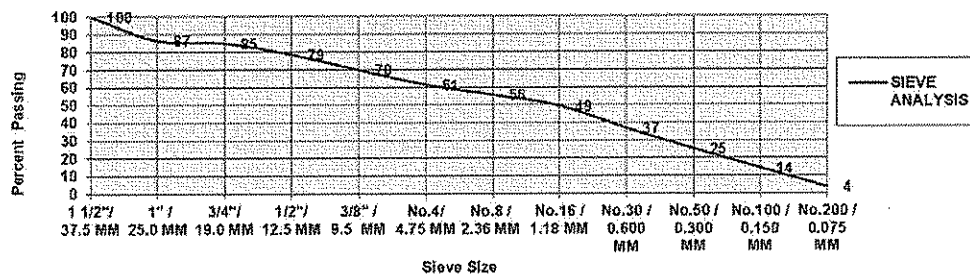
ASTM D4318

Liquid Limit: NP

Plastic Limit: NP

Plasticity Index: NP

**Gradation Chart**



### Classification (UCS)

ASTM D2487 Group Name:

Poorly Graded Sand

Group Symbol:

SP

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.

## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 8 - 3'

Sample	Total Wt
Sample/Tare	637.1
Tare /Ratio	0
Sample Wt.	637.1

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

### Sieve Analysis

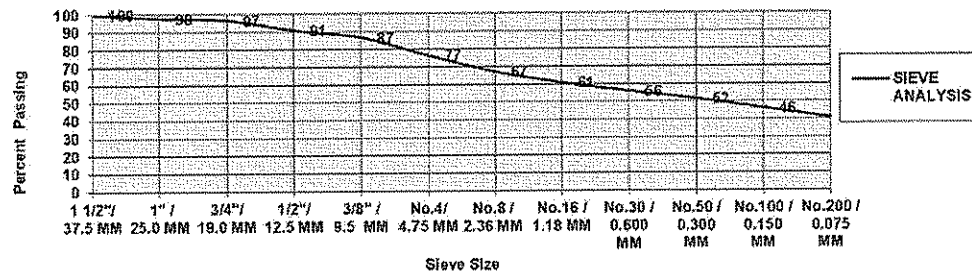
ASTM C136 -C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	14.2	2.2	98
3/4" / 19.0 MM	21.5	3.4	97
1/2" / 12.5 MM	57.9	9.1	91
3/8" / 9.5 MM	84.1	13.2	87
No. 4 / 4.75 MM	149.6	23.5	77
No. 8 / 2.36 MM	209.1	32.8	67
No. 16 / 1.18 MM	249.2	39.1	61
No. 30 / 0.600 MM	279.9	43.9	56
No. 50 / 0.300 MM	308.4	48.4	52
No. 100 / 0.150 MM	342.5	53.8	46
No. 200 / 0.075 MM	377.6	59.3	41

### Atterburg Limits

ASTM D4318

Liquid Limit: NP
Plastic Limit: NP
Plasticity Index: NP

**Gradation Chart**



### Classification (UCS)

ASTM D2487 Group Name:

**Silty Sand with Gravel**

Group Symbol:

**SM**

Reviewed By: \_\_\_\_\_

W.J. Nenno C.E.T.

## SIEVE ANALYSIS

ASTM C136 & C117

## SOIL CLASSIFICATION

ASTM D2487

Project: Twin Falls Idaho - VA

Sample Description:: Test Hole 9 - 5 - 6.5'

Sample	Total Wt
Sample/Tare	349.3
Tare /Ratio	0
Sample Wt.	349.3

Date Sampled: 04/06/16

EHM Job # 052-16

Sampled By: S. Kaster

Tested By: B. Kruger

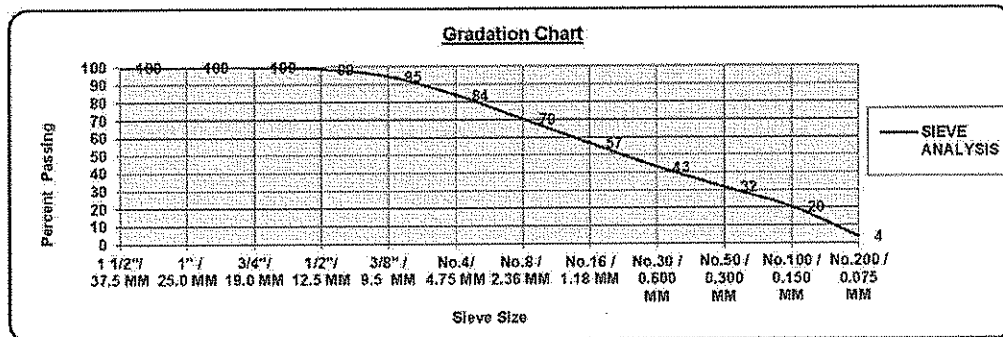
### Sieve Analysis

ASTM C136 -C117	Accum. Wt.	% Retained	% Passing
1 1/2" / 37.5 MM	0.0	0.0	100
1" / 25.0 MM	0.0	0.0	100
3/4" / 19.0 MM	0.0	0.0	100
1/2" / 12.5 MM	4.1	1.2	99
3/8" / 9.5 MM	18.8	5.4	95
No. 4 / 4.75 MM	55.3	15.8	84
No. 8 / 2.36 MM	103.5	29.6	70
No. 16 / 1.18 MM	151.4	43.3	57
No. 30 / 0.600 MM	198.4	56.8	43
No. 50 / 0.300 MM	238.6	68.3	32
No. 100 / 0.150 MM	278.1	79.6	20
No. 200 / 0.075 MM	336.8	96.4	4

### Atterburg Limits

ASTM D4318

Liquid Limit: NP
Plastic Limit: NP
Plasticity Index: NP



### Classification (UCS)

ASTM D2487 Group Name:

Poorly Graded Sand

Group Symbol:

SP

Reviewed By:

W.J. Nenno C.E.T.



**MAGIC VALLEY LABS**  
210 Addison Ave / PO Box 1867  
Twin Falls ID 83303-1867  
Phone: (208) 733-4250  
Fax: (208) 734-2539

**EHM ENGINEERS**

**621 N COLLEGE ROAD #100**  
**TWIN FALLS ID 83301**

Collection Date 4/12/2016		Received Date 4/12/2016	Location	
Collection Time 12:15 PM		Received Time 1:14 PM	052-16-TH1	
Sample #	Test / Method Code	Results in mg/L		Date Analyzed Analyst
1581651				

Signature

Report Date: Wednesday, April 20, 2016



AGRICULTURAL SOIL REPORT

Field ID: 1581651

31564

Split apply Nitrogen. Nitrogen, sulfur and boron recs are made for this year. All other nutrient recs can be split over a two-year program. Tissue and soil test in-season gives the best results.

Elemental Sulfur = Reclamation Sulfur

*"Always practice the laws of Agronomy"*  
John P. Taberna, Soil Scientist





MAGIC VALLEY LABS  
210 Addison Ave / PO Box 1867  
Twin Falls ID 83303-1867  
Phone: (208) 733-4250  
Fax: (208) 734-2539

EHM ENGINEERS

621 N COLLEGE ROAD #100  
TWIN FALLS ID 83301

Collection Date 4/12/2016		Received Date 4/12/2016		Location	
Collection Time 12:30 PM		Received Time 1:14 PM		052-16-TH6	
Sample #	Test / Method Code	Results in mg/L		Date Analyzed	Analyst
1581651					

Signature

Report Date: Wednesday, April 20, 2016



**EHM Engineers, Inc.**  
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# **Western Laboratories.com**

211 Highway 95 • P.O. Box 1020 • Parma, ID 83660  
800-658-3858 • FAX 208-722-8550  
<http://www.westernlaboratories.com>  
Methods: [www.westernlaboratories.com/methods](http://www.westernlaboratories.com/methods)



Dealer: 17-27 L Smith  
Reported: 4-15-2016  
Test #: 1  
Grower: Magic Valley Labs  
Field ID: 1581681

Lab #:
31555

## **AGRICULTURAL SOIL REPORT**

ELEMENT	ANSWER	INTERP	SHOULD BE	ELEMENT	ANSWER	INTERP	SHOULD BE
pH-Soil	8.1	Moderately Basic		Sulfur-ppm	32	Optimum	20 +
pH-SMP				Calcium-ppm	4538	High	1,800 +
Soluble Salts	0.47	Optimum	< 1.5	Magnesium-ppm	613	High	250 +
% Lime	M	3.1 to 5.5 % lime		Sodium-ppm	170	Optimum	< 225
% Organic Matter	2.54	Medium		Zinc-ppm	1.5	Optimum	1.0 - 3.0
Nitrates-ppm	28	Optimum	10 - 35	Copper-ppm	1.1	Optimum	0.8 - 2.5
Ammonium-ppm	4	Low	5 +	Manganese-ppm	4	Low	6 - 30
Phosphorus-ppm	40	Optimum	25 - 40	Iron-ppm	31	High	7 +
Phos-ppm-Bray			50 - 100	Boron-ppm	1.1	Optimum	0.7 - 1.5
Potassium-ppm	420	Optimum	300 +	TBS%		94	
Texture	Loam	Water Holding Capacity/foot	2.08	Bulk Density	1.4		
Cation Exchange Capacity - CEC	15	P Index		Fertilizer Suggestions in Pounds per Acre for the whole season			
Percent Base Saturation	194			Crop			
BASES	IDEAL	YOURS		Yield Goal			
Calcium-% of CEC	65-80	149	1 Ft	Past Crop			
Magnesium-% of CEC	10-20	33	2 Ft	Acres			
Potassium-% of CEC	2-6	7	3 Ft	Nitrogen			
Sodium-% of CEC (ESP)	< 5	5	Total N PPM	32	Phosphate		
Hydrogen-% of CEC	< 15		Lbs N / Acre	96			
Ratio	Ideal	Yours	Evaluation	Recommendations	Add Phos for P INDEX		
Ca:Mg	6-20:1	7:1	OK		Potash		
Ca:K pH > 7	15:1	11:1	OK		P.F. Sulfur		
Ca:K pH < 7	10:1	:1			Elemental Sulfur	294	
Ca:P pH > 7	100:1	113:1	High	Watch P	Gypsum		
Ca:P pH < 7	40:1	:1			Lime		
P:Zn	15:1	27:1	High	Watch Zn	Dolomite		
P:Mn	4:1	10:1	High	Watch Mn	Magnesium		
P:Cu	25:1	36:1	High	Watch Cu	Zinc		
Zn:Cu	3:1	1:1	OK		Manganese		
Mn:Zn	3:1	3:1	OK		Copper		
Mn:Cu	7:1	4:1	OK		Boron		
K:B	200:1	382:1	High	Watch B			
Mg:K	2:1	1:1	Low	Watch Mg			

Split apply Nitrogen. Nitrogen, sulfur and boron recs are made for this year. All other nutrient recs can be split over a two-year program. Tissue and soil test in-season gives the best results.

P.F. Sulfur = Plant Food Sulfur

Elemental Sulfur = Reclamation Sulfur

*"Always practice the laws of Agronomy"*  
John P. Taberna, Soil Scientist

## Lateral Earth Pressure Calculation U.S. Department of Veterans Affairs

Assumptions	ML/SP Soil Type
$K_0$	0.45
$\Phi$	33°
$\gamma_{\text{soil}}$	105 pcf

$$K_0 = 1 - \sin \Phi$$

$$K_a = (1 - \sin \Phi) / (1 + \sin \Phi)$$

$$K_p = (1 + \sin \Phi) / (1 - \sin \Phi)$$

### Vertical Principle Stress

$$\sigma_v = 105 \text{ pcf/ft}$$

### Horizontal Principle Stress

$$\sigma_h = \sigma_v \cdot K_0$$

### At Rest

$$P_0 = \sigma_h = \sigma_v \cdot K_0$$

$$P_0 = \sigma_h = 47.81 \text{ psf}$$

### Active

$$P_a = \sigma_v \cdot \tan^2 (45 - \Phi/2)$$

$$P_a = 30.95 \text{ psf}$$

### Passive

$$P_p = \sigma_v \cdot \tan^2 (45 + \Phi/2)$$

$$P_p = 356.17 \text{ psf}$$

## Appendix D      Records Search

DRAFT

2/18/2016

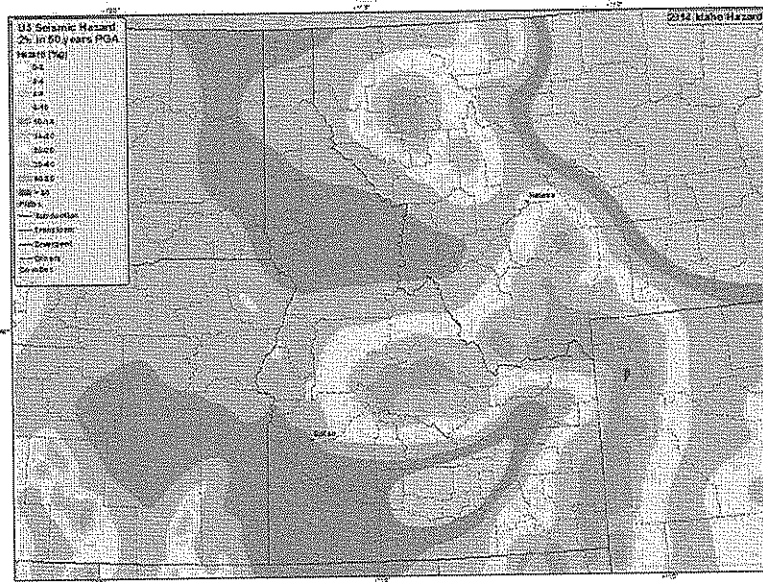
Idaho



**Earthquake Hazards Program**

**Idaho**

**2014 Seismic Hazard Map**



[USGS National Seismic Hazard Maps](#)

Share this page: [Facebook](#) [Twitter](#) [Google](#) [Email](#)

<http://earthquake.usgs.gov/earthquakes/states/idaho/hazards.php>

1/1

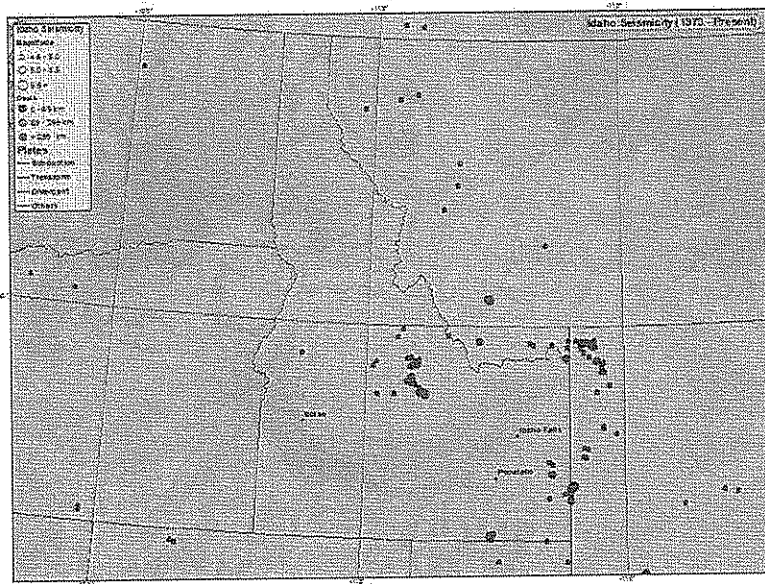
2/18/2016

Idaho



Idaho

Seismicity Map - 1973 to March 2012



Share this page: [Facebook](#) [Twitter](#) [Google](#) [Email](#)

<http://earthquake.usgs.gov/earthquakes/states/idaho/seismicity.php>

1/1



**EHM Engineers, Inc.**  
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Form 238-7  
1/78

STATE OF IDAHO  
DEPARTMENT OF WATER RESOURCES  
**WELL DRILLER'S REPORT**

USE TYPEWRITER OR  
BALLPOINT PEN

State law requires that this report be filed with the Director, Department of Water Resources within 30 days after the completion or abandonment of the well.

<p><b>1. WELL OWNER</b></p> <p>Name <u>Buhl Cemetery</u></p> <p>Address <u>Buhl, Idaho 83316</u></p> <p>Owner's Permit No. _____</p>	<p><b>7. WATER LEVEL</b></p> <p>Static water level <u>105</u> feet below land surface.</p> <p>Flowing? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No G.P.M. flow _____</p> <p>Artesian closed-in pressure _____ p.s.i.</p> <p>Controlled by: <input type="checkbox"/> Valve <input type="checkbox"/> Cap <input type="checkbox"/> Plug</p> <p>Temperature <u>57°F</u> Quality _____</p>																																																																								
<p><b>2. NATURE OF WORK</b></p> <p><input checked="" type="checkbox"/> New well <input type="checkbox"/> Deepened <input type="checkbox"/> Replacement</p> <p><input type="checkbox"/> Abandoned (describe method of abandoning) _____</p>	<p><b>8. WELL TEST DATA</b></p> <p><input type="checkbox"/> Pump <input checked="" type="checkbox"/> Bailor <input type="checkbox"/> Air <input type="checkbox"/> Other _____</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>Discharge G.P.M.</th> <th>Pumping Level</th> <th>Hours Pumped</th> </tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </table>	Discharge G.P.M.	Pumping Level	Hours Pumped																																																																					
Discharge G.P.M.	Pumping Level	Hours Pumped																																																																							
<p><b>3. PROPOSED USE</b></p> <p><input checked="" type="checkbox"/> Domestic <input type="checkbox"/> Irrigation <input type="checkbox"/> Test <input type="checkbox"/> Municipal</p> <p><input type="checkbox"/> Industrial <input type="checkbox"/> Stock <input type="checkbox"/> Waste Disposal or Injection</p> <p><input type="checkbox"/> Other _____ (specify type)</p>	<p><b>9. LITHOLOGIC LOG</b> <span style="float: right;"><b>73627</b></span></p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Note</th> <th colspan="2">Depth</th> <th rowspan="2">Material</th> <th rowspan="2">Water</th> </tr> <tr> <th>From</th> <th>To</th> </tr> </thead> <tbody> <tr><td> </td><td>0</td><td>1</td><td>Top Soil</td><td> </td></tr> <tr><td> </td><td>1</td><td>3</td><td>Hard PAN</td><td> </td></tr> <tr><td> </td><td>3</td><td>12</td><td>L.V.L.</td><td> </td></tr> <tr><td> </td><td>12</td><td>15</td><td>Boulders</td><td> </td></tr> <tr><td> </td><td>15</td><td>26</td><td>L.V.L.</td><td> </td></tr> <tr><td> </td><td>26</td><td>61</td><td>L.V.L. Strip 327 Cylinders</td><td> </td></tr> <tr><td> </td><td>57</td><td>73</td><td>Reddish Brown Ryolite</td><td> </td></tr> <tr><td> </td><td>73</td><td>80</td><td>Cylinders</td><td> </td></tr> <tr><td> </td><td>80</td><td>84</td><td>Brown Ryolite</td><td> </td></tr> <tr><td> </td><td>84</td><td>90</td><td>Cylinders</td><td> </td></tr> <tr><td> </td><td>90</td><td>121</td><td>Brownish Red Ryolite</td><td> </td></tr> <tr><td> </td><td>121</td><td>136</td><td>L.V.L. Strip 327 Cylinders</td><td> </td></tr> <tr><td> </td><td>136</td><td>135</td><td>Cylinders</td><td> </td></tr> </tbody> </table>	Note	Depth		Material	Water	From	To		0	1	Top Soil			1	3	Hard PAN			3	12	L.V.L.			12	15	Boulders			15	26	L.V.L.			26	61	L.V.L. Strip 327 Cylinders			57	73	Reddish Brown Ryolite			73	80	Cylinders			80	84	Brown Ryolite			84	90	Cylinders			90	121	Brownish Red Ryolite			121	136	L.V.L. Strip 327 Cylinders			136	135	Cylinders	
Note	Depth		Material	Water																																																																					
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<p><b>4. METHOD DRILLED</b></p> <p><input type="checkbox"/> Rotary <input type="checkbox"/> Air <input type="checkbox"/> Hydraulic <input type="checkbox"/> Reverse rotary</p> <p><input checked="" type="checkbox"/> Cable <input type="checkbox"/> Dug <input type="checkbox"/> Other _____</p>	<p><b>10. RECEIVED</b></p> <p>MAR 12 1982</p> <p>Department of Water Resources</p> <p><b>RECEIVED</b></p> <p>MAR 8 1982</p> <p>Department of Water Resources</p> <p>Business Office</p>																																																																								
<p><b>5. WELL CONSTRUCTION</b></p> <p>Casing schedule: <input checked="" type="checkbox"/> Steel <input type="checkbox"/> Concrete <input type="checkbox"/> Other</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Thickness</th> <th>Diameter</th> <th>From</th> <th>To</th> </tr> </thead> <tbody> <tr> <td>250 inches</td> <td>6 inches</td> <td>1 feet</td> <td>20 feet</td> </tr> <tr><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td></tr> </tbody> </table> <p>Was casing drive shoe used? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No</p> <p>Was a packer or seal used? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No</p> <p>Perforated? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No</p> <p>How perforated? <input type="checkbox"/> Factory <input type="checkbox"/> Knife <input type="checkbox"/> Torch</p> <p>Size of perforation _____ inches by _____ inches</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Number</th> <th>From</th> <th>To</th> </tr> </thead> <tbody> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table> <p>Well screen installed? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No</p> <p>Manufacturer's name _____</p> <p>Type _____ Model No. _____</p> <p>Diameter _____ Slot size _____ Set from _____ feet to _____ feet</p> <p>Diameter _____ Slot size _____ Set from _____ feet to _____ feet</p> <p>Gravel packed? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> Size of gravel _____</p> <p>Placed from _____ feet to _____ feet</p> <p>Surface seal depth <u>19</u> Material used in seal: <input checked="" type="checkbox"/> Cement grout</p> <p><input type="checkbox"/> Pudding clay <input checked="" type="checkbox"/> Well cuttings</p> <p>Sealing procedure used: <input type="checkbox"/> Slurry pit <input checked="" type="checkbox"/> Temp. surface casing</p> <p><input checked="" type="checkbox"/> Overbore to seal depth</p> <p>Method of joining casing: <input type="checkbox"/> Threaded <input type="checkbox"/> Welded <input type="checkbox"/> Solvent</p> <p><input type="checkbox"/> Weld</p> <p><input type="checkbox"/> Cemented between strata</p> <p>Describe access port <u>1 1/4" Plug In Well Cap</u></p>	Thickness	Diameter	From	To	250 inches	6 inches	1 feet	20 feet																	Number	From	To										<p><b>11. DRILLERS CERTIFICATION</b></p> <p>I/We certify that all minimum well construction standards were complied with at the time the rig was removed.</p> <p>Firm Name <u>B+B Drilling</u> Firm No. <u>171</u></p> <p>Address <u>411 Hagerman</u> Date <u>2/10/82</u></p> <p>Signed by (Firm Official) <u>Ron Bishop</u></p> <p>and <u>Leon Burkhardt</u></p> <p>(Operator)</p>																																				
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**EHM Engineers, Inc.**  
BUILDING THE FUTURE ON A FOUNDATION OF EXCELLENCE

Basin 47 App 8 25063

Per 789675

10360690

9874

8-7

**IDAHO DEPARTMENT OF WATER RESOURCES  
WELL DRILLER'S REPORT**

1. WELL TAG NO. D 6023490  
DRILLING PERMIT NO. 789675  
Water Right or Injection Well No. \_\_\_\_\_

2. OWNER: Homestead Investments, Inc.  
Name 2151-A East 3950 North  
Address Filer State ID Zip 83328  
City \_\_\_\_\_

3. LOCATION OF WELL by legal description:

You must provide address or Lot, Blk, Sub. or Directions to well.  
Twp. 5 North ☐ or South ☒  
Rge. 15 East ☒ or West ☐  
Sec. 21 1/4 NW 1/4 NE 1/4  
Gov't Lot \_\_\_\_\_ County Blaine Falls  
Lat. : : Long. : :  
Address of Well Site Nr Buell Cemetery City Filer  
(Give street name or name of road - Distance to Road or Landmark)  
Lt. 1 Blk. Sub. Name Easy Living Estates

4. USE:  
☒ Domestic ☐ Municipal ☐ Monitor ☐ Irrigation  
☐ Thermal ☐ Injection ☐ Other \_\_\_\_\_

5. TYPE OF WORK check all that apply (Replacement etc.)  
☒ New Well ☐ Modify ☐ Abandonment ☐ Other \_\_\_\_\_

6. DRILL METHOD:  
☒ Air Rotary ☐ Cable ☐ Mud Rotary ☐ Other \_\_\_\_\_

7. SEALING PROCEDURES

Seal Material	From	To	Weight/Volume	Seal Placement Method
BENTONITE	5	18	3 S	Poured

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s) \_\_\_\_\_  
Was drive shoe seal tested? ☐ Y ☒ N How? \_\_\_\_\_

8. CASING/LINER:

Diameter	From	To	Gauge	Material	Casing	Lin.	Welded	Threaded
6"	42	18	250	Steel	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

Length of Headpipe \_\_\_\_\_ Length of Tailpipe \_\_\_\_\_  
Packer ☐ Y ☒ N Type \_\_\_\_\_

9. PERFORATIONS/SCREENS PACKER TYPE

Perforation Method						Casing	Liner
Screen Type & Method of Installation							
From	To	Slot Size	Number	Diameter	Material		
						<input type="checkbox"/>	<input type="checkbox"/>
						<input type="checkbox"/>	<input type="checkbox"/>
						<input type="checkbox"/>	<input type="checkbox"/>
						<input type="checkbox"/>	<input type="checkbox"/>

10. FILTER PACK

Filter Material	From	To	Weight/Volume	Placement Method

11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:

124 ft. below ground Artesian pressure \_\_\_\_\_ ft.  
Depth flow encountered \_\_\_\_\_ ft. Describe access port or control devices: \_\_\_\_\_

12. WELL TESTS:

Yield gal/min	Drawdown	Pumping Level	Time

Water Temp. \_\_\_\_\_ Bottom hole temp. \_\_\_\_\_  
Water Quality test or comments: \_\_\_\_\_

13. LITHOLOGIC LOG: (Describe repairs or abandonment)

Bore Dia.	From	To	Remarks: Lithology, Water Quality & Temperature	Water
8"	0	9	Topsoil	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	9	18	Gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
6"	18	64	Gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	64	80	Fractured gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	80	85	Ash & cinders	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	85	133	Fractured gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	133	144	Brown basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	144	200	Fractured gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
			5-12-03 Deepen	
	200	248	Fractured gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	248	252	Brown cinders	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N
	252	280	Fractured gray basalt	<input checked="" type="checkbox"/> Y <input type="checkbox"/> N

**RECEIVED**

**MAY 15 2003**

Department of Water Resources  
Southern Region

Completed Depth 280 ft. (Maximum)  
Date: Started 4-16-03 Completed 4-16-03

14. DRILLER'S CERTIFICATION

(We certify that all minimum well construction standards were complied with at the time the rig was removed.)

Company Name Easing Drilling Firm No. 31  
Principal Driller Donald H. Hines Date 4-16-03  
and Don H. Hines Date 4-16-03  
Driller or Operator II Don H. Hines Date 4-16-03  
Operator I Don H. Hines Date \_\_\_\_\_  
Principal Driller and Rig Operator Required.  
Operator I must have signature of Driller/Operator II.

FORWARD WHITE COPY TO WATER RESOURCES



Form 238-7  
1/78

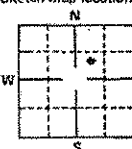
STATE OF IDAHO  
 DEPARTMENT OF WATER RESOURCES  
**WELL DRILLER'S REPORT**

State law requires that this report be filed with the Director, Department of Water Resources within 30 days after the completion or abandonment of the well.

USE TYPEWRITER OR  
BALLPOINT PEN

**RECEIVED**

DEC 18 1985

<b>1. WELL OWNER</b> Name <u>West End Cemetery</u> Address <u>Rt 4 Buhl, Idaho</u> Owner's Permit No. <u>47-7439</u>	<b>7. WATER LEVEL</b> Static water level <u>105</u> feet below land surface Flowing? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No G.P.M. flow _____ Artesian closed-in pressure _____ p.s.i. Controlled by: <input type="checkbox"/> Valve <input type="checkbox"/> Cap <input type="checkbox"/> Plug Temperature <u>57°F</u> Quality _____																																																																																																																												
<b>2. NATURE OF WORK</b> <input checked="" type="checkbox"/> New well <input type="checkbox"/> Deepened <input type="checkbox"/> Replacement <input type="checkbox"/> Abandoned (describe method of abandoning) _____	<b>8. WELL TEST DATA</b> <input checked="" type="checkbox"/> Pump <input checked="" type="checkbox"/> Bailor <input type="checkbox"/> Air <input type="checkbox"/> Other _____ <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>Discharge G.P.M.</th> <th>Pumping Level</th> <th>Hours Pumped</th> </tr> <tr> <td align="center"><u>110</u></td> <td align="center"><u>168</u></td> <td align="center"><u>24</u></td> </tr> </table>	Discharge G.P.M.	Pumping Level	Hours Pumped	<u>110</u>	<u>168</u>	<u>24</u>																																																																																																																						
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USE ADDITIONAL SHEETS IF NECESSARY - FORWARD THE WHITE COPY TO THE DEPARTMENT



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Page 50