

GENERAL NOTES

I. GENERAL

1. MATERIALS AND WORKMANSHIP TO CONFORM WITH THE 2009 EDITION OF THE INTERNATIONAL BUILDING CODE AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
2. THESE GENERAL NOTES SUPPLEMENT THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS. IN CASE OF CONFLICT BETWEEN THE PLANS AND SPECIFICATIONS, CONTACT THE OWNER'S REPRESENTATIVE.
3. REFERENCE TO CODES, RULES, REGULATIONS, STANDARDS, MANUFACTURER'S INSTRUCTIONS OR REQUIREMENTS OF REGULATORY AGENCIES IS TO THE LATEST PRINTED EDITION OF EACH IN EFFECT AT THE DATE OF SUBMISSION OF BID UNLESS THE DOCUMENT DATE IS SHOWN.
4. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW BY THE OWNER'S REPRESENTATIVE.
5. DETAILS ON SHEETS TITLED "TYPICAL DETAILS" APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. SUCH DETAILS ARE NOT NOTED AT EACH LOCATION THAT THEY OCCUR.
6. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND FOR CHECKING DIMENSIONS. NOTIFY THE OWNER'S REPRESENTATIVE OF ANY DISCREPANCIES AND RESOLVE BEFORE PROCEEDING WITH THE WORK.
7. DO NOT SCALE THE DRAWINGS.
8. PROVIDE MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES INCLUDE, BUT MAY NOT BE LIMITED TO, BRACING AND SHORING FOR LOADS DURING CONSTRUCTION. RETAIN A REGISTERED CIVIL ENGINEER WHOM IS PROPERLY QUALIFIED TO DESIGN BRACING, SHORING, ETC. VISITS TO THE SITE BY THE OWNER'S REPRESENTATIVE WILL NOT INCLUDE OBSERVATION OF THE ABOVE NOTED ITEMS.
9. INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENTS THE PRESENT KNOWLEDGE, BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE OWNER'S REPRESENTATIVE. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE OWNER'S REPRESENTATIVE.
10. REFER TO ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF FLOOR, ROOF AND WALL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS. COORDINATE THE SIZE AND LOCATION OF OPENINGS ASSOCIATED WITH, BUT NOT LIMITED TO, ELECTRICAL, MECHANICAL AND PLUMBING TRADES. SUBMIT FINAL SIZING AND LOCATION REQUIREMENTS OF OPENINGS TO THE OWNER'S REPRESENTATIVE FOR REVIEW.
11. REFERENCE DATUM FOR THE ELEVATIONS IS ELEVATION = 471.0 FEET, LEVEL 1 FINISH FLOOR.

II. FOUNDATION AND SITE WORK

1. THE DESIGN OF THE FOUNDATION SYSTEM IS BASED UPON THE CRITERIA AND RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT ENTITLED "GEOTECHNICAL INVESTIGATION REPORT, ROSEBURG VETERANS ADMINISTRATION BUILDINGS PROJECT, ROSEBURG, OREGON" BY PBS ENGINEERING + ENVIRONMENTAL, DATED APRIL 21, 2011
2. GROUNDWATER ELEVATION IS ESTIMATED BETWEEN APPROXIMATELY ELEVATION 454 AND 460 FEET.
3. LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
4. REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED.
5. NOTIFY THE OWNER'S REPRESENTATIVE IF ANY BURIED STRUCTURES NOT INDICATED, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC., ARE FOUND.
6. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
7. REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE.
8. EXCAVATIONS FOR FOUNDATIONS MUST BE ACCEPTED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING REINFORCING AND CONCRETE. NOTIFY THE GEOTECHNICAL ENGINEER WHEN EXCAVATIONS ARE READY FOR INSPECTION.
9. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE OR MASONRY HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.
10. PROVIDE POSITIVE SLOPES, BOTH DURING AND AFTER CONSTRUCTION, TO DIRECT SURFACE AND ROOF RUNOFF A MINIMUM OF 10'-0" FROM BUILDING FOUNDATIONS.

III. REINFORCING STEEL

1. REINFORCING TO CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED:

LOCATION	TYPE
REINFORCING STEEL #7 AND SMALLER	ASTM A615, 60 KSI
REINFORCING STEEL #8 AND LARGER AND REINFORCING STEEL TO BE WELDED	ASTM A706, 60 KSI

2. ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCING FROM DISPLACING DUE TO FORMWORK, CONSTRUCTION, OR CONCRETE PLACEMENT OPERATIONS. LOCATE AND SUPPORT REINFORCING BY METAL CHAIRS, RUNNERS, BOLSTERS, SPACERS, & HANGERS AT A MAXIMUM 3'-0" SPACING.
3. MECHANICAL COUPLERS: LENTON THREADED OR INTERLOCK COUPLERS BY ERICO, ICC ER-3967, CADWELD BY ERICO, ICC ER-3967, OR XTENDER BY HEADED REINFORCEMENT CORPORATION, ICC ER-5308. COUPLERS FOR BEAM AND SLAB BARS AT FORMED CONSTRUCTION JOINTS MAY BE LENTON FORM SAVERS BY ERICO, ICC ER-3967, OR EQUIVALENT.
4. WELD REINFORCING STEEL IN ACCORDANCE WITH AWS D1.4 USING QUALIFIED WELDERS.
5. TERMINATE REINFORCING STEEL IN STANDARD HOOKS, UNLESS OTHERWISE SHOWN.
6. PROVIDE REINFORCING SHOWN OR NOTED CONTINUOUS IN LENGTHS AS LONG AS PRACTICABLE.

IV. CAST-IN-PLACE CONCRETE

1. CONCRETE IS REINFORCED AND CAST-IN-PLACE UNLESS OTHERWISE NOTED. WHERE REINFORCING IS NOT SPECIFICALLY SHOWN OR WHERE DETAILS ARE NOT GIVEN, PROVIDE REINFORCING SIMILAR TO THAT SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE OWNER'S REPRESENTATIVE. SUBSTITUTION OF SHOTCRETE FOR CAST-IN-PLACE CONCRETE IS NOT ACCEPTABLE.
2. ROUGHEN CONCRETE SURFACES OF CONSTRUCTION JOINTS TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES. LOCATE CONSTRUCTION JOINTS AS DIRECTED ON THE DRAWINGS. SUBMIT ALTERNATE JOINT LOCATIONS OR JOINTS NOT SHOWN TO THE OWNER'S REPRESENTATIVE FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH THE WORK.
3. AT LOCATIONS WHERE CONCRETE IS CAST AGAINST EXISTING CONCRETE, ROUGHEN CONTACT SURFACES TO ¼ INCH AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES.

IV. CAST-IN-PLACE CONCRETE (cont.)

4. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF ADDITIONAL CONCRETE CURBS AND HOUSEKEEPING PADS NOT SHOWN.
5. CONCRETE CLEAR COVER TO REINFORCING BARS IS AS FOLLOWS, UNLESS OTHERWISE NOTED:

LOCATION	CLEAR COVER
CONCRETE PLACED AGAINST EARTH	3 INCHES
FORMED SURFACES EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: #6 BARS AND LARGER #5 BARS AND SMALLER SLABS ON GRADE (TOP CLEARANCE)	2 INCHES 1 ½ INCHES 1 ½ INCHES
BEAMS, GIRDERS AND COLUMNS NOT EXPOSED TO WEATHER OR EARTH	1 ½ INCHES
WALL OR SLAB SURFACES NOT EXPOSED TO WEATHER OR EARTH: #11 & SMALLER #14 & #18	¾ INCH 1 ½ INCHES
6. CONCRETE TYPES:

CLASS	28-DAY STRENGTH	TYPE	LOCATION
A	4000 PSI	NORMAL WEIGHT	FOUNDATIONS.
B	4000 PSI	NORMAL WEIGHT	SLABS ON GRADE.
C	4000 PSI	NORMAL WEIGHT	PILASTERS AND WALLS
D	4000 PSI	NORMAL WEIGHT	FILL ON METAL DECK
E	4000 PSI	NORMAL WEIGHT	CURBS, PADS, AND MISC.
7. CONTINUOUSLY MOIST CURE CONCRETE SLABS-ON-GRADE FOR 7 DAYS MINIMUM. WATER FOG SPRAYS, PONDING, SATURATED ABSORPTIVE COVERS, OR MOISTURE RETAINING COVERS MAY BE USED. CURING COMPOUNDS ARE NOT ACCEPTABLE.
8. CHAMFER: ¾ INCH ON EXPOSED CORNERS, U.O.N.
9. PIPES OR CONDUITS: PLACE IN MIDDLE THIRD OF SLABS AND WALLS. MAXIMUM DIAMETER ONE-THIRD OF SLAB OR WALL THICKNESS. MINIMUM SPACING 3 DIAMETERS O.C.

V. STRUCTURAL STEEL

1. STRUCTURAL STEEL TO CONFORM TO THE FOLLOWING UNLESS OTHERWISE NOTED:

SECTIONS	TYPE
ROLLED SHAPES	
WIDE FLANGES	ASTM A992
CHANNELS, ANGLES, & OTHER	ASTM A36
PLATES	
COLUMN BASE PLATES	ASTM A572, GR 50
BRACE GUSSET PLATES	ASTM A572, GR 50
BEAM SHEAR CONNECTION PLATES	ASTM A572, GR 50
COLUMN CONTINUITY PLATES	ASTM A572, GR 50
BEAM STIFFENER PLATES	ASTM A542, GR 50
DECK CLOSURE PLATES	ASTM A36
OTHER	ASTM A572, GR 50
STEEL PIPE	ASTM A53 GRADE B
COLD FORMED HOLLOW STRUCTURAL SECTION (HSS)	ASTM A500 GRADE B
STAINLESS STEEL SHAPES, PLATES AND BARS	ASTM A276
BOLTS	ASTM A325X
MACHINE BOLTS	ASTM A307
ANCHOR RODS	ASTM F1554, GR36
THREADED AND HANGER ROD	ASTM A572, GR50
WELDED SHEAR CONNECTORS	ASTM A 108, GRADE 1015 THROUGH 1020
WELDED THREADED STUDS	ASTM A 108, GRADE 1015 THROUGH 1020
NUTS FOR BOLTS AND MACHINE BOLTS	ASTM A563
HARDENED WASHERS	ASTM F436
UNHARDENED WASHERS	ASTM F844
PLAIN WASHERS	ANSI B18.22.1
BEVELED WASHERS	ANSI B18.23.1

2. HOT DIP GALVANIZE IN ACCORDANCE WITH ASTM A123 AND ASTM A153 STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER. REPAIR GALVANIZING AFTER WELDING IN ACCORDANCE WITH ASTM A780.
3. STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO VIEW IN THE COMPLETED BUILDING ARE DESIGNATED ARCHITECTUALLY EXPOSED STRUCTURAL STEEL (AESS). SEE PLANS FOR LOCATIONS.
4. ARC-WELDING ELECTRODES/FILLER METALS TO BE LOW HYDROGEN TYPES E7XTX, E7XTXX OR E70XXX MINIMUM AS APPLICABLE. ELECTRODES WITH CHARPY V-NOTCH (CVN) TESTS VALUES OF A MINIMUM 20 FOOT-POUNDS AT -20 DEGREES FAHRENHEIT ARE TO BE USED AT THE FOLLOWING LOCATIONS:
 - COMPLETE JOINT PENETRATION WELDS
 - BEAM TO COLUMN MOMENT CONNECTIONS: INCLUDING FLANGE, WEB, AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
 - BRACE CONNECTIONS: INCLUDING BRACE, GUSSET, BASE PLATES, BEAM STIFFENER PLATES, AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
 - WELDS NOTED "CVN" ON THE DRAWINGS
5. WELDERS TO BE CERTIFIED BY AWS AND THE GOVERNING JURISDICTION.
6. WHERE FIELD WELDING IS NOTED, THE DESIGNATION IS GIVEN AS A SUGGESTED CONSTRUCTION PROCEDURE ONLY.
7. PROVIDE NATURAL CAMBER UP, UNLESS NOTED OTHERWISE, EXCEPT AT CANTILEVERS. AT CANTILEVERS PROVIDE CAMBER SUCH THAT TIP OF CANTILEVER IS ABOVE FINAL ELEVATION.
8. SPLICE MEMBERS ONLY WHERE INDICATED.
9. BUCKLING RESTRAINED BRACES SHALL BE SUPPLIED BY COREBRACE IN ACCORDANCE WITH SPECIFICATIONS AND SHEET SF-512.

VI. METAL DECKING

1. METAL ROOF DECK TO HAVE MINIMUM SECTION PROPERTIES SHOWN IN THE METAL DECK SCHEDULE REFERENCED FROM THE PLANS.
2. ALL ROOF DECK TO BE GALVANIZED IN ACCORDANCE WITH ASTM A653 COATING CLASS G60. REPAIR DAMAGED COATING.
3. WHERE POSSIBLE, LAYOUT METAL DECK TO SPAN AT LEAST THREE SPANS CONTINUOUSLY. TERMINATE ENDS OVER SUPPORTS EXCEPT AT OPENINGS OR BUILDING EDGES WHERE METAL DECKS MAY BE CANTILEVERED AS SHOWN.
4. MINIMUM DECK ATTACHMENT TO BE PER THE METAL DECK SCHEDULE.

VII. COLD FORMED METAL FRAMING

1. METAL STUD FRAMING MATERIALS TO CONFORM TO AISI SPECIFICATIONS.
2. SHEET METAL SCREWS "S.M.S." SHALL BE SELF-DRILLING/SELF-TAPPING SCREWS INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
3. MINIMUM SPACING AND EDGE DISTANCE OF SCREWS SHALL BE 3/4" FOR ALL SCREWS UNLESS OTHERWISE NOTED.
4. ALL SHEET METAL SCREWS SHALL BE #8 UNLESS OTHERWISE NOTED. INSTALLED SCREWS SHALL PROTRUDE THROUGH STEEL FRAMING THE LARGER OF 1/4" OR (3) EXPOSED FULL-DIAMETER THREADS.
- VIII. MECHANICAL ANCHORS
1. EXPANSION OR WEDGE ANCHORS INTO CONCRETE: HILTI KWIK BOLT TZ (ICC-ES ESR-1917) OR SIMPSON STRONG TIE STRONG-BOLT (ICC-ES ESR-1771). INSTALL ANCHORS IN ACCORDANCE WITH ICC REPORT. X-RAY OR FERROSCAN CONCRETE TO LOCATE REINFORCING PRIOR TO DRILLING ANY HOLES.
2. PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN EXPOSED TO WEATHER. PROVIDE ELECTRO-PLATED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.
3. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.
4. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.
5. ANCHORS WILL BE PROOF-TESTED BY OWNER'S TESTING AND INSPECTION AGENCY.
6. IF ANY ANCHOR FAILS TESTING, REPLACE ANCHOR AND TEST ADDITIONAL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME INITIAL TESTING FREQUENCY.
7. APPLY TEST LOAD BY ANY METHOD THAT WILL EFFECTIVELY MEASURE THE TENSION ON THE ANCHOR SUCH AS DIRECT PULL WITH A HYDRAULIC JACK, TORQUE WRENCH, OR CALIBRATED SPRING LOADING DEVICES, ETC.
8. TEST ANCHORS NO SOONER THAN 24 HOURS AFTER INSTALLATION.
9. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY A BASEPLATE OR OTHER FIXTURE. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE THE FIXTURE PRIOR TO TESTING.
10. TEST 10% OF WEDGE ANCHORS PER ONE OF THE FOLLOWING METHODS:
 - A. HYDRAULIC RAM METHOD: APPLY PROOF TEST LOAD WITHOUT REMOVING THE NUT. IF IT IS NOT POSSIBLE TO TEST WITH THE NUT INSTALLED, REPLACE THE NUT WITH A THREADED COUPLER TO THE SAME TORQUE MEASURED WITH A TORQUE WRENCH AND THEN APPLY THE LOAD. ANCHOR IS ACCEPTABLE IF NO MOVEMENT IS OBSERVED AT THE TEST LOAD. MOVEMENT MAY BE DETERMINED WHEN THE WASHER UNDER THE NUT BECOMES LOOSE.
 - B. TORQUE WRENCH METHOD: TEST ANCHORS TO THE TORQUE LOAD INDICATED IN THE TABLE BELOW, FOR WEDGE ANCHORS, ONE-HALF TURN OF THE NUT. WITHIN THE FOLLOWING LIMITS:
- | ANCHOR DIA. (INCHES) | TENSION LOAD (LBS) | TORQUE LOAD (FT-LBS) |
|----------------------|--------------------|----------------------|
| 3/8" | 2100 | 25 |
| 1/2" | 4500 | 40 |
| 5/8" | 6100 | 60 |
| 3/4" | 7800 | 110 |
- IX. ADHESIVE ANCHORS AND DOWELS
1. ANCHORS AND DOWELS INSTALLED INTO CONCRETE: SIMPSON STRONG TIE SET-XP (ICC-ES ESR-2508), HILTI HIT-RE 500-SD (ICC-ES ESR-2322). EMBEDMENT DEPTH FOR ANCHORS AND DOWELS IS AS FOLLOWS, UNLESS OTHERWISE NOTED. THE TESTING LABORATORY WILL PERFORM TENSION TESTS ON 25% OF ANCHORS AND DOWELS TO THE FOLLOWING TEST LOADS. X-RAY OR FERROSCAN CONCRETE TO LOCATE REINFORCING PRIOR TO DRILLING ANY HOLES.

ROD DIA. OR BAR SIZE	EMBEDMENT	TEST LOAD
3/8"	4"	1800#
1/2"	5"	3200#
5/8"	6"	5000#
3/4"	7"	7100#
7/8"	9"	9700#
1"	11"	12800#
#3	5"	3000#
#4	6 1/2"	5400#
#5	8"	8400#
#6	10"	11900#
#7	12"	16200#
#8	14"	21300#

2. ANCHORS: ASTM A572 GR50 THREADED RODS WITH ASTM A 563 GRADE A NUTS AND ANSI B18.22.1 TYPE A WASHERS, UNLESS OTHERWISE NOTED. ANCHORS DESIGNATED AS ASTM A193 GRADE B7 THREADED RODS TO USE ASTM A 563 GRADE DH HEAVY HEX NUTS AND ASTM F 436 WASHERS.

3. DOWELS: ASTM A615 GRADE 60 REINFORCING STEEL.

4. WHEEL STOP EPOXY SHALL COMPLY WITH ASTM C881 AND AASHTO M235.

5. REMOVE GREASE, OIL, RUST, AND OTHER LAITANCE FROM RODS AND DOWELS PRIOR TO INSTALLATION.

6. REPLACE ANCHORS AND DOWELS THAT FAIL DURING TESTING AND RETEST. IF MORE THAN 10% OF THE TESTED DOWELS AND ANCHORS FAIL TO ACHIEVE THE SPECIFIED TEST LOAD, TEST 100% OF THE DOWELS AND ANCHORS INSTALLED IN THE LAST 2 DAYS OF ANCHOR INSTALLATION.

7. PREPARE HOLES AND INSTALL ANCHORS AND DOWELS IN ACCORDANCE WITH ICC-ES REPORT AND MANUFACTURERS INSTRUCTIONS.

8. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.

9. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH ADHESIVE ANCHORS.
- X. STRUCTURAL TESTS, INSPECTIONS, AND OBSERVATIONS
1. AN INDEPENDENT TESTING AGENCY AND SPECIAL INSPECTORS WILL BE RETAINED BY THE OWNER TO PERFORM TESTS AND INSPECTIONS IN ACCORDANCE WITH THE APPROVED TESTING, INSPECTION AND OBSERVATION PROGRAM. PROVIDE ACCESS AND FURNISH SAMPLES TO THE AGENCY AS REQUIRED BY THE CONTRACT DOCUMENTS.

2. IF INITIAL TESTS OR INSPECTIONS MADE BY THE OWNER'S TESTING AGENCY REVEAL THAT ANY PORTION OF THE WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS WILL BE MADE AT THE CONTRACTOR'S EXPENSE.

3. THE FOLLOWING ITEMS REQUIRE TESTS & INSPECTIONS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CHAPTER "STRUCTURAL TESTS & INSPECTIONS" OF THE 2009 INTERNATIONAL BUILDING CODE. ADDITIONAL ITEMS AND REQUIREMENTS FOR TESTS & INSPECTIONS ARE IDENTIFIED IN THE SPECIFICATIONS.
 - A. REINFORCING STEEL
 - B. FOUNDATIONS.
 - C. CONCRETE
 - D. STRUCTURAL STEEL
 - E. WELDING
 - F. POST-INSTALLED ANCHORS
 - G. BUCKLING RESTRAINED BRACES.
 - H. DEFORMED BAR ANCHORS
 - I. METAL DECK SHEAR STUDS

4. NOTIFY THE ENGINEER AT SIGNIFICANT CONSTRUCTION STAGES 72 HOURS IN ADVANCE & PROVIDE ACCESS FOR THE FOLLOWING STRUCTURAL OBSERVATIONS:
 - A. FOUNDATIONS
 - 1. REINFORCEMENT
 - B. STEEL FRAMING
 - 1. GENERAL
 - 2. BRACED FRAMES
 - 3. METAL DECKING & SHEAR CONNECTORS
 - 4. DEFORMED BAR ANCHORS.
 - C. CONCRETE
 - 1. WALL REINFORCEMENT
 - 2. SLABS & SLABS ON GRADE.
 - 3. CONCRETE FILL REINFORCEMENT.
- XI. DEFERRED SUBMITTALS
1. 2009 IBC, 107.3.4.2 DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE WHO SHALL REVIEW THEM AND FORWARD THEM TO THE VA WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE VA.
- THE FOLLOWING IS A LIST OF DEFERRED SUBMITTALS:
1. PREFABRICATED METAL STAIRS

2. SUSPENDED ACOUSTICAL CEILINGS

3. SEISMIC RESTRAINT FOR NONSTRUCTURAL COMPONENTS

4. ROOF-TIE-OFF SAFETY ANCHOR DESIGN

5. EXTERIOR METAL STUD FRAMING AND ATTACHMENT TO FACADE MATERIAL AND STRUCTURE

6. BUCKLING - RESTRAINED BRACES AND CONNECTIONS

7. GUARDRAIL AT LOADING DOCK RAMP

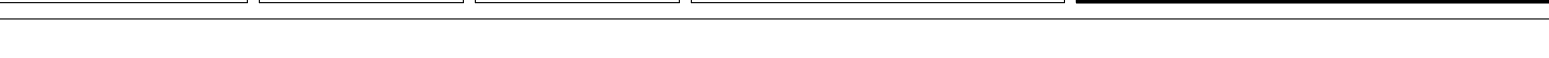
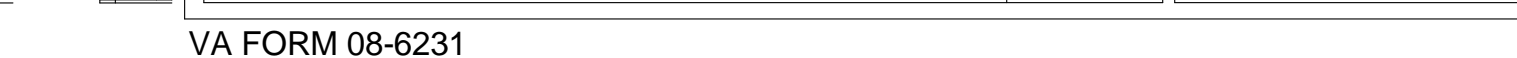
8. EQUIPMENT HOUSEKEEPING PADS
- XII. DESIGN CRITERIA
1. APPLICABLE CODE: 2009 INTERNATIONAL BUILDING CODE WITH DEPARTMENT OF VETERANS AFFAIRS H-18-8-FEBRUARY 2011 AMENDMENTS.

2. FOUNDATIONS HAVE BEEN DESIGNED BASED ON PROJECT GEOTECHNICAL REPORT BY
 - SPREAD AND CONTINUOUS STRIP FOOTINGS:
 - ALLOWABLE NET SOIL PRESSURE FOR DL + LL = 2000 PSF
 - ALLOWABLE NET SOIL PRESSURE FOR DL + LL + EQ = 4000 PSF
 - COEFFICIENT OF FRICTION = 0.35
 - PASSIVE RESISTANCE = 250 PCF
 - NEW SOIL RETAINING STRUCTURES HAVE BEEN DESIGNED WITH THE FOLLOWING CRITERIA:
 - A. AT-REST PRESSURES = 52 PCF (DRAINED)
 - B. ACTIVE PRESSURES = 34 PCF (DRAINED)
 - C. EARTHQUAKE LATERAL PRESSURE = 6.5*H² PLF @ 0.6H FROM THE BASE OF THE WALL

3. GRAVITY LOADS:
 - A. DEAD LOADS - VARY BASED ON ACTUAL BUILDING MATERIAL AND EQUIPMENT OPERATING WEIGHTS
 - B. LIVE LOADS: (REDUCIBLE PER CODE)
 - ROOF = 20 PSF OR 300# CONCENTRATED, 25 PSF FOR 4' GREEN ROOF, 45 PSF FOR 7' GREEN ROOF
 - FLOORS = 100 PSF OR 2000# CONCENTRATED
 - MECHANICAL ROOMS = 150 PSF
 - C. SNOW LOADS:
 - FLAT ROOF SNOW LOAD = 20 PSF
 - SNOW EXPOSURE FACTOR = 1.0
 - SNOW LOAD IMPORTANCE FACTOR = 1.2
 - THERMAL FACTOR = 1.0

4. SEISMIC DESIGN:
 - IMPORTANCE FACTOR = 1.5
 - OCCUPANCY CATEGORY = IV
 - SITE CLASS C
 - SITE SPECIFIC SHORT PERIOD SPECTRAL RESPONSE ACCELERATION, Sds = 0.59g
 - SITE SPECIFIC ONE-SECOND PERIOD SPECTRAL RESPONSE ACCELERATION, Sd1 = 0.39g
 - SEISMIC DESIGN CATEGORY = D
 - SEISMIC FORCE RESISTING SYSTEM = BUCKLING RESTRAINED BRACED FRAMES
 - DESIGN BASE SHEAR = 302K
 - SEISMIC RESPONSE COEFFICIENT, Cs = 0.127
 - RESPONSE MODIFICATION FACTOR, R = 7.0
 - ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

5. WIND DESIGN:
 - A. MAIN WIND FORCE RESISTING SYSTEM:
 - BASIC WIND SPEED = 85 MPH
 - WIND IMPORTANCE FACTOR = 1.15
 - WIND EXPOSURE = C
 - INTERNAL PRESSURE COEFFICIENT = +/- 0.18
 - B. COMPONENTS AND CLADDING:
 - ROOF CENTER = -19.3 PSF, +6.2 PSF
 - ROOF EDGE = -32.3 PSF, +6.2 PSF
 - ROOF CORNER = -48.6 PSF, +6.2 PSF
 - WALL CENTER = -19 PSF, +17.6 PSF
 - WALL CORNER = -23.4 PSF, +17.6 PSF
- XIII. DESIGN TEAM
- | | |
|------------------|-----------------------|
| JAMES O. MALLEY | PRINCIPAL-IN-CHARGE |
| KENT YU | PROJECT MANAGER |
| CALE ASH | ASSOCIATE PRINCIPAL |
| JENNIFER EGGERS | PROJECT ENGINEER |
| DARREN BEYER | DESIGN ENGINEER |
| JENNIFER AHLPORT | DESIGN ENGINEER |
| DAVID SOMMER | DESIGNER |
| MATT CAVANESS | SENIOR CAD SPECIALIST |
| IRINA WONG | SENIOR CAD SPECIALIST |
- | | | | | | | | | | | | | | |
|------------|--|---|--|---|--|----------------------------|--|---|--|------------------------|--|---|--------------------------|
| | | CONSULTANTS: | | ARCHITECT/ENGINEERS: | | Drawing Title | | Project Title | | Project Number | | Office of
Construction and
Facilities
Management | |
| | | Degenkolb | | Tina Ely architect | | GENERAL NOTES | | SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD | | 653-322 | | | |
| | | 1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00 | | 2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net | | Approved: Project Director | | Location
ROSEBURG, OREGON | | Building Number
086 | | | |
| Revisions: | | Date | | | | | | Date
30 APRIL 2012 | | Checked | | | Drawing Number
SF-001 |
- VA FORM 08-6231



NOTES:

1. ℓ_d = DEVELOPMENT LENGTH
2. WHEN SPLICING BARS OF DIFFERENT SIZE, USE LAP SPlice LENGTH OF LARGER BAR, U.O.N.
3. STAGGER SPLICES AS INDICATED ON DRAWING

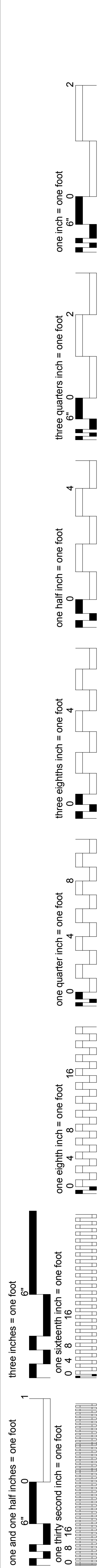
ℓ_s = LAP SPlice LENGTH

ℓ_{dh} = HOOK DEVELOPMENT LENGTH

2'-0" CLR.

COUPLER OR WELDED SPlice

KEY SCHEDULE	
T	D
< 8"	3/4"
8"-16"	1 1/2"
>16"	2 1/2"



1

A1 REINF. BAR TERMINATORS
N.T.S.

2

HEAD DIMENSIONS					
BAR SIZE	A IN.	B IN.	E IN.	F IN.	
#4	1.375"	9/16"	-	-	
#5	1.375"	7/8"	-	-	
#6	1.875"	1 1/8"	-	-	
#7	2.25"	1 1/4"	-	-	
#8	2.25"	1 3/8"	-	-	
#9	2.75"	1 1/2"	-	-	
#10	3.00"	1 9/16"	-	-	
#11	3.25"	1 11/16"	-	-	
#14	4.00"	2 1/8"	1.0	3.0"	

NOTES:

- T-HEADS TO BE LENTON TERMINATOR D6 (ICC ER 3967) BY ERICO, INC. OR EQUAL.
- ALL TYPES OF BAR ENDS ARE ACCEPTABLE, UNLESS SPECIFICALLY NOTED.
- NUMBERS SHOWN IN TABLES ABOVE ARE APPROXIMATE. VERIFY HEAD, BAR, & BULB DIMENSIONS FOR PLACEMENT OF REINF.
- EMBEDMENT LENGTH, λ_{dt} , = λ_{dh} , MEASURED FROM THE INSIDE FACE OF THE TERMINATOR.

3

A DOUBLE CURTAIN REINF. AT WALL
N.T.S.

4

B SINGLE CURTAIN REINF. AT WALL
N.T.S.

5

A5 FOOTING REINFORCING AT CORNER AND INTERSECTION
N.T.S.

B2 T-HEADED REINF. BAR STAGGER
N.T.S.

B3 OVERLAPPING FOUNDATION
N.T.S.

B4 CONCRETE CURB ON METAL DECK
3/4" = 1'-0"

B5 HOUSEKEEPING PADS
N.T.S.

C2 WALL REINFORCING AT OPENINGS
N.T.S.

TRIM REINFORCING SCHEDULE		
WALL THICKNESS "t"	MIN. TRIM REINF.	DIAGONAL REINF.
6" ≤ t ≤ 9"	(2) #5	#5
9" < t ≤ 12"	(2) #6	#5
12" < t ≤ 16"	(2) #7	#5
t > 16"	(2) #8	#7

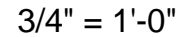
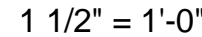
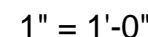
NOTES:

- SCHED. REINF. APPLIES TO ALL OPENINGS, UNLESS OTHERWISE SHOWN.
- MIN. TRIM REINF. TO BE LARGER OF TYP. WALL REINF. OR SIZE SHOWN IN SCHED.
- AT SERIES OF OPNGS WHERE PIER OR SPANDREL IS NARROWER THAN THREE TIMES λ_d , RUN TRIM REINF. CONT.
- MAY OMIT DIAGONALS IF THE LARGEST OPENING DIMENSION IS LESS THAN 3'-0".
- DETAIL IS NOT REQ'D. FOR OPENINGS SMALLER THAN THE WALL THICKNESS OR 12", WHICHEVER IS SMALLER.
- COORDINATE OPENING LOCATIONS AND SIZES W/ OTHER TRADES INCLUDING BUT NOT LIMITED TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING.

C4 PIPE SLEEVES THRU STRIP FOOTINGS
N.T.S.

C5 HOUSEKEEPING PADS
N.T.S.

C6 HOUSEKEEPING PADS
N.T.S.



NOTES: 1. ALL CELLS TO BE FULLY GROUTED.
2. SECURE REINF. AT 200 d_b MAX.
3. CONCAVE JOINTS, TYP. U.O.N.

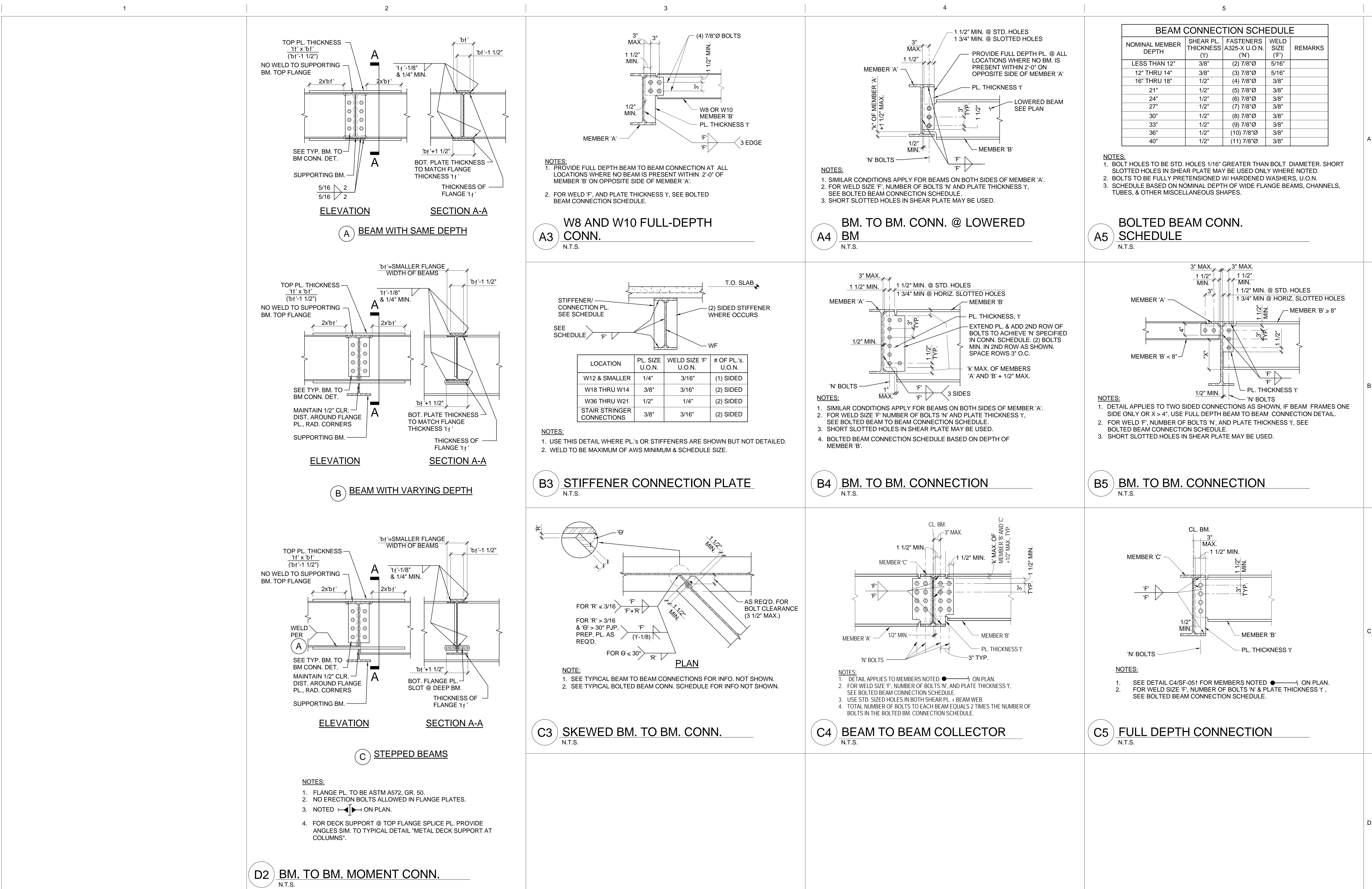
$$3/4'' = 1'-0''$$
$$3/4'' = 1'-0''$$

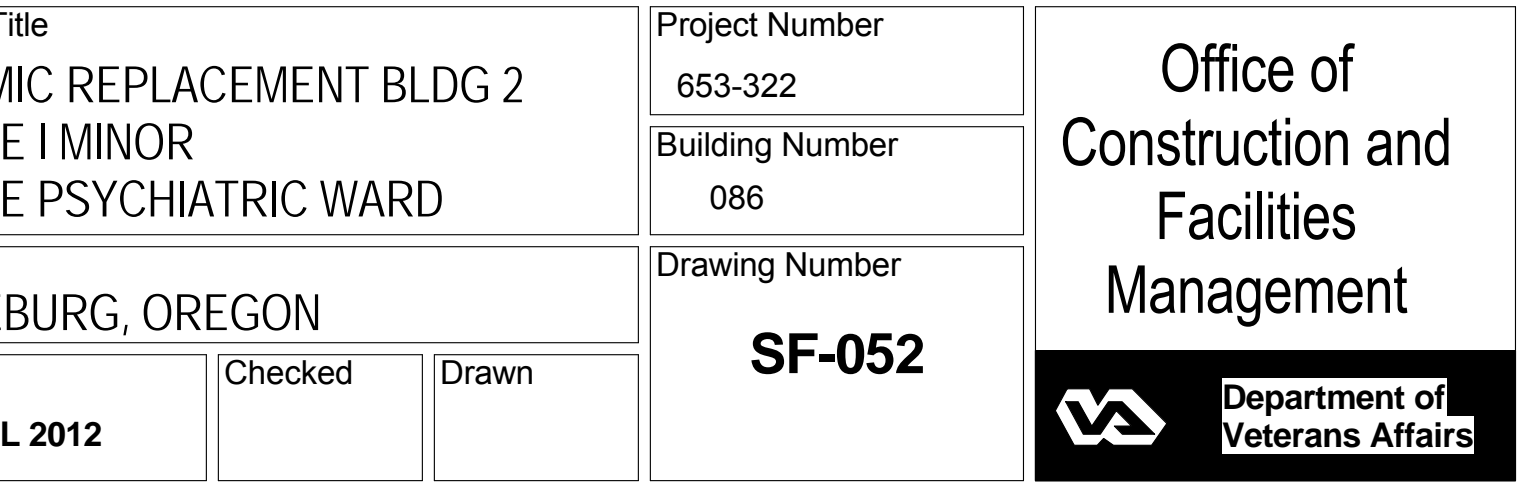
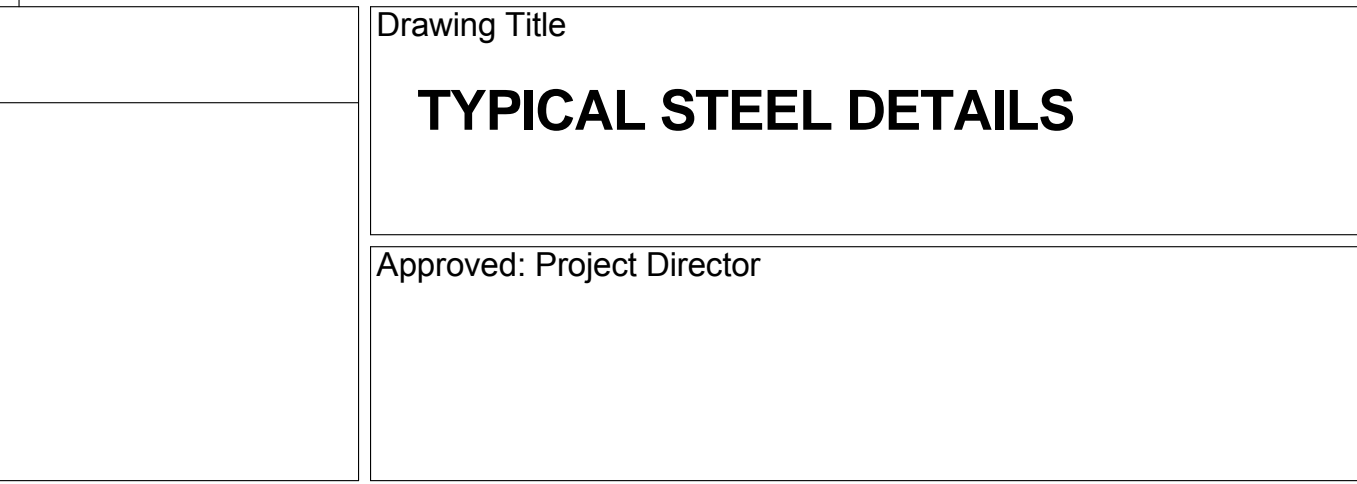
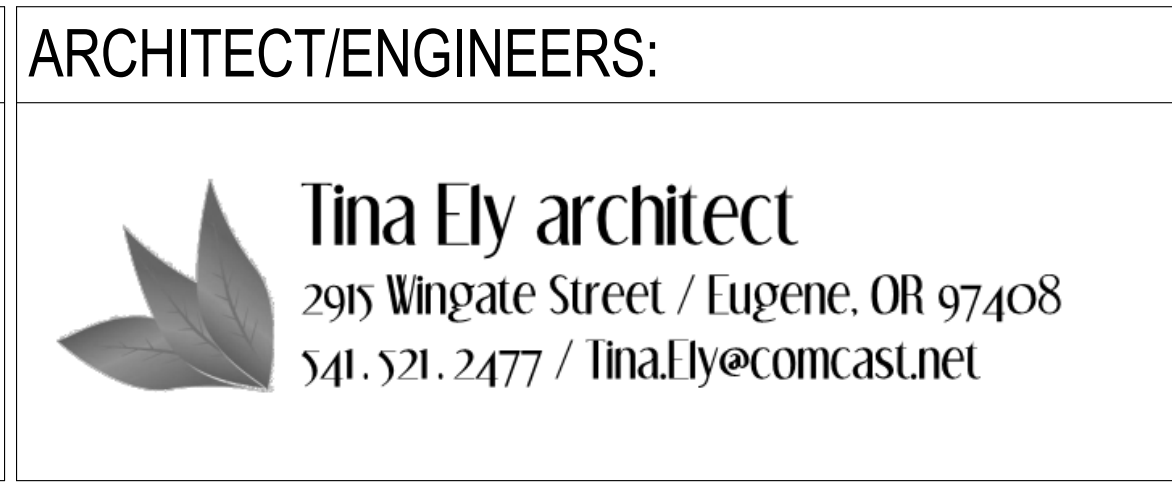
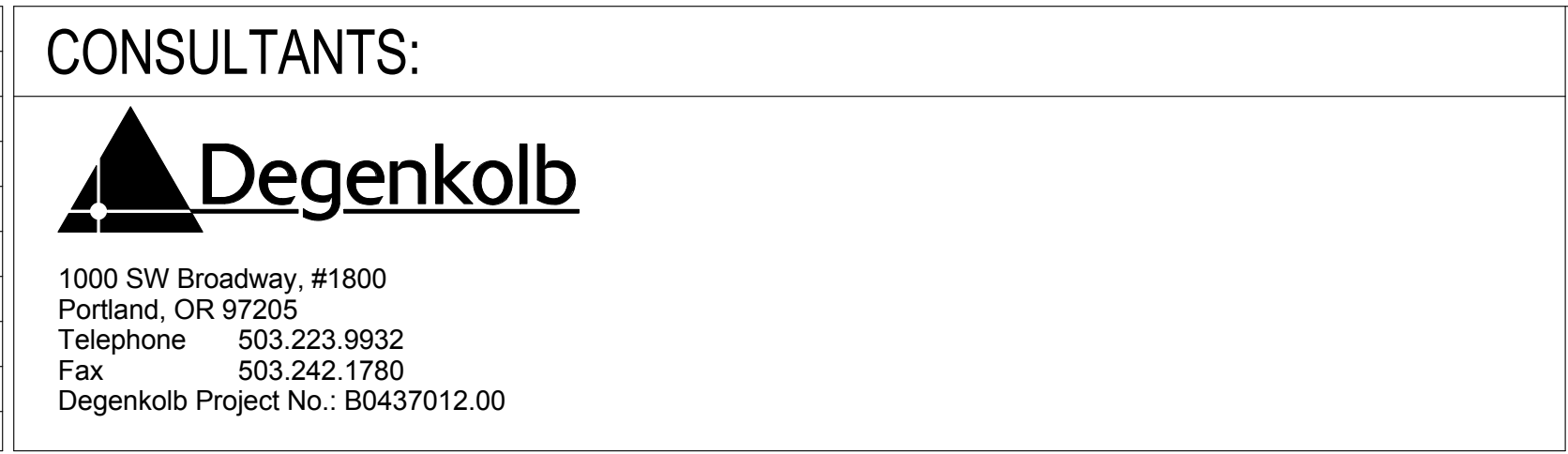
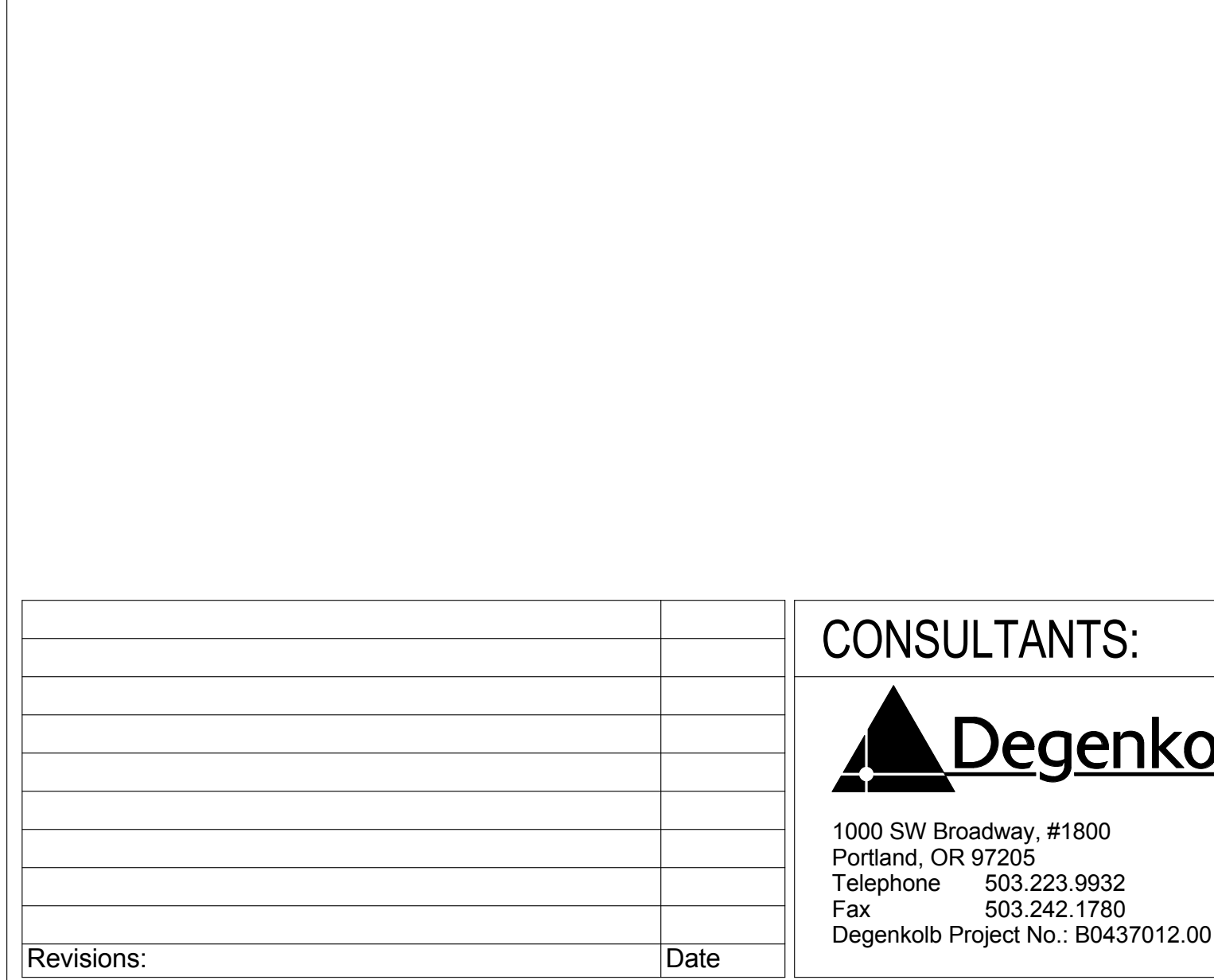
1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

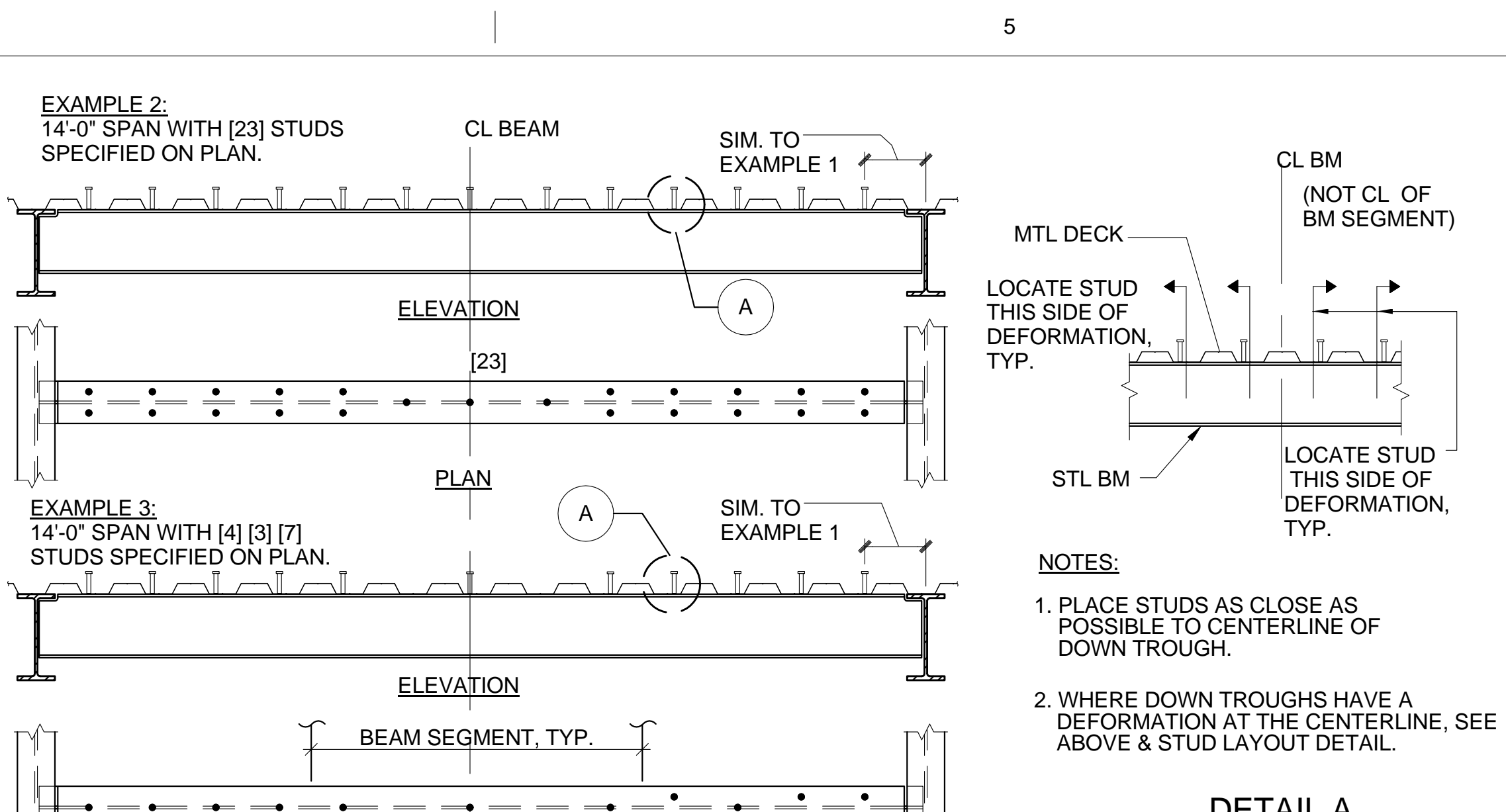
Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Project Number	653-322
Building Number	086
Drawing Number	SF-041

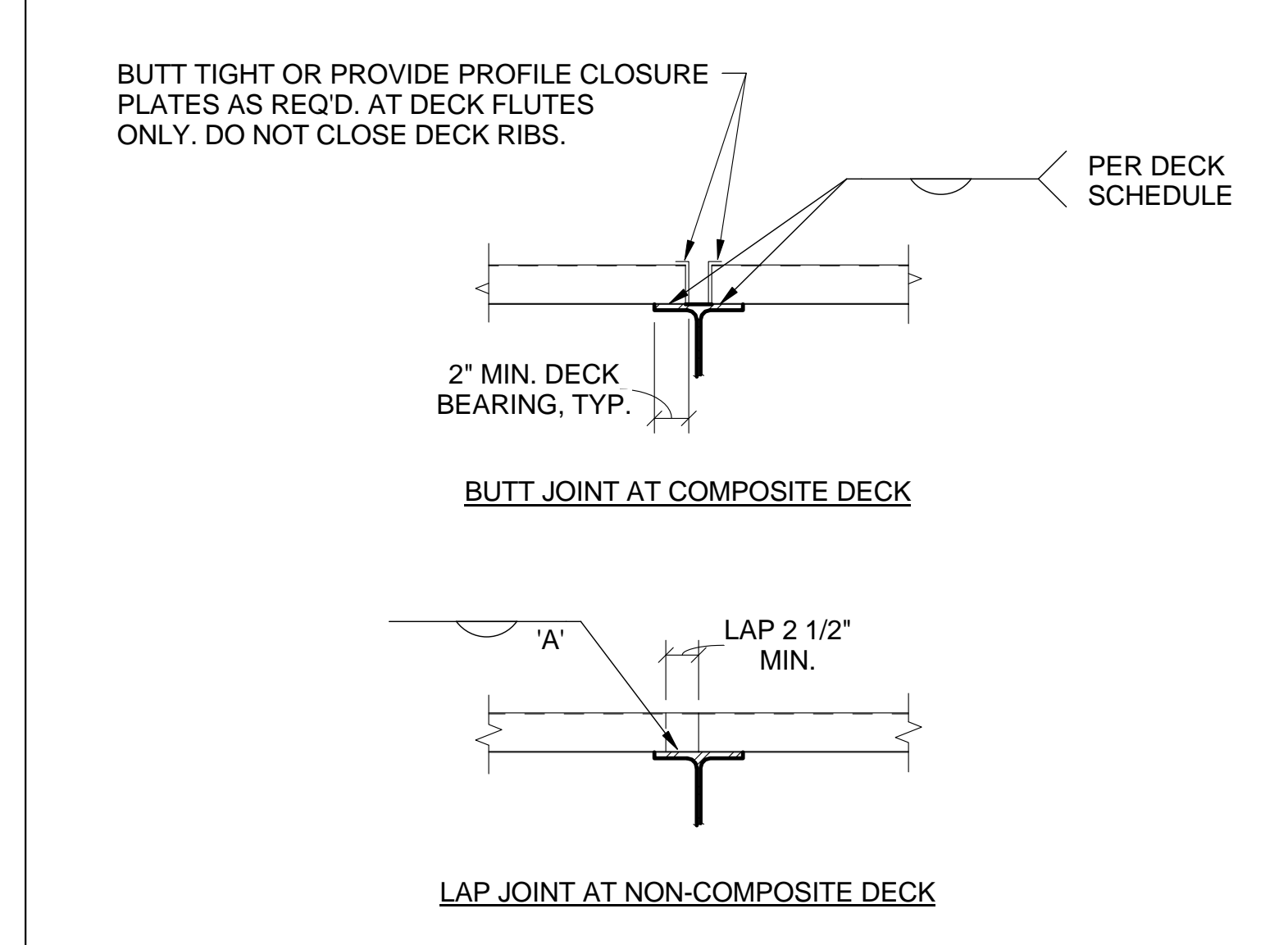
Department of
Veterans Affairs



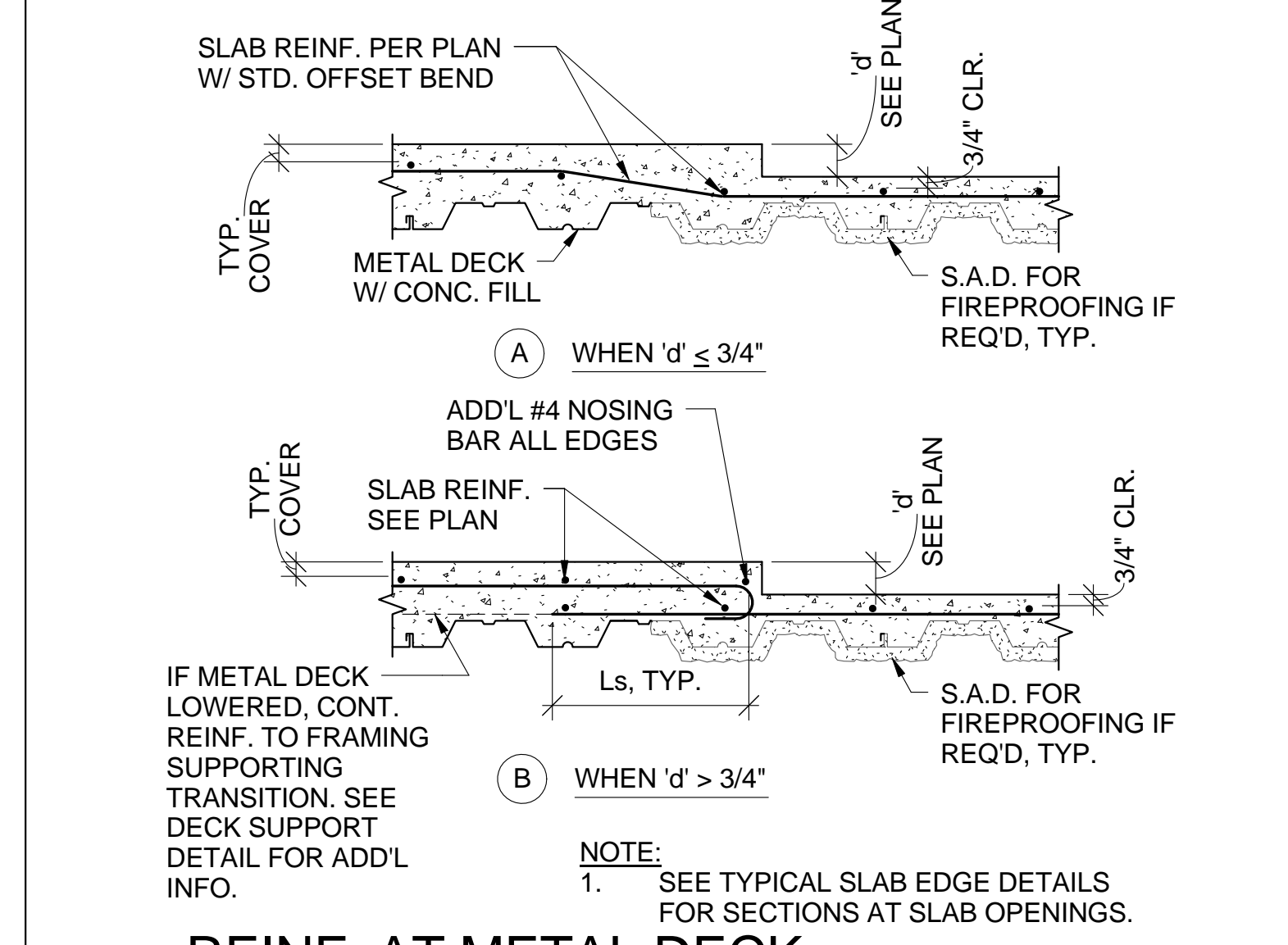




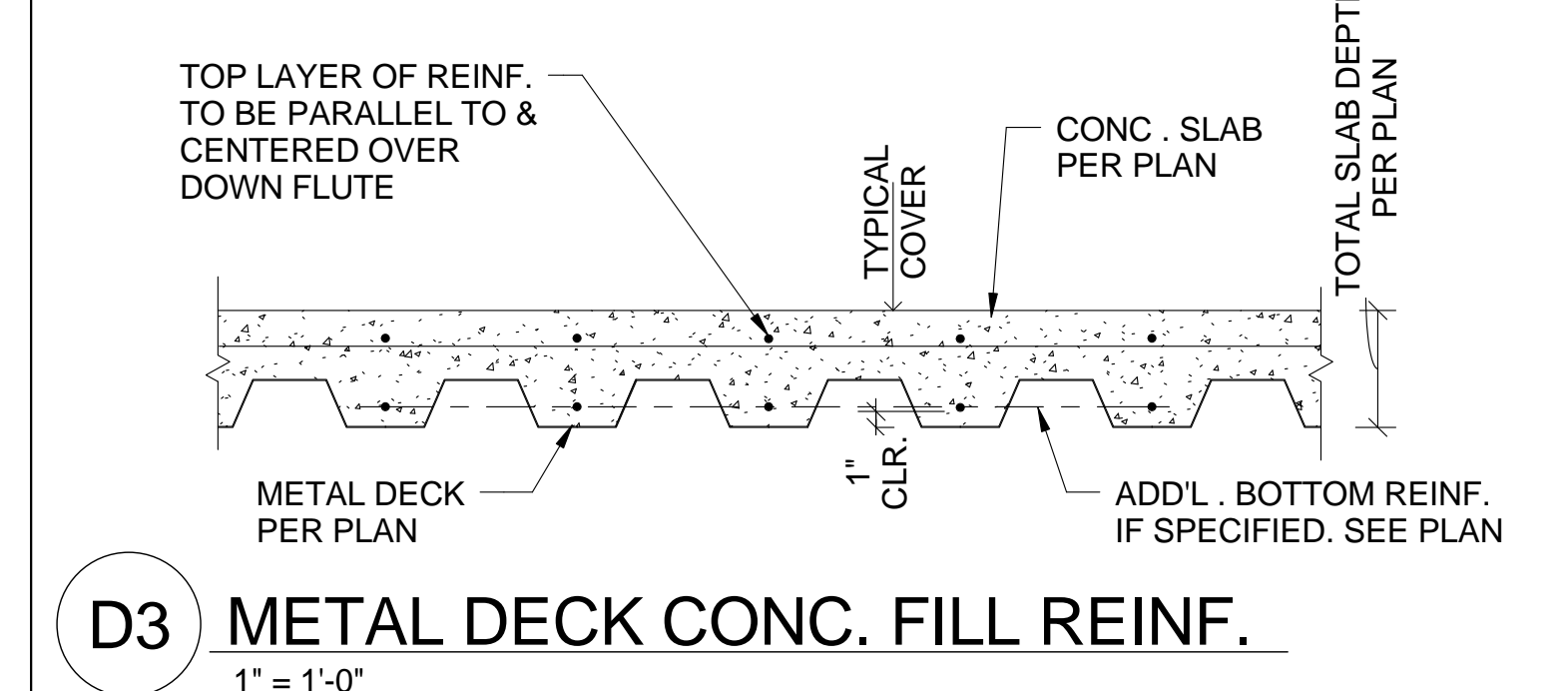
A3 STUD PLACEMENT



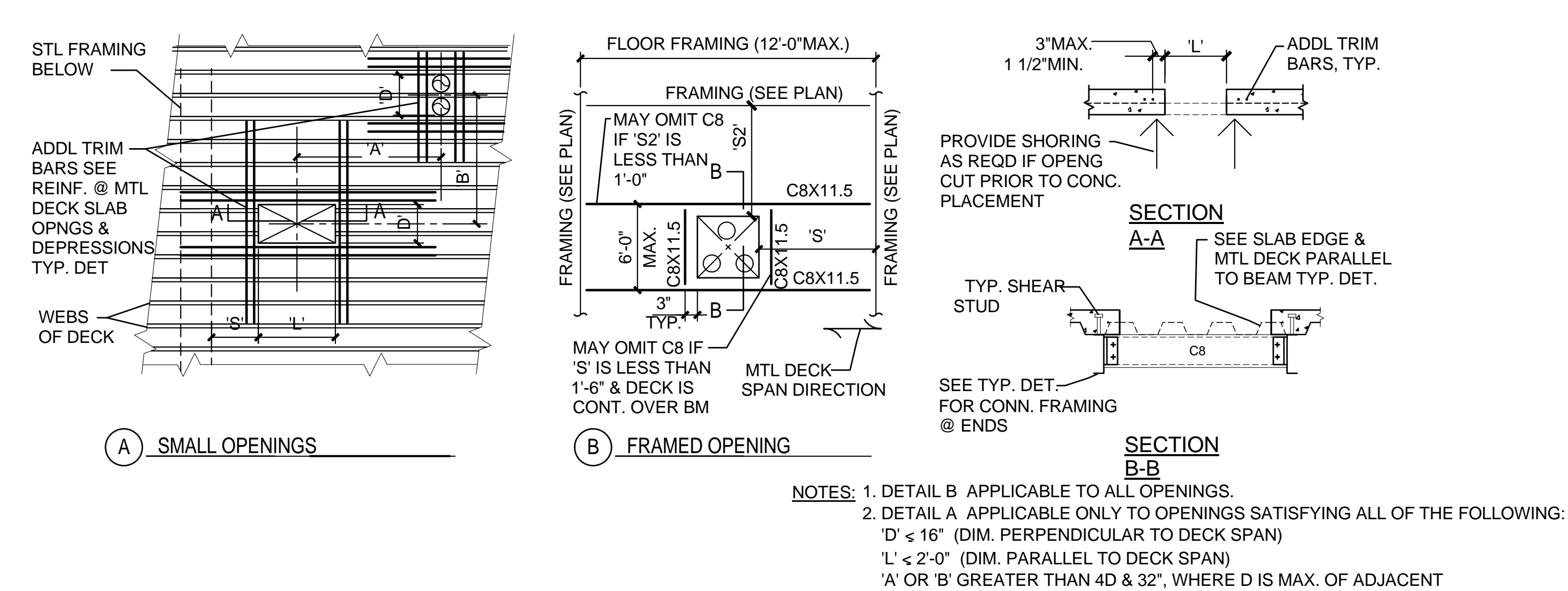
B3 METAL DECK - END JOINTS
CONN.



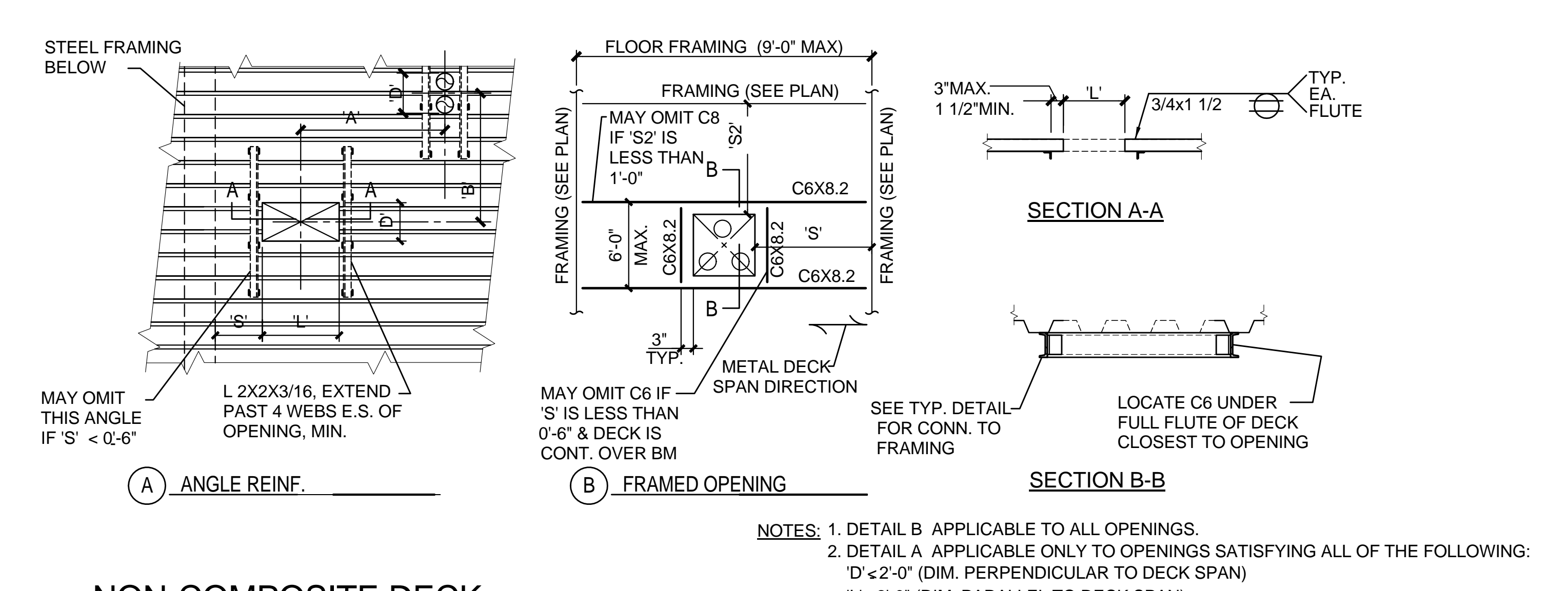
C3 DEPRESSIONS
1" = 1'-0"



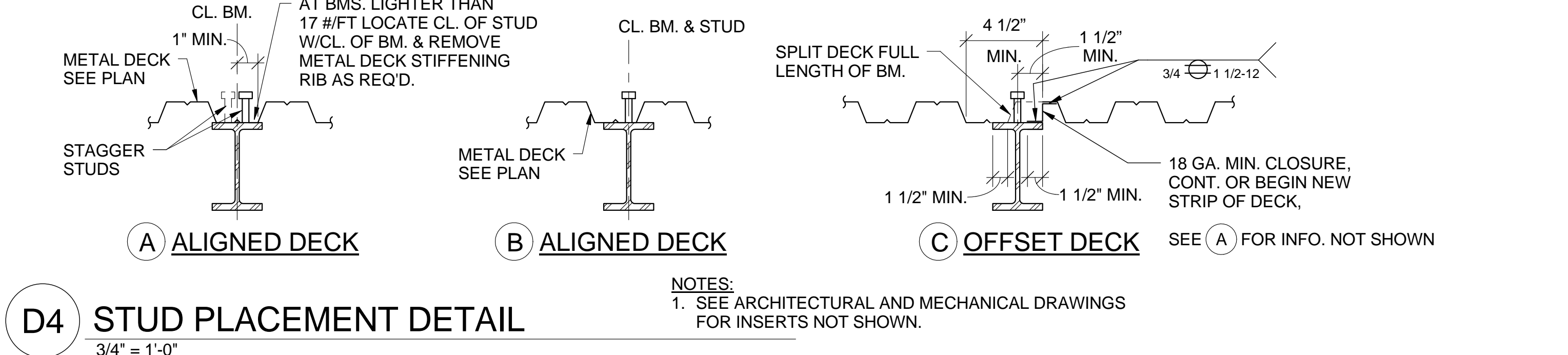
	ARCHITECT/ENGINEERS:



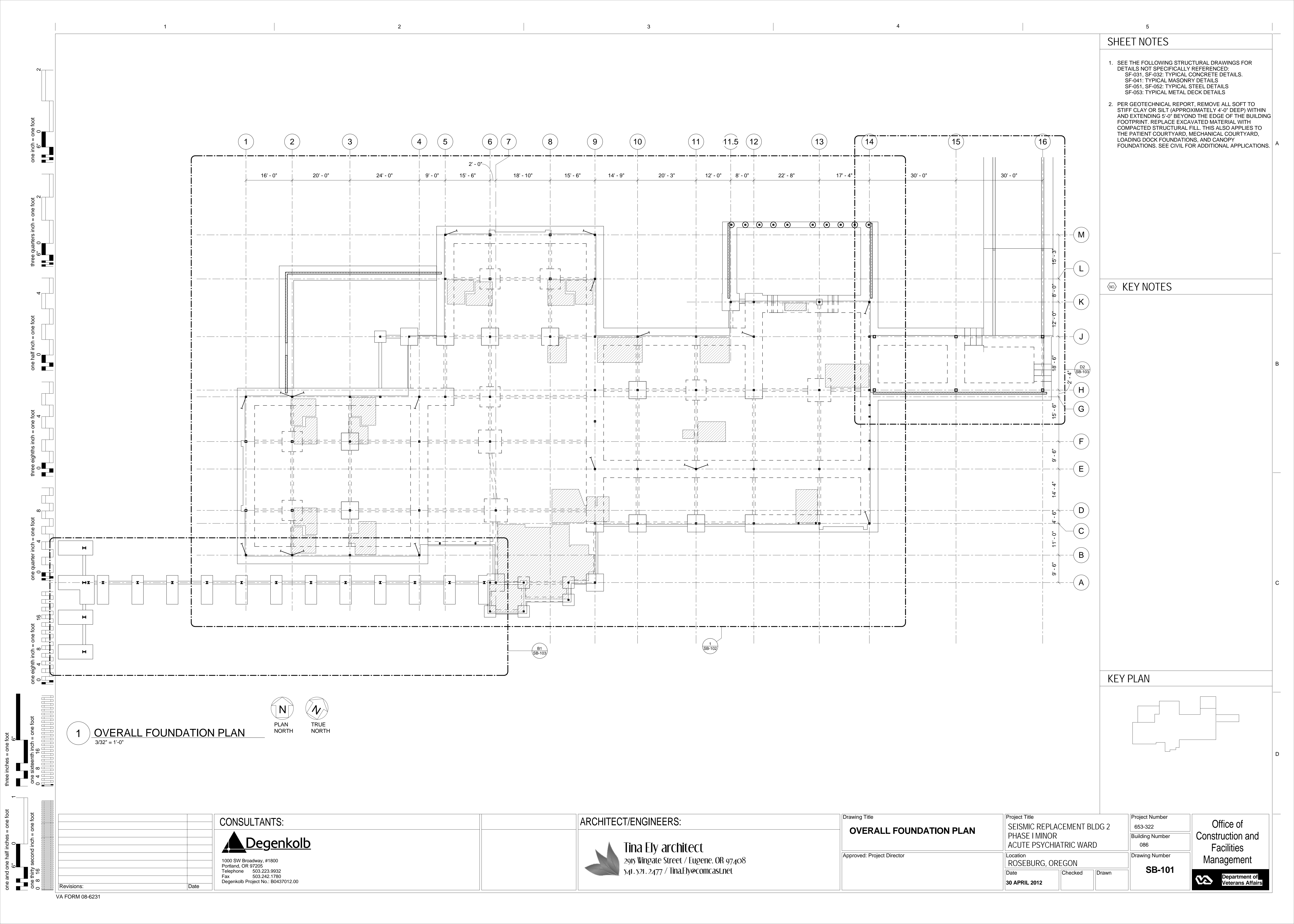
B4 COMMUNITY DEVELOPMENT
REINFORCING



C4 OPENING REINFORCING



	Drawing Title
	TYPICAL METAL DECK DETAIL

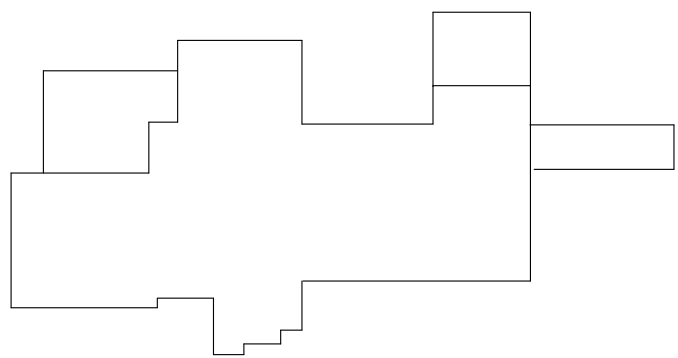


SHEET NOTES

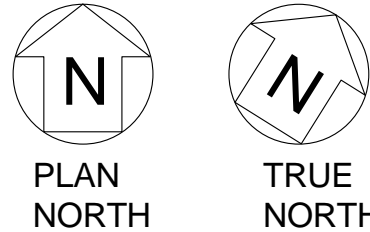
- SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
- PER GEOTECHNICAL REPORT, REMOVE ALL SOFT TO STIFF CLAY OR SILT (APPROXIMATELY 4'-0" DEEP) WITHIN AND EXTENDING 5'-0" BEYOND THE EDGE OF THE BUILDING FOOTPRINT. REPLACE EXCAVATED MATERIAL WITH COMPACTED STRUCTURAL FILL. THIS ALSO APPLIES TO THE PATIENT COURTYARD, MECHANICAL COURTYARD, LOADING DOCK FOUNDATIONS, AND CANOPY FOUNDATIONS. SEE CIVIL FOR ADDITIONAL APPLICATIONS.

KEY NOTES

KEY PLAN



1 OVERALL FOUNDATION PLAN
3/32" = 1'-0"



CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:

Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / tina.ely@comcast.net

Drawing Title
OVERALL FOUNDATION PLAN

Approved: Project Director

Project Title
SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location
ROSEBURG, OREGON

Date
30 APRIL 2012

Checked

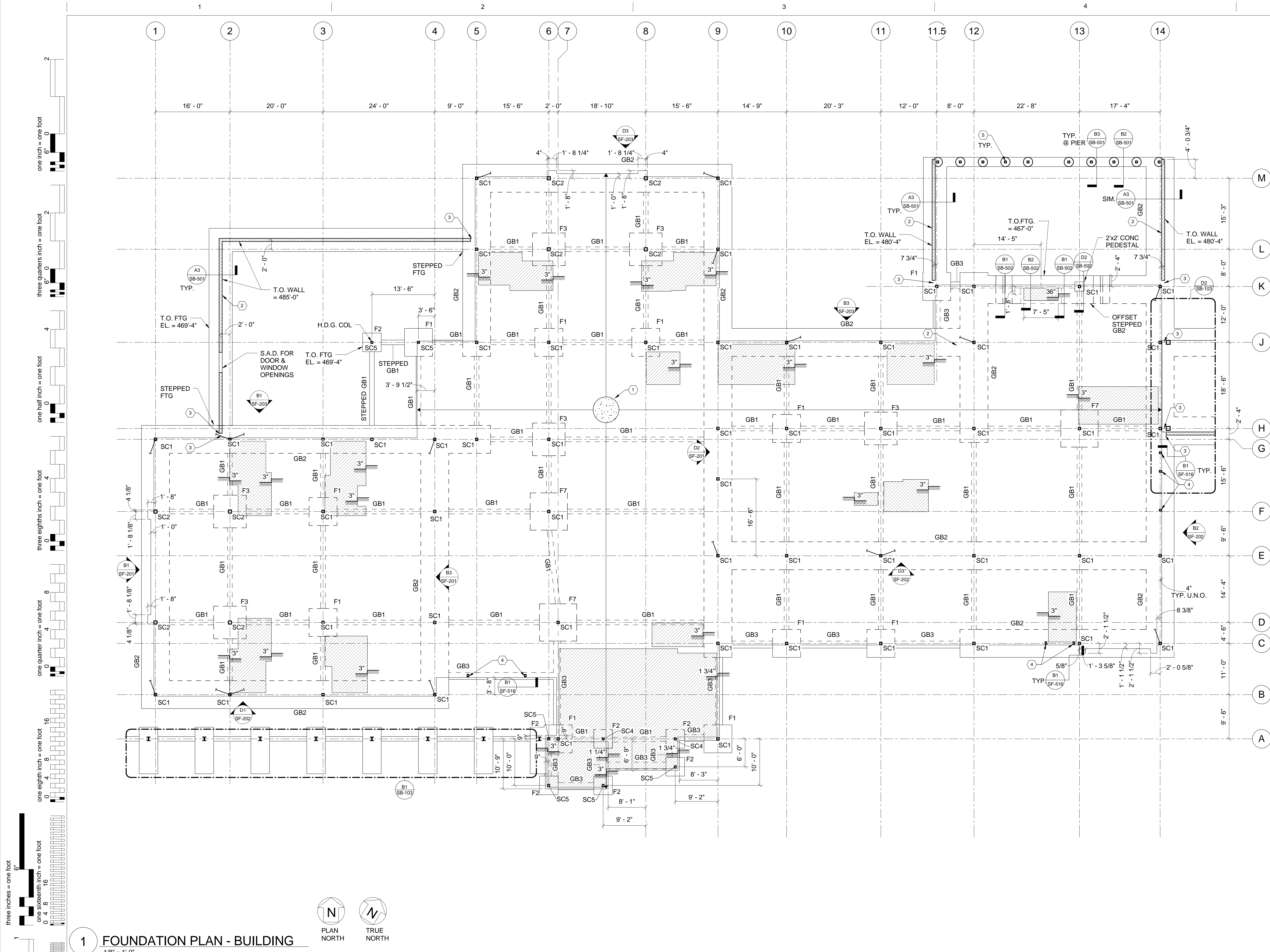
Drawn

Project Number
653-322
Building Number
086

Drawing Number
SB-101

Office of
Construction and
Facilities
Management





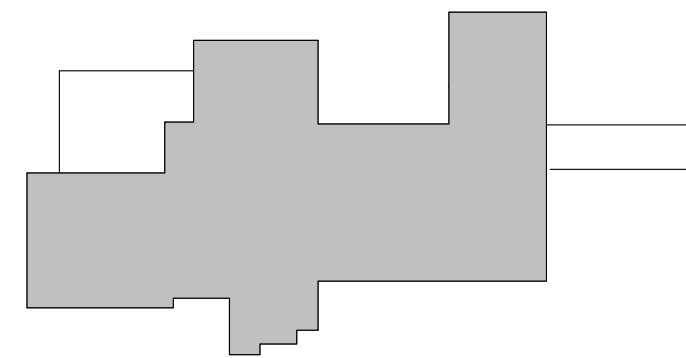
SHEET NOTES

- SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
- FB** DENOTES SPREAD FOOTING PER DETAIL B4/SB-501.
- GB#** DENOTES GRADE BEAM PER DETAIL D3/SB-501.
- PI** DENOTES DRILLED PIER PER DETAIL B3/SB-501.
- SC#** DENOTES STEEL COLUMN PER DETAIL B1/SF-511.
- TOP OF SLAB ELEVATION EQUALS 471'-0", UNLESS NOTED OTHERWISE.
- TOP OF FOOTING ELEVATION EQUALS 470'-0", UNLESS NOTED OTHERWISE.
- TOP OF GRADE BEAM ELEVATION EQUALS 470'-0", UNLESS NOTED OTHERWISE.
- PER GEOTECHNICAL REPORT, REMOVE ALL SOFT TO STIFF CLAY OR SILT (APPROXIMATELY 4'-0" DEEP) WITHIN AND EXTENDING 5'-0" BEYOND THE EDGE OF THE BUILDING FOOTPRINT. REPLACE EXCAVATED MATERIAL WITH COMPACTED STRUCTURAL FILL. THIS ALSO APPLIES TO THE PATIENT COURTYARD, MECHANICAL COURTYARD, LOADING DOCK FOUNDATIONS, CMU WALL FOUNDATIONS, AND CANOPY FOUNDATIONS. SEE CIVIL FOR ADDITIONAL APPLICATIONS.

KEY NOTES

- 5" CONCRETE SLAB ON GRADE W/ #4 @ 16" O.C. E.W.
- 8" CMU WALL, SEE TYP. DETAILS ON SF-041. SEE ARCHITECTURAL DRAWINGS FOR WINDOW AND DOOR OPENINGS.
- 4" SEISMIC GAP.
- HSS6x4x1/4, SEE DETAIL C3/SF-516 FOR CONNECTION AT TOP, U.O.N.
- SC5 & P1, TYP. E.S. OF DOOR, 6'-0" O.C. MAX. CONNECT HSS TO PIER PER DETAIL C2/SF-516. T.O.S. POST = 480'-4". H.D.G. POST. INSTALL H.D.G. CAP PLATE ON POST PER DETAIL B5/SF-052. S.A.D. FOR CHAIN LINK FENCE AND GATE BETWEEN POSTS.

KEY PLAN



1 FOUNDATION PLAN - BUILDING

1/8" = 1'-0"

CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:



Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Drawing Title

FOUNDATION PLAN

Approved: Project Director

Project Title

SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location
ROSEBURG, OREGON

Date
30 APRIL 2012

Checked

Drawn

Project Number

653-322

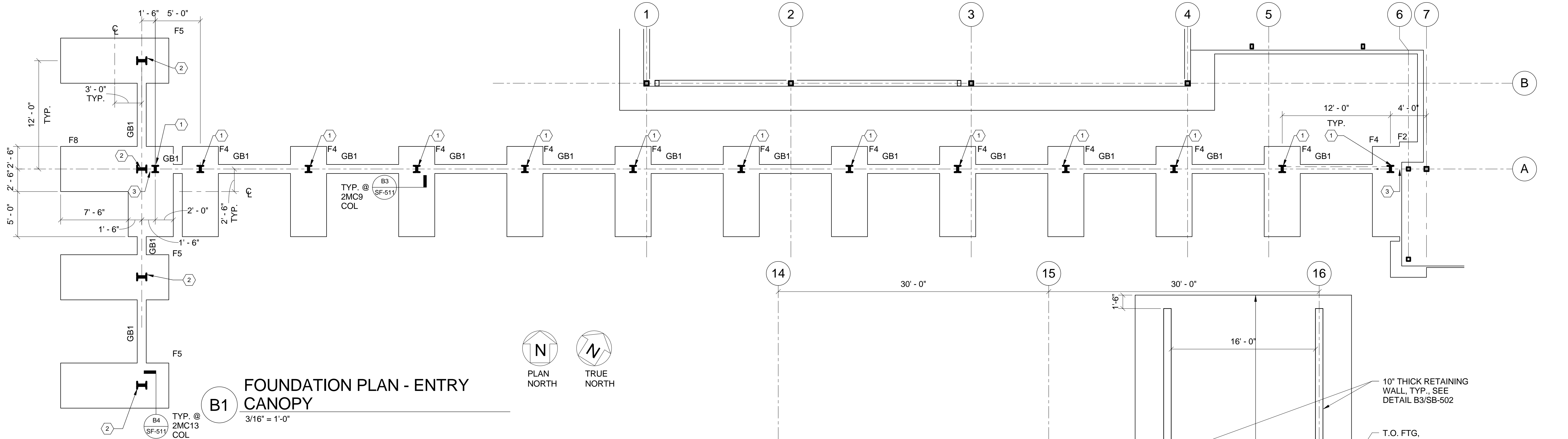
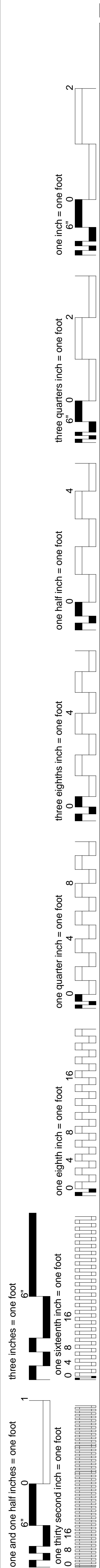
Building Number
086

Drawing Number

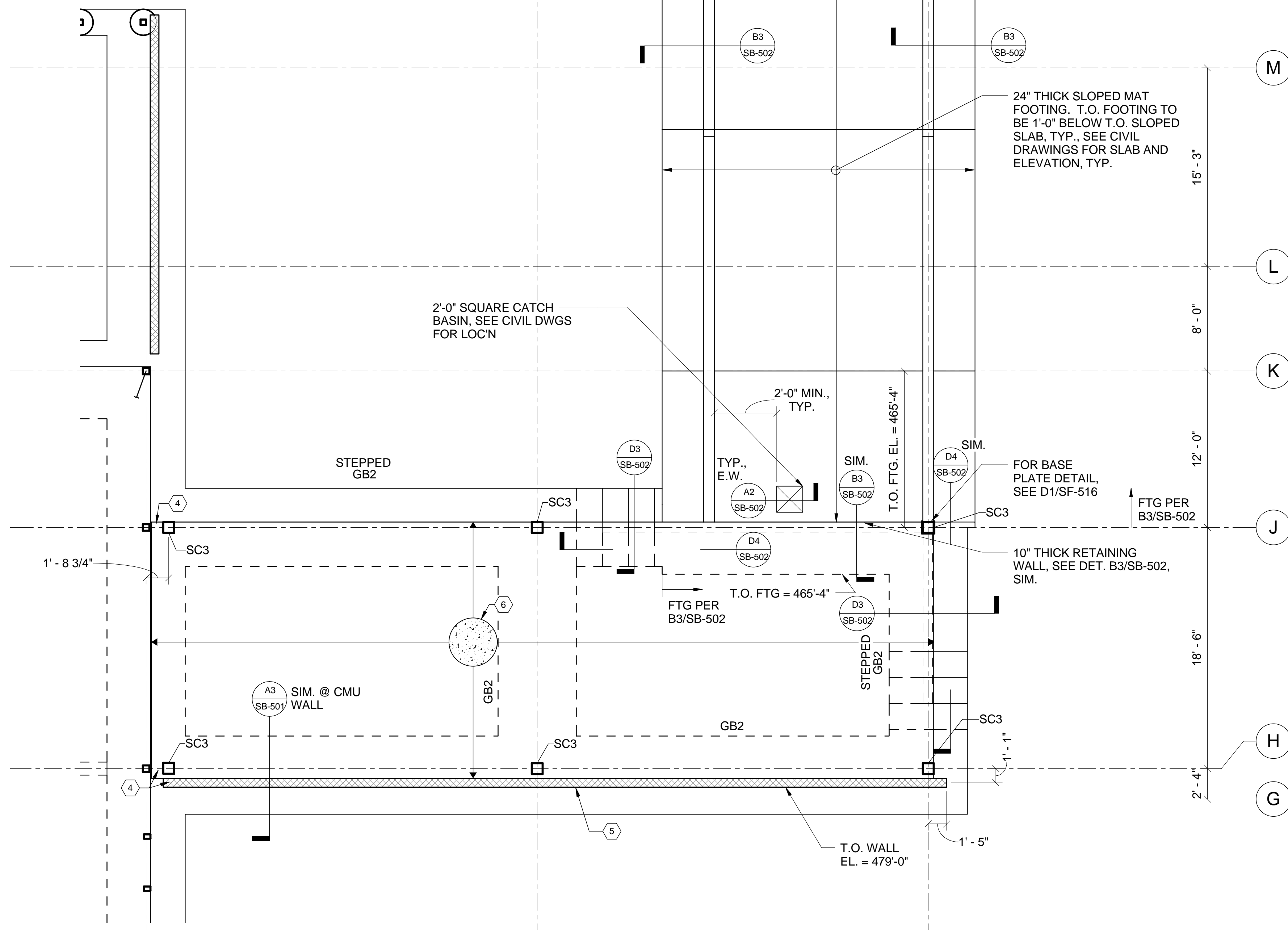
SB-102

Office of
Construction and
Facilities
Management





B1 FOUNDATION PLAN - ENTRY CANOPY
3/16" = 1'-0"



D2 FOUNDATION PLAN - LOADING DOCK CANOPY
3/16" = 1'-0"

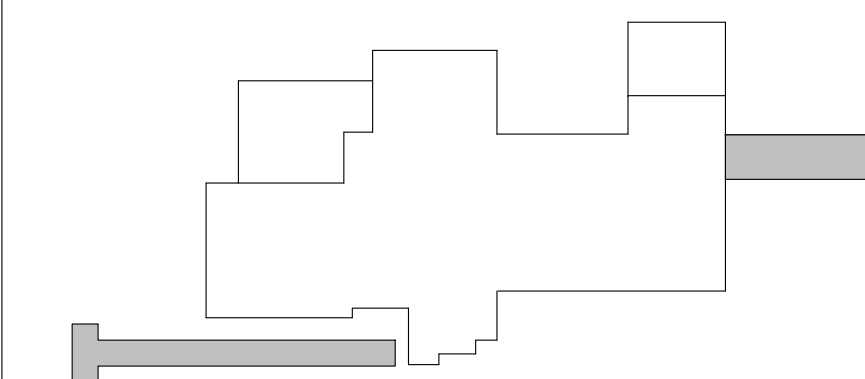
SHEET NOTES

- SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
- F#** DENOTES SPREAD FOOTING PER DETAIL B4/SB-501.
- GB#** DENOTES GRADE BEAM PER DETAIL D3/SB-501.
- PI#** DENOTES DRILLED PIER PER DETAIL B3/SB-501.
- SC#** DENOTES STEEL COLUMN PER DETAIL B1/SF-511.
- TOP OF SLAB ELEVATION EQUALS 471'-0", UNLESS NOTED OTHERWISE.
- TOP OF FOOTING ELEVATION EQUALS 470'-0", UNLESS NOTED OTHERWISE.
- TOP OF GRADE BEAM ELEVATION EQUALS 470'-0", UNLESS NOTED OTHERWISE.
- PER GEOTECHNICAL REPORT, REMOVE ALL SOFT TO STIFF CLAY OR SILT (APPROXIMATELY 4'-0" DEEP) WITHIN AND EXTENDING 5'-0" BEYOND THE EDGE OF THE BUILDING FOOTPRINT. REPLACE EXCAVATED MATERIAL WITH COMPACTED STRUCTURAL FILL. THIS ALSO APPLIES TO THE PATIENT COURTYARD, MECHANICAL COURTYARD, LOADING DOCK FOUNDATIONS, CMU WALL FOUNDATIONS, AND CANOPY FOUNDATIONS. SEE CIVIL FOR ADDITIONAL APPLICATIONS.

KEY NOTES

- 2MC9x23.9 COLUMN W/ 1 1/2" GAP BET. CHANNELS. CHANNELS TO BE STITCHED TOGETHER @ 3'-0" O.C. VERTICALLY PER DETAIL D5/SF-515, SIM.
- 2MC13x40 COLUMN W/ 1 1/2" GAP BET. CHANNELS. CHANNELS TO BE STITCHED TOGETHER @ 4'-0" O.C. VERTICALLY PER DETAIL D5/SF-515.
- 6" SEISMIC GAP
- 4" SEISMIC GAP
- 8" CMU WALL, SEE TYP. DETAILS ON SF-041. SEE ARCHITECTURAL DRAWINGS FOR WINDOW AND DOOR OPENINGS.
- 5' CONCRETE SLAB ON GRADE W/ #4 @ 16" O.C., E.W.

KEY PLAN



CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:

Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / tina@comcast.net

Drawing Title

FOUNDATION PLANS

Approved: Project Director

Project Title

SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location
ROSEBURG, OREGON

Date
30 APRIL 2012

Checked

Drawn

Project Number

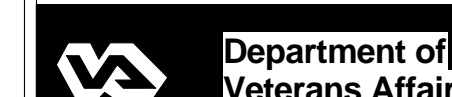
653-322

Building Number
086


Drawing Number

SB-103

Office of
Construction and
Facilities
Management

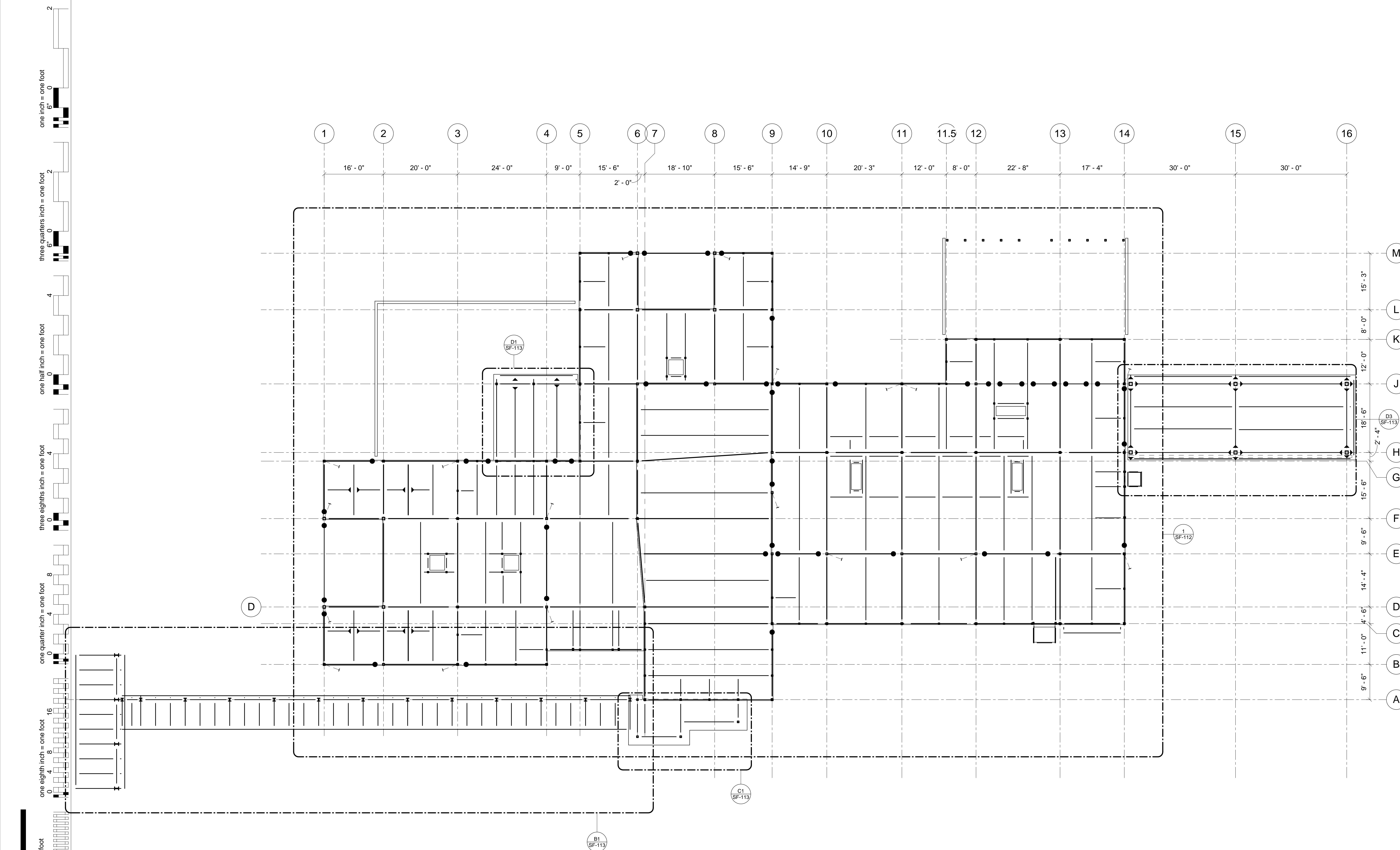
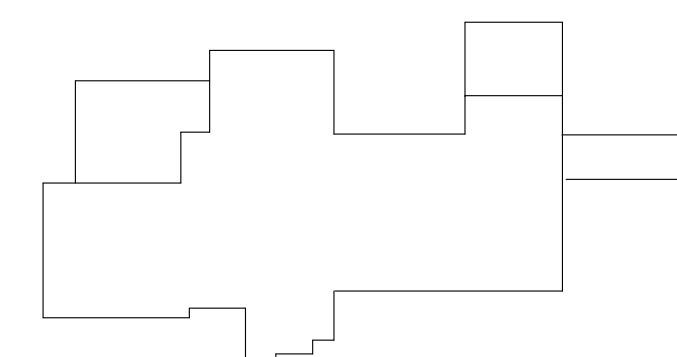


SHEET NOTES

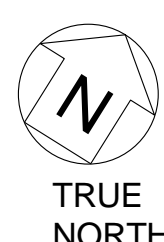
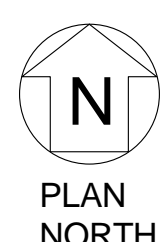
1. SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
 SF-031, SF-032: TYPICAL CONCRETE DETAILS.
 SF-041: TYPICAL MASONRY DETAILS
 SF-051, SF-052: TYPICAL STEEL DETAILS
 SF-053: TYPICAL METAL DECK DETAILS
2. **D#** DENOTES DECK PER DETAIL A2/SF-511.
3. T.O. STEEL ELEVATION EQUALS 487'-0", U.O.N.
4.  INDICATES A COLLECTOR CONNECTION. SEE DETAILS C4/SF-051 AND A5/ AND B4/SF-052, U.O.N.
5. EDGE OF SLAB IS 3" OUTBOARD OF CENTERLINE OF WF PERIMETER BEAMS.

KEY NOTES

KEY PLAN



(1

OVERALL FRAMING PLAN -
ROOF
$$\frac{3}{32}'' = 1'-0''$$


CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:



Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

	Drawing Title
--	---------------

OVERALL ROOF FRAMING PLAN

Approved: Project Director

Project Title

SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location	ROSEBURG, OREGON
----------	------------------

Date
30 APRIL 2012

☐ Checke☐ Drawn

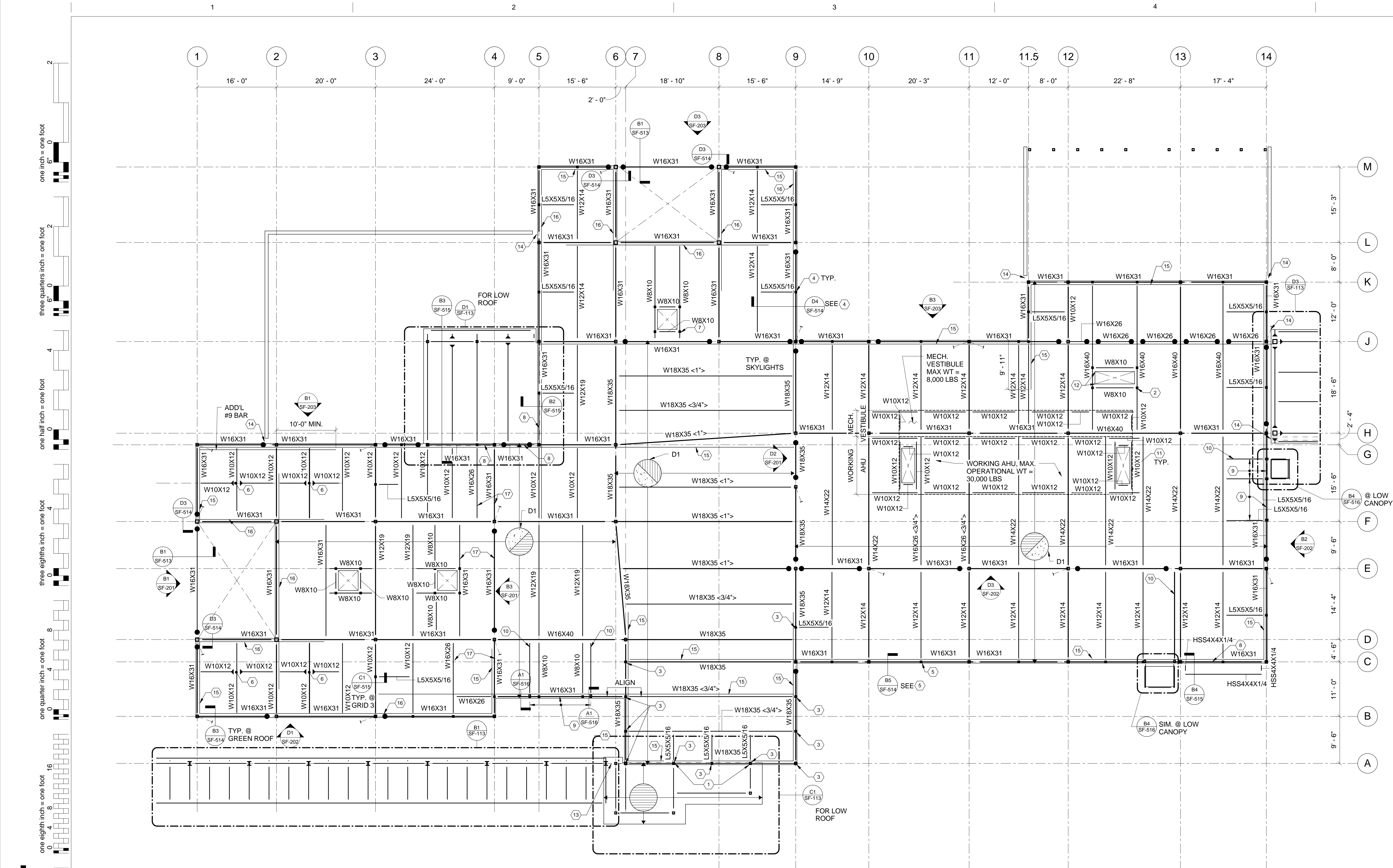
Project Number	653-322
----------------	---------

Building Number	086
-----------------	-----

	Drawing Number
--	----------------

Office of
Construction and
Facilities
Management



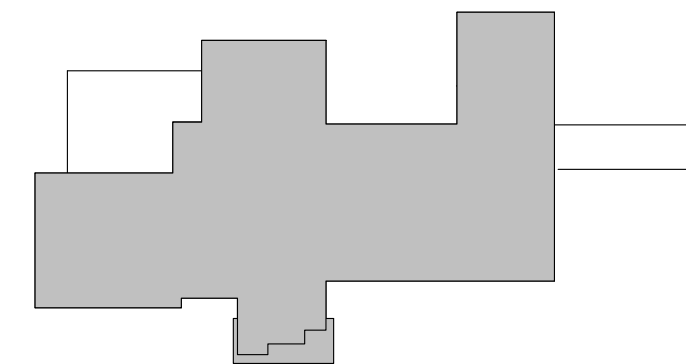


- SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
- D# DENOTES DECK PER DETAIL A2/SF-511.
- T.O. STEEL ELEVATION EQUALS 487'-0", U.O.N.
- INDICATES A COLLECTOR CONNECTION. SEE DETAILS C4/SF-051 AND A5/ AND B4/SF-052, U.O.N.
- EDGE OF SLAB IS 3" OUTBOARD OF CENTERLINE OF WF PERIMETER BEAMS.

KEY NOTES

- PROVIDE SLIP COL. CONN. AT TOP PER C3/SF-516.
- HSS4x4x1/4 POST, TYP. @ STAIR HATCH CORNERS. CONNECTION AND PERIMETER HATCH CURB BEAM PER D4/SF-514, S.A.D. FOR T.O.S.
- HSS5x4x5/16 POST, TYP. AT PARAPET SUPPORT. SEE KEYNOTES 4 THRU 5 FOR SIMILAR CONN. DETAILS. ALIGN W/ HSS5x5 COL. BELOW BM. IF APPLICABLE.
- HSS4x4x1/4 POST, TYP. AT PARAPET SUPPORT & L5x5x5/16 BRACE, SEE D4/SF-514 FOR CONN. DETAILS.
- HSS4x4x1/4 POST, TYP. (HSS6x4x1/4 ON GRID 3) AT PARAPET SUPPORT & BM INTERSECTION, SEE B5/SF-514 FOR CONN. DETAILS.
- TIE-OFF LOCN., S.A.D. FOR EXACT LOC. SEE D2/SF-051.
- HSS4x4x1/4 POST WITH CAP PLATE AT SKYLIGHT CURB CORNERS.
- HSS BM FOR BRICK SUPPORT AND WF OR HSS BM FOR LOW ROOF BELOW. SEE SECTION CUTS ON PLAN. CONNECT BRICK SUPPORT BEAM TO HSS COLS PER C3/SF-515.
- HSS8x8x5/16 BM FOR BRICK SUPPORT BELOW. SEE ARCHITECTURAL DRAWINGS FOR TOP OF STEEL. CONNECT BEAM TO HSS COLS PER C2/SF-515. BRICK LEDGER ANGLE SUPPORT CONN. PER A1/SF-515.
- ALIGN L5x5x5/16 BRACE W/ HSS COL. BELOW. ATTACH BRACE TO FRAMING PER D4/SF-514 (SIMILAR).
- PROVIDE WF BM FRAMING UNDER THE "WORKING AHU" & MECH. VESTIBULE EXTENTS OF THE MECHANICAL UNIT CURB. PROVIDE BMS UNDER PERIMETER & INTERIOR EXTENTS OF CURB. SEE ARCH. DWGS, MECH. DWGS, AND CURB MANUFACTURER'S CUT SHEET FOR EXACT LOC(S).
- ALLOWANCE FOR AN UN-REDUCED LOADING FOR EACH STRINGER (ASSUMING [2] STRINGERS) OF:
DEAD LOAD = 800 LBS
LIVE LOAD = 1,500 LBS
- 6" SEISMIC GAP
- 4" SEISMIC GAP
- ADDL (2) #9 BARS @ SLAB EDGE. EXTENTS OF ADDL BARS TO BE MAX OF 10'-0" PAST PERP. EDGE OF BLDG OR TO GRIDLINE SHOWN ON PLAN.
- ADDL (3) #9 BARS @ SLAB EDGE, SEE KEYNOTE 15 FOR EXTENTS.
- BEAM SIZED FOR A POST-COMPOSITE DEFLECTION LIMIT OF 1/4" FOR THE PARTITION BELOW.

KEY PLAN



FRAMING PLAN - BUILDING ROOF

1/8" = 1'-0"

CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:

Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Drawing Title

ROOF FRAMING PLAN

Approved: Project Director

Project Title

SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location

ROSEBURG, OREGON

Date

30 APRIL 2012

Checked

Drawn

Project Number

653-322

Building Number

086

Drawing Number

SF-112

Office of
Construction and
Facilities
Management

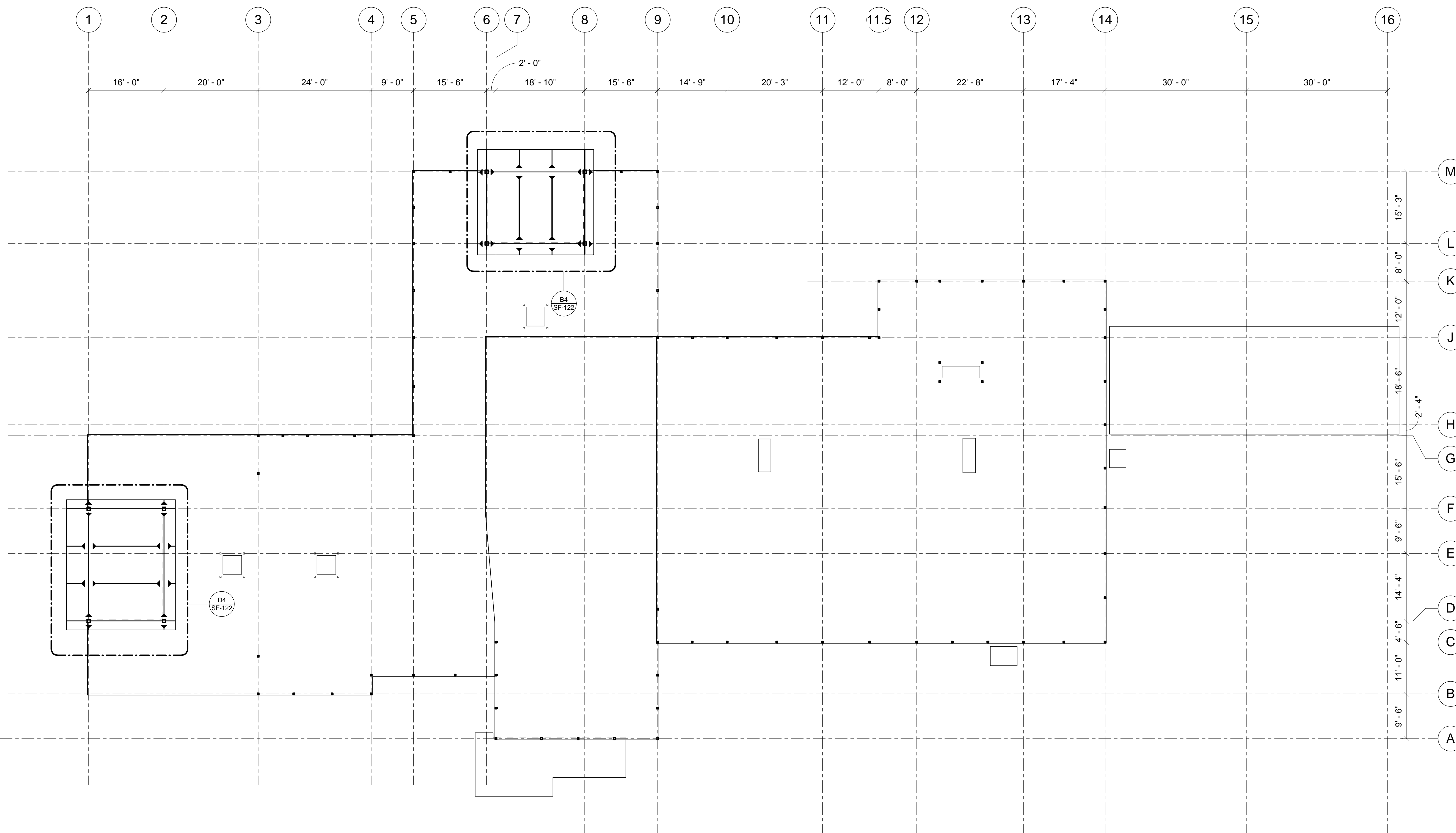
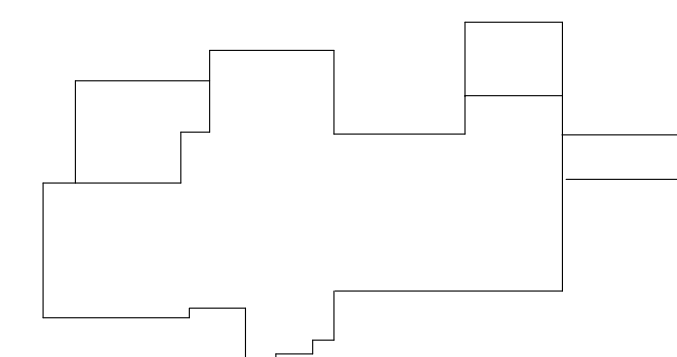


SHEET NOTES

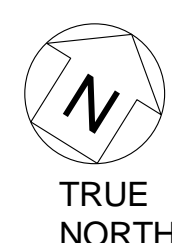
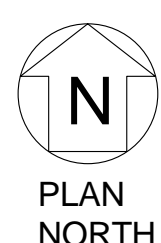
1. SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS

KEY NOTES

KEY PLAN



OVERALL FRAMING PLAN -
CLERESTORY ROOF

$$3/32'' = 1'-0''$$


CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:



Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Drawing Title

**OVERALL CLERESTORY FRAMING
PLAN**

Approved: Project Director

Project Title
SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location	ROSEBURG, OREGON
----------	------------------

Date
30 APRIL 2012

☐ Check

Drawn

Project Number	653-322
Building Number	086

Drawing Number
SF-12

Office of
Construction and
Facilities
Management



1. SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
2. D# DENOTES DECK PER DETAIL A2/SF-511



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

 **Tina Ely architect**
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Approved: Project Director

	Location
--	----------

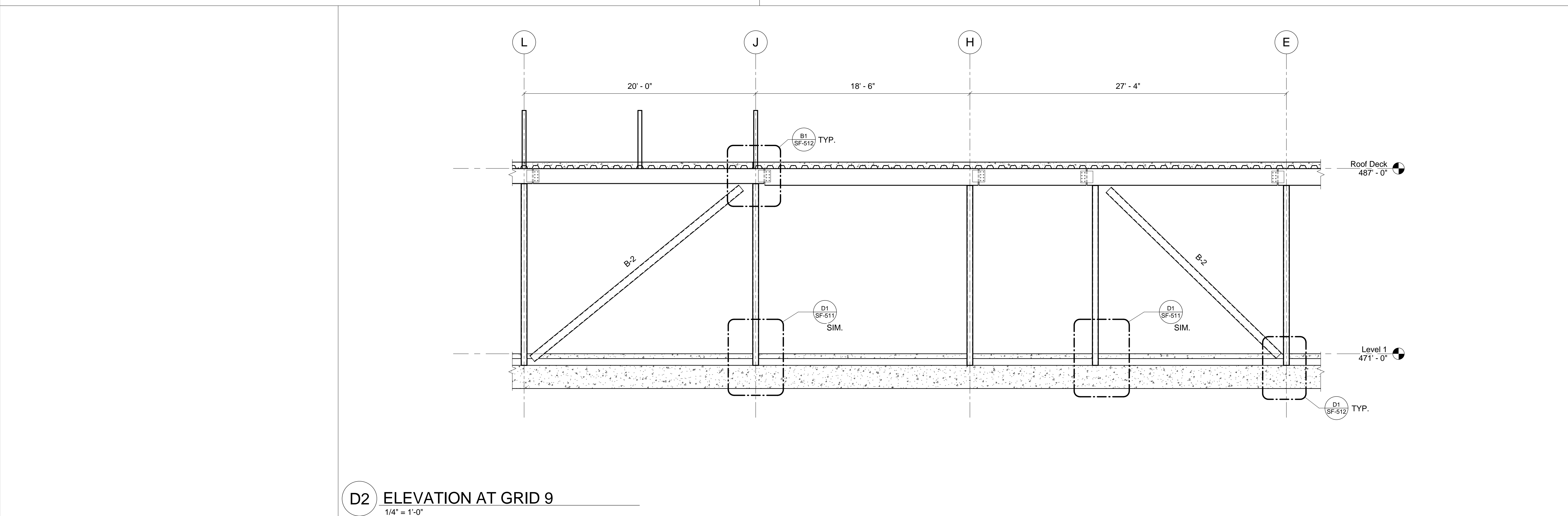
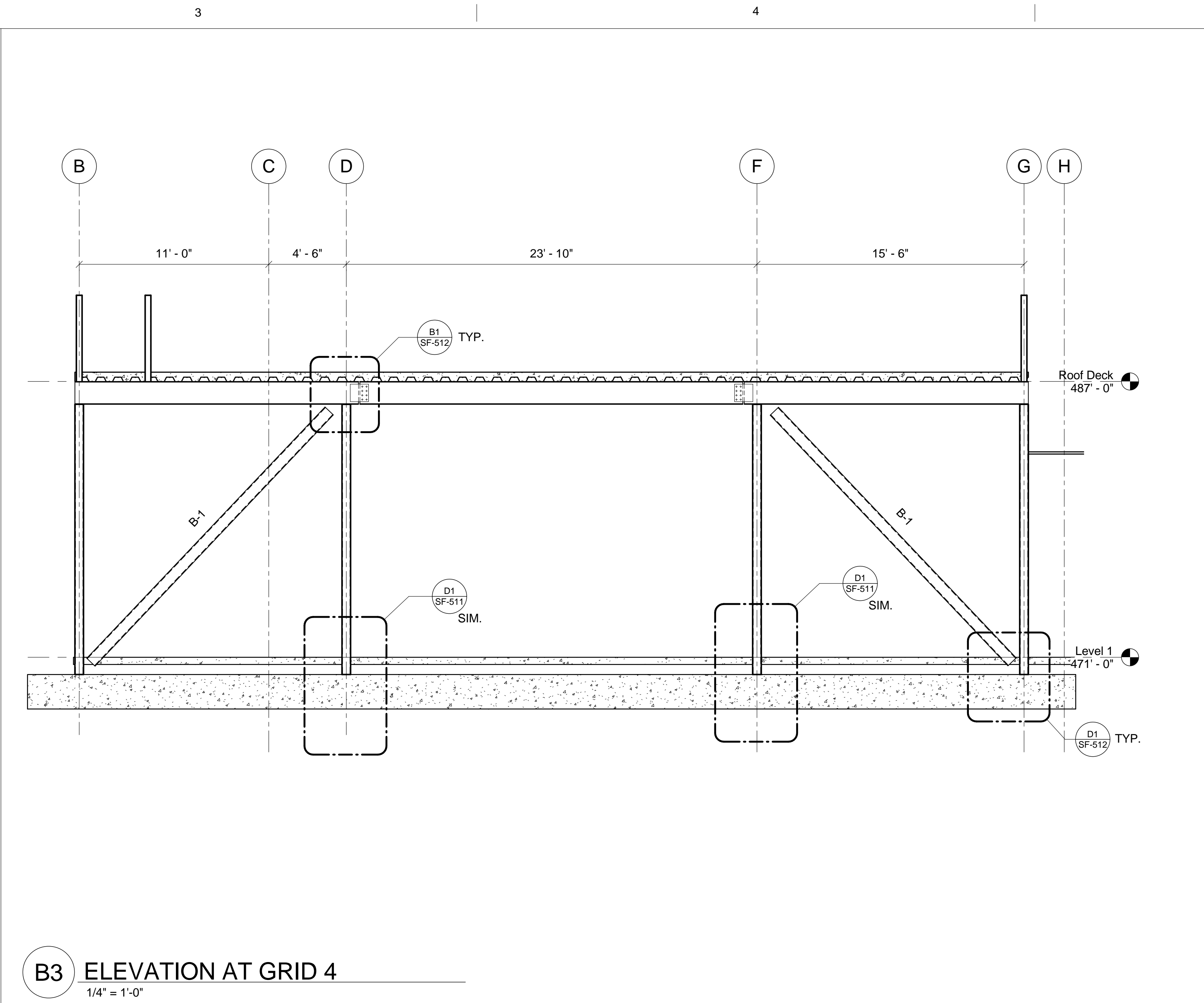
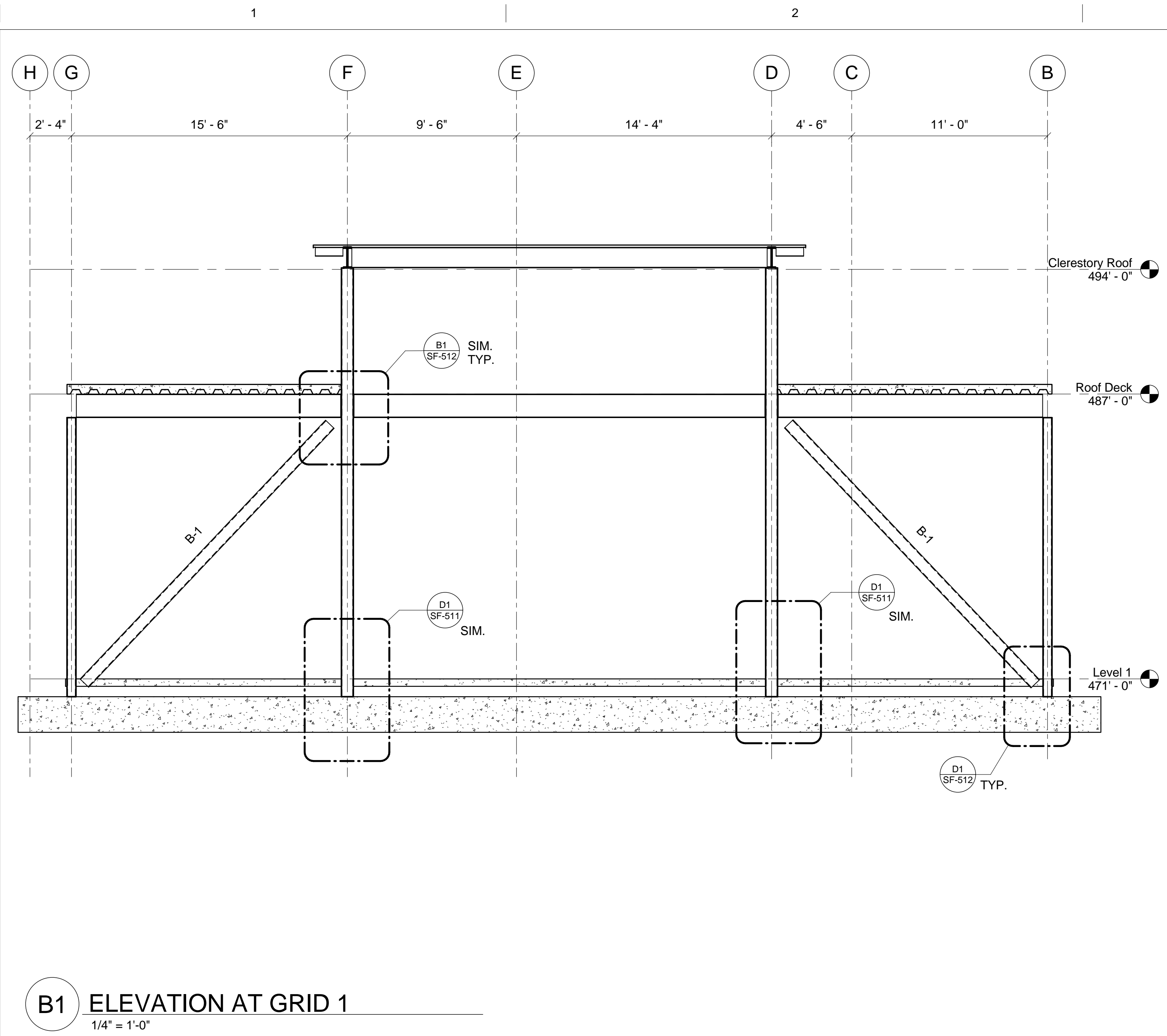
ROSEBURG,

REGON

Drawing Number

SF-122

Department of
Veterans Affairs



SHEET NOTES

1. SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS

2. **B-#** DENOTES BUILDING-RESTRAINED BRACE PER SCHEDULE B4/SF-512.

KEY NOTES

Project Title		Project Number	
SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD		653-322	
Location		Building Number	
ROSEBURG, OREGON		086	
Date	Checked	Drawn	
30 APRIL 2012			

Drawing Title		Project Title	
BRACED FRAME ELEVATIONS		SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD	
Approved: Project Director		Location	
		ROSEBURG, OREGON	
		Date	Checked
		30 APRIL 2012	

CONSULTANTS:		ARCHITECT/ENGINEERS:	
<div><div><div></div><div>Degenkolb</div></div><div>1000 SW Broadway, #1800 Portland, OR 97205 Telephone 503.223.9932 Fax 503.242.1780 Degenkolb Project No.: B0437012.00</div></div>		<div><div><div></div><div>Tina Ely architect</div></div><div>2915 Wingate Street / Eugene, OR 97408 541.521.2477 / Tina.Ely@comcast.net</div></div>	

Office of Construction and Facilities Management	
Department of Veterans Affairs	



CONSULTANTS:

 **Degenkolb**

1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00



Tina Ely architect
 2915 Wingate Street / Eugene, OR 97408
 541.521.2477 / Tina.Ely@comcast.net

Office of
Construction and
Facilities
Management

1. SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
2. **B-#** DENOTES BUILDING-RESTRAINED BRACE PER SCHEDULE B4/SF-512.

NO. KEY NOTES

three inches = one foot
0 6"

one and one half inches = one foot
0 6"

one thirty second inch = one foot
0 8 16

one sixteenth inch = one foot
0 4 8 16

one eighth inch = one foot
0 4 8 16

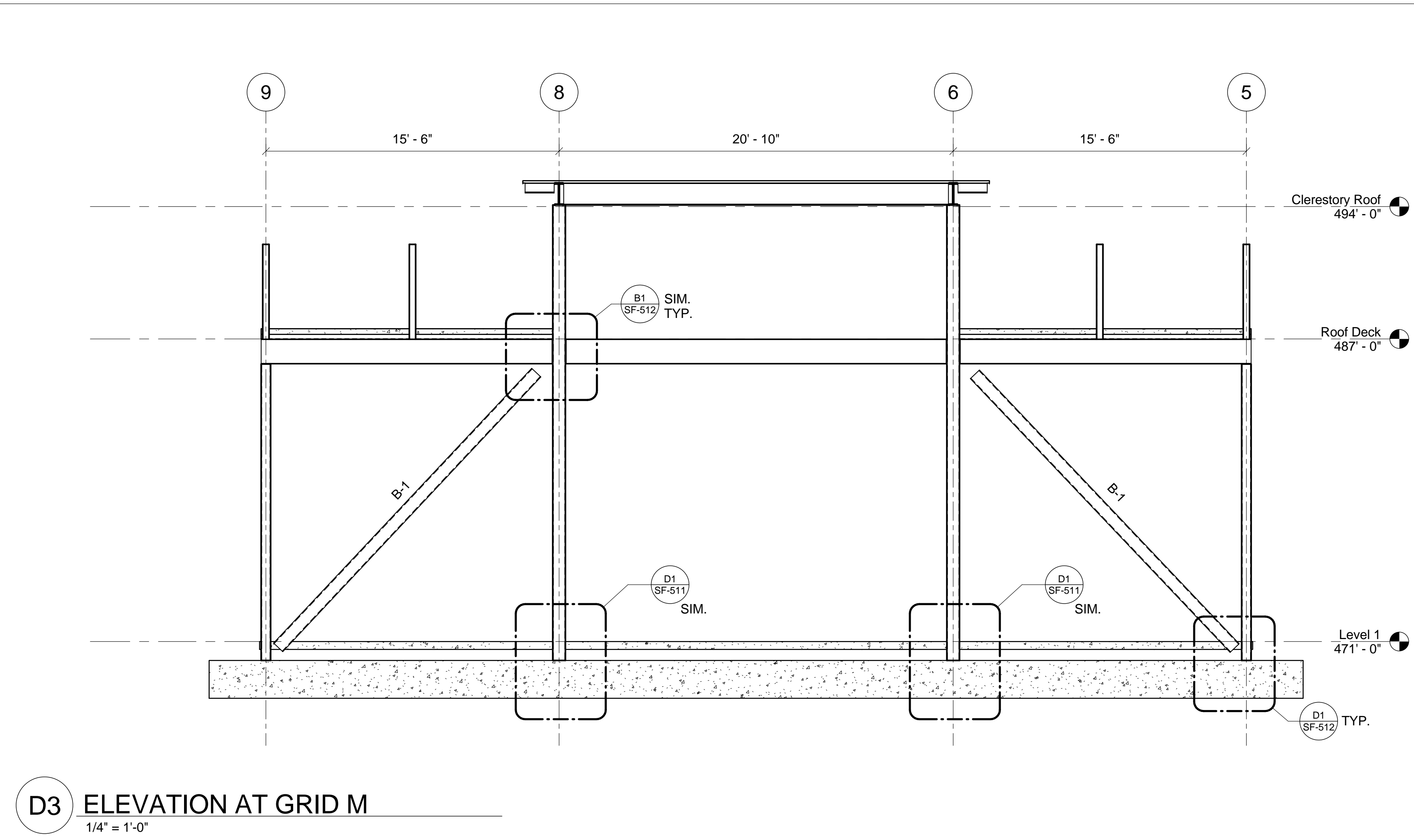
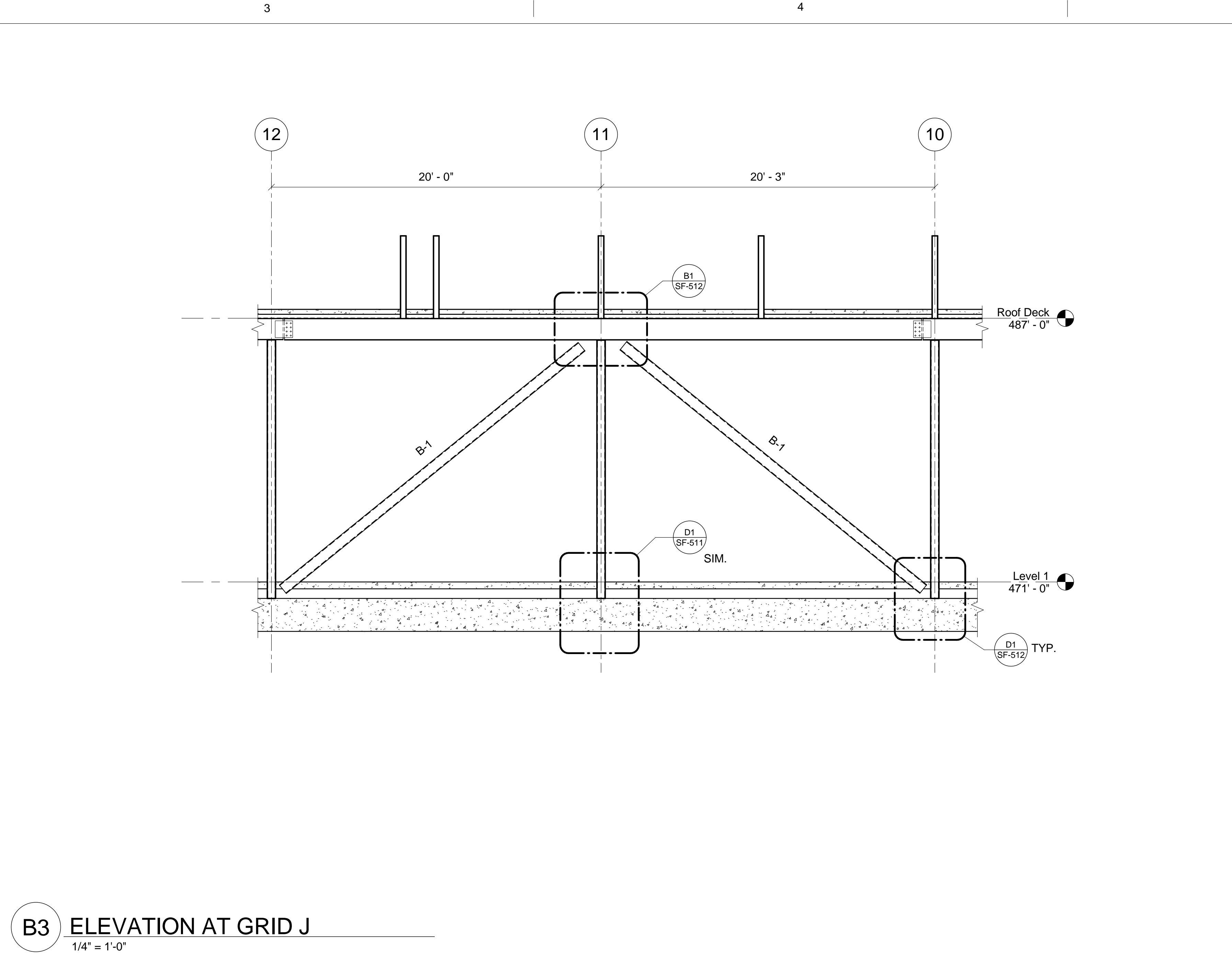
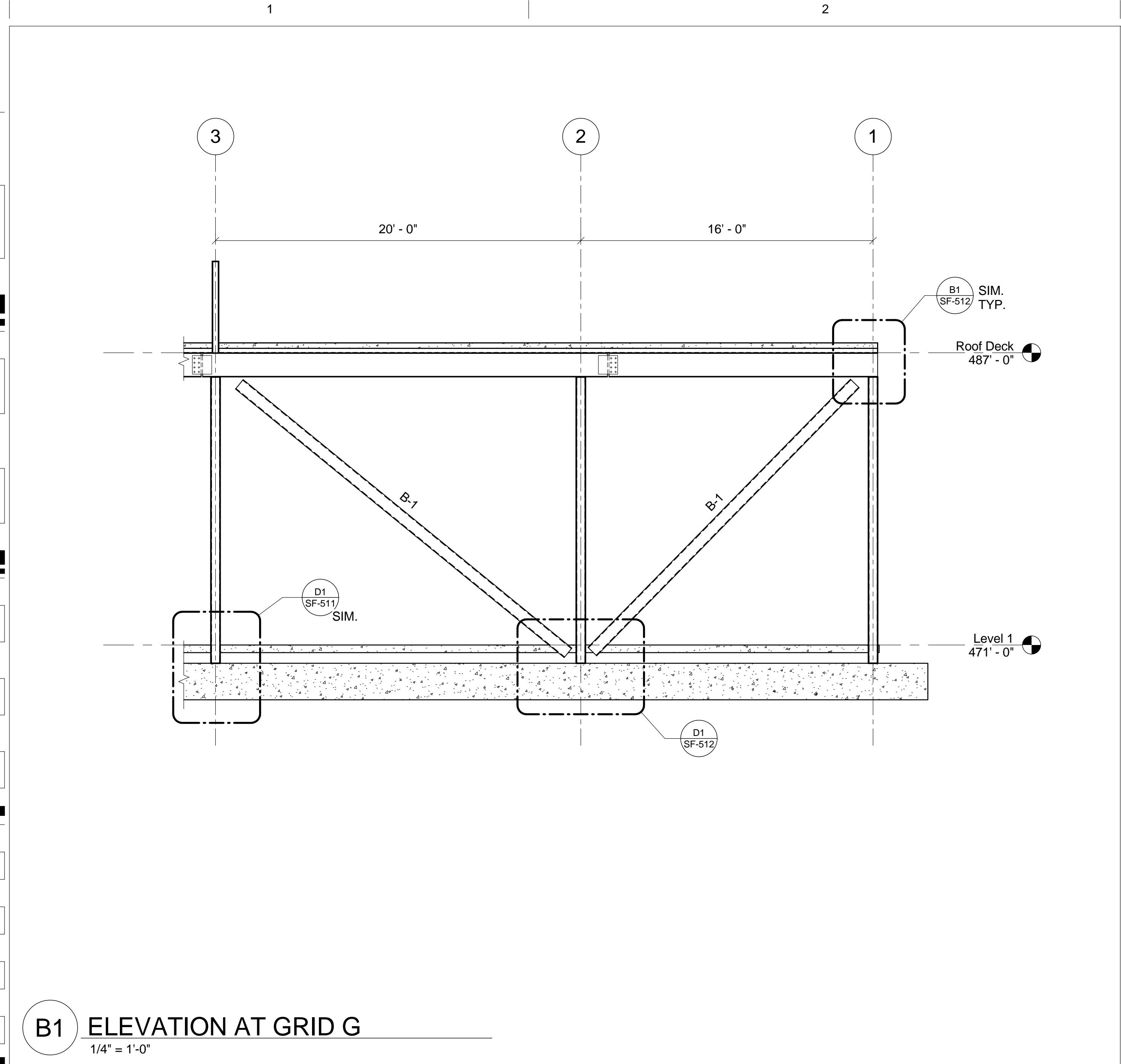
one quarter inch = one foot
0 4 8

three eighths inch = one foot
0 4

one half inch = one foot
0 2

three quarters inch = one foot
0 2 6"

one inch = one foot
0 6"

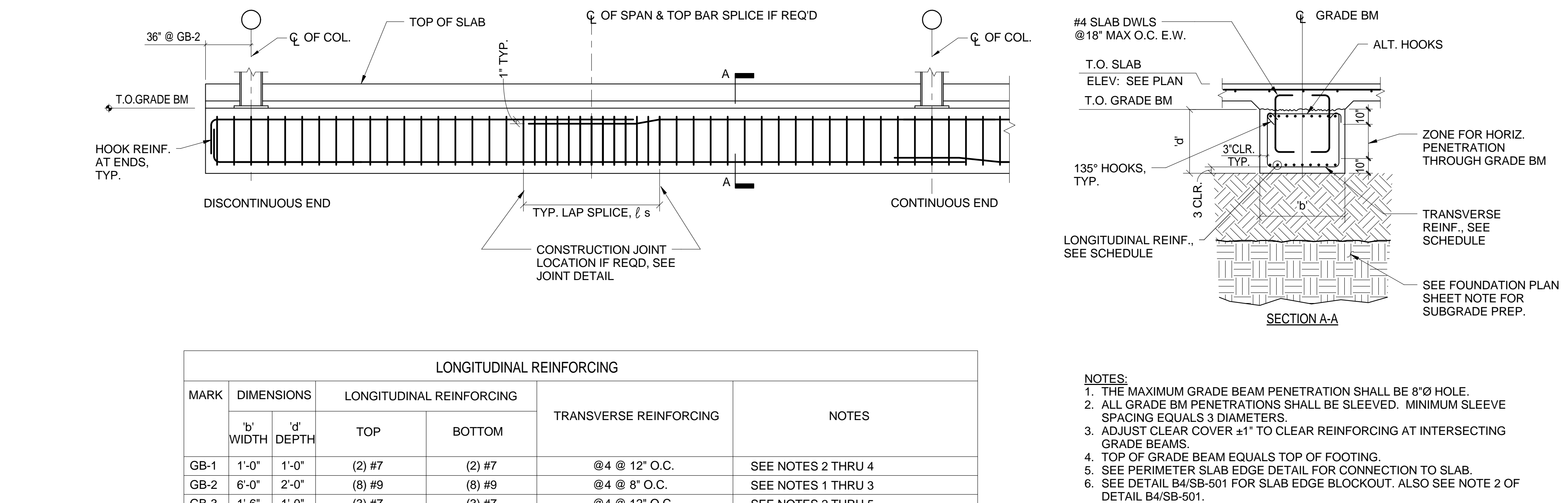
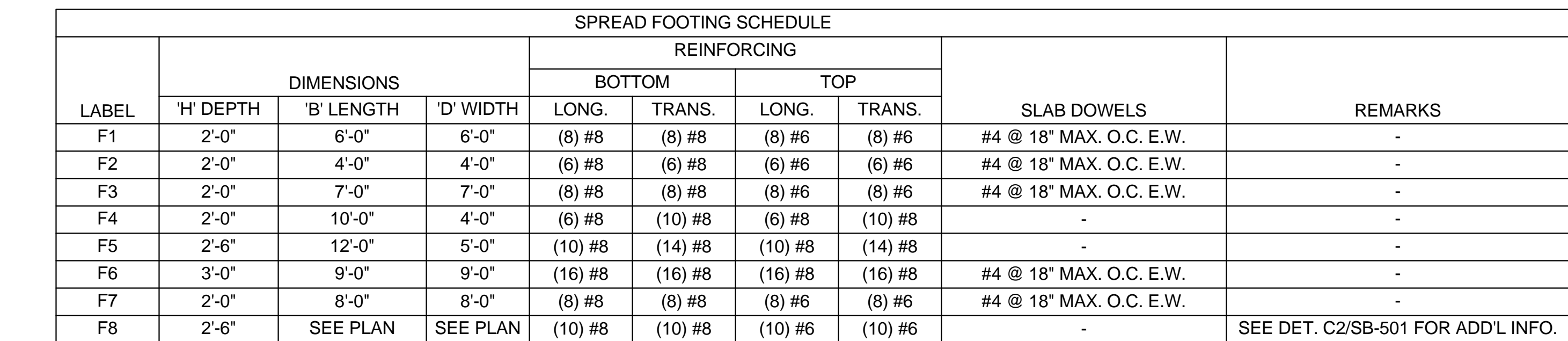
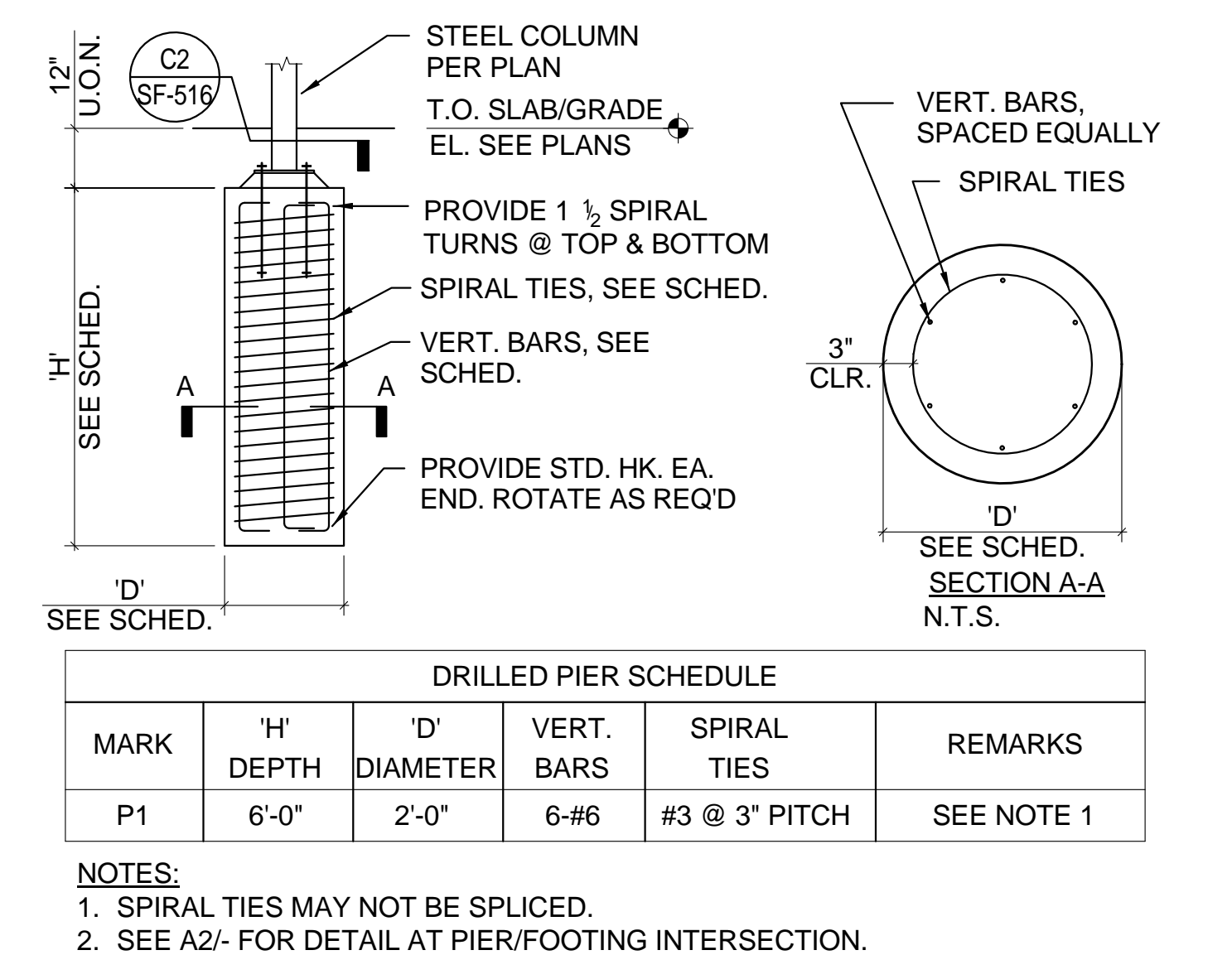


SHEET NOTES

- SEE THE FOLLOWING STRUCTURAL DRAWINGS FOR DETAILS NOT SPECIFICALLY REFERENCED:
SF-031, SF-032: TYPICAL CONCRETE DETAILS.
SF-041: TYPICAL MASONRY DETAILS
SF-051, SF-052: TYPICAL STEEL DETAILS
SF-053: TYPICAL METAL DECK DETAILS
- B-#** DENOTES BUILDING-RESTRAINED BRACE PER SCHEDULE B4/SF-512.


KEY NOTES

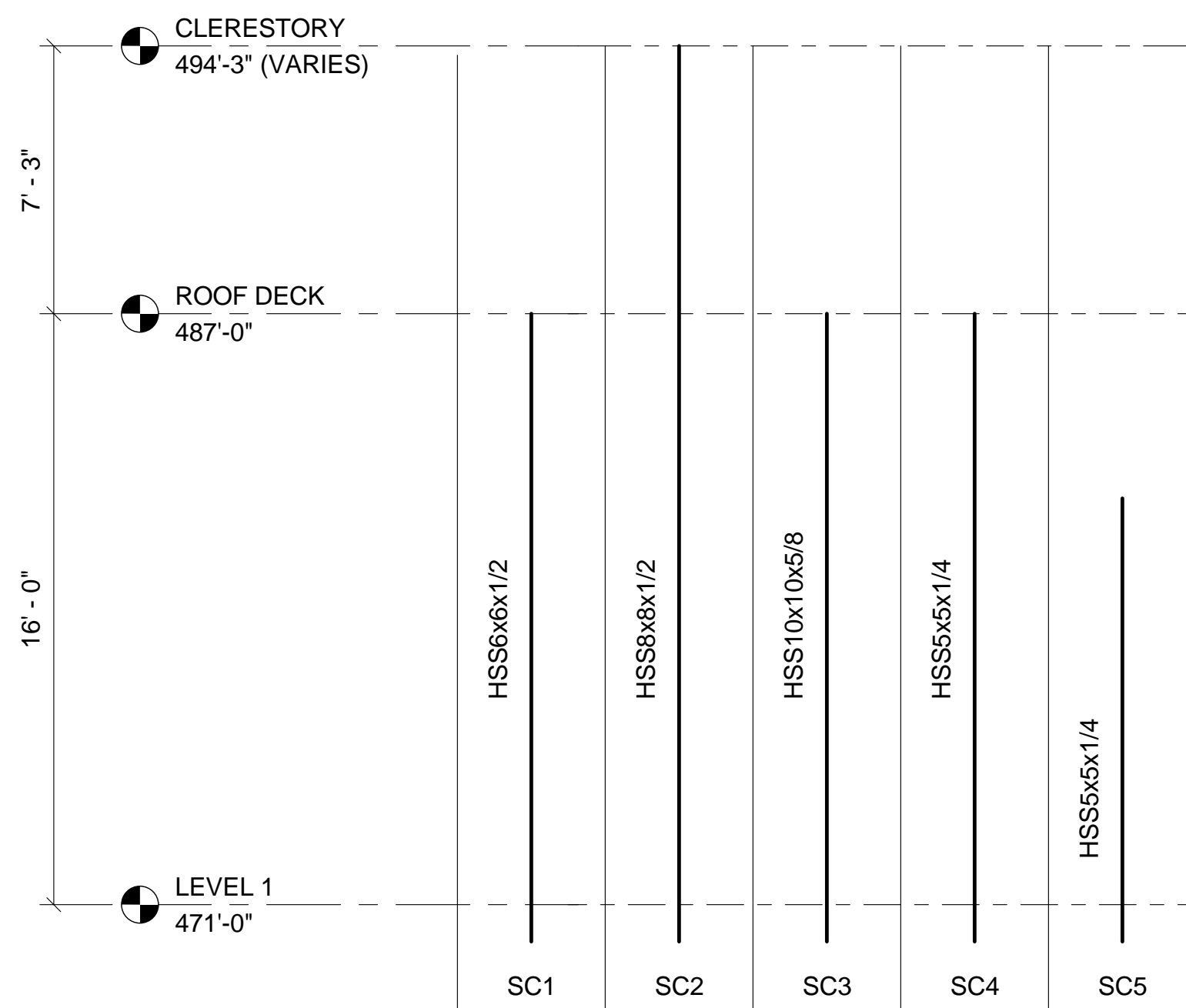
<div>Revisions:</div> <div>Date</div>		<div>CONSULTANTS:</div> <div> Degenkolb</div> <div>1000 SW Broadway, #1800 Portland, OR 97205 Telephone 503.223.9932 Fax 503.242.1780 Degenkolb Project No.: B0437012.00</div>	<div>ARCHITECT/ENGINEERS:</div> <div> Tina Ely architect</div> <div>2915 Wingate Street / Eugene, OR 97408 541.521.2477 / Tina.Ely@comcast.net</div>	<div>Drawing Title</div> <div>BRACED FRAME ELEVATIONS</div> <div>Approved: Project Director</div>	<div>Project Title</div> <div>SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD</div> <div>Location</div> <div>ROSEBURG, OREGON</div> <div>Date</div> <div>30 APRIL 2012</div> <div>Checked</div> <div>Drawn</div>	<div>Project Number</div> <div>653-322</div> <div>Building Number</div> <div>086</div> <div>Drawing Number</div> <div>SF-203</div>	<div>Office of Construction and Facilities Management</div> <div> Department of Veterans Affairs</div>
---------------------------------------	--	---	---	--	--	---	---



LONGITUDINAL REINFORCING						
MARK	DIMENSIONS		LONGITUDINAL REINFORCING		TRANSVERSE REINFORCING	NOTES
	'b' WIDTH	'd' DEPTH	TOP	BOTTOM		
GB-1	1'-0"	1'-0"	(2) #7	(2) #7	@ 4" @ 12" O.C.	SEE NOTES 2 THRU 4
GB-2	6'-0"	2'-0"	(8) #9	(8) #9	@ 4" @ 8" O.C.	SEE NOTES 1 THRU 3
GB-3	1'-6"	1'-0"	(3) #7	(3) #7	@ 4" @ 12" O.C.	SEE NOTES 2 THRU 5

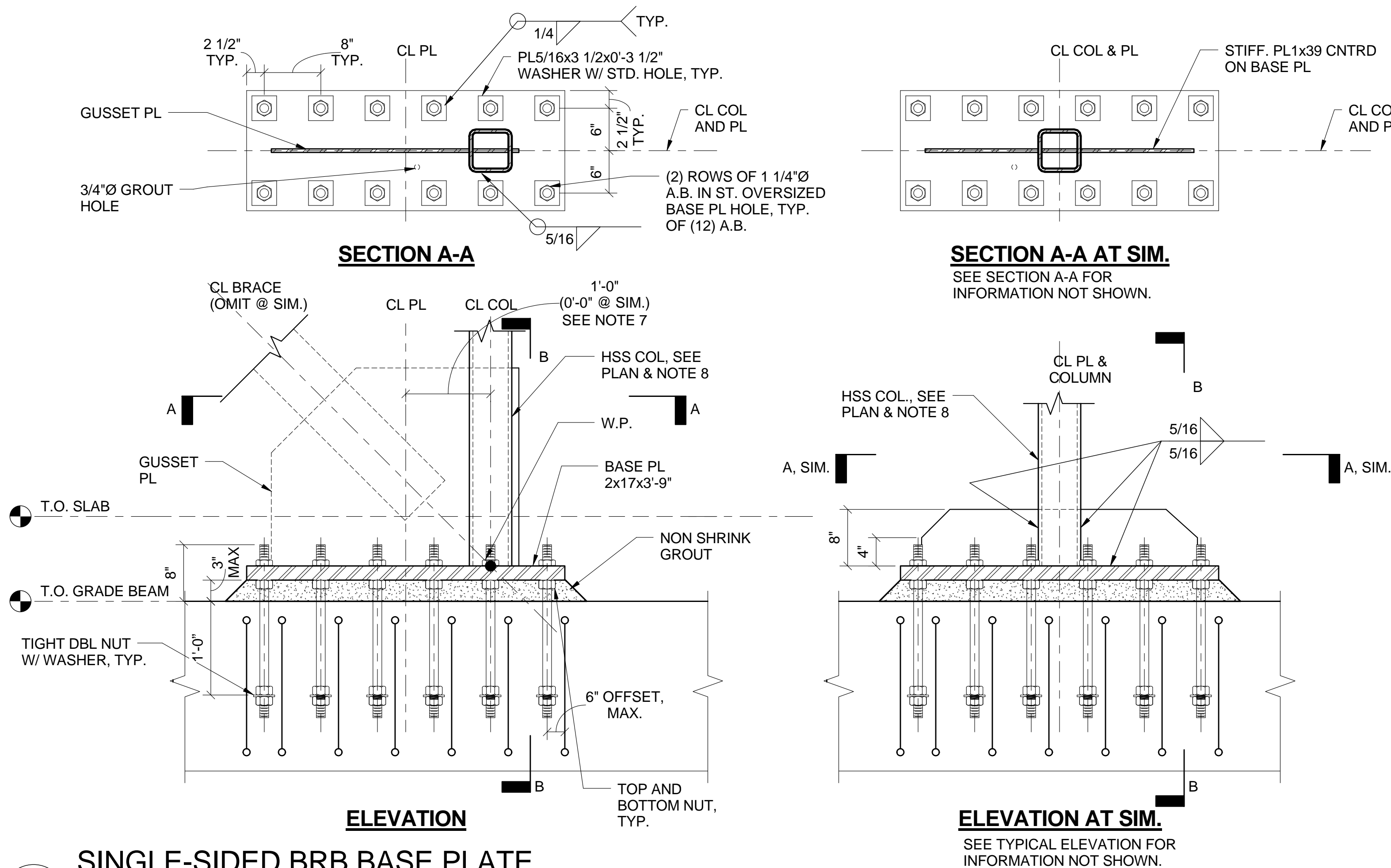


Drawing Title	FOUNDATION DETAILS		Project Title		Project Number	Office of Construction and Facilities Management
			SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD		653-322	
Approved: Project Director	Location ROSEBURG, OREGON		Date 30 APRIL 2012	Checked	Drawn	Building Number 086
						SB-501
						 Department of Veterans Affairs



NOTE: SEE B5/SF-511 BASE PLATE DETAIL U.O.N.

B1 COLUMN SCHEDULE

$$\frac{1}{4}'' = 1'-0''$$


D1 SINGLE-SIDED BRB BASE PLATE CONNECTION

$$1'' = 1' - 0''$$
[illegible]

CONSULTANTS:



1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00

ARCHITECT/ENGINEERS:

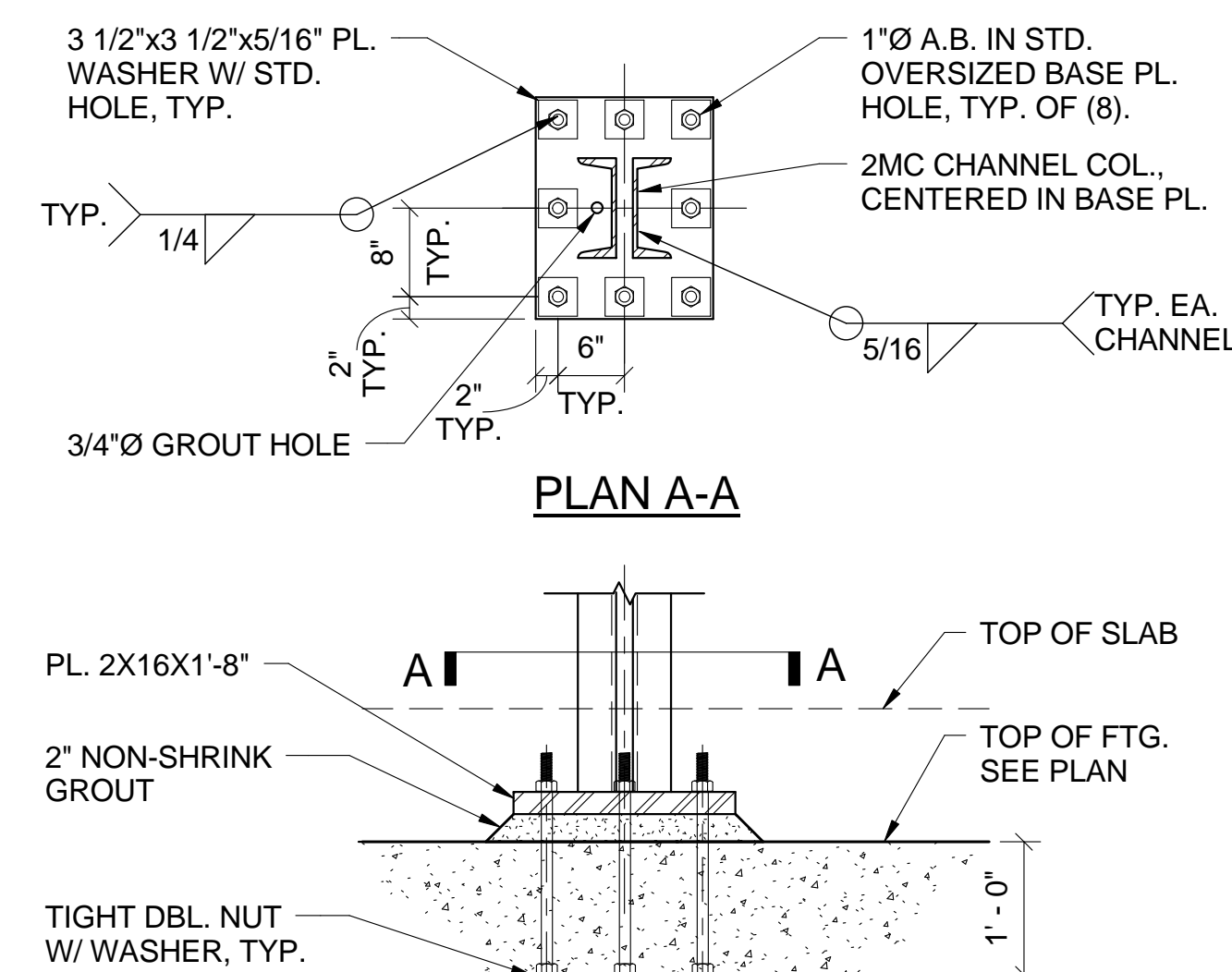


Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

SLAB TYPE	DECK TYPE	HEIGHT	GAUGE	FACTORY VENTED	MIN. SECTION PROPERTIES			MIN. FILL THICKNESS ABOVE TOP FLUTE	STUD SIZE	SLAB REINFORCING	ATTACHMENT TYPE				SIDE LAP	REMARKS
					I (in4)	+S (in3)	-S (in3)				TO PERIMETER SUPPORT		TO INTERMEDIATE SUPPORT			
											PERPENDICULAR TO DECK	PARALLEL TO DECK	PERPENDICULAR TO DECK	PARALLEL TO DECK		
D1	W	3"	18	YES	1.203	0.767	0.767	3 1/2"	3/4"Ø x 4 1/2"	#5 @ 12" O.C., EA. WAY	1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	1/2" PUDDLE WELD AT ALL DOWN FLUTES	1/2" PUDDLE WELD @ 12" O.C.	BUTTON PUNCH @ 24" O.C.	SHORE SINGLE SPANS IN EXCESS OF 10'-0"
D2	B	1 1/2"	18	NO	0.302	0.322	0.355	-	-	-	1/2" PUDDLE WELD AT ALL FLUTES	1/2" PUDDLE WELD @ 6" O.C.	1/2" PUDDLE WELD AT ALL FLUTES	1/2" PUDDLE WELD @ 6" O.C.	TOP SEAM WELD @ 12" O.C.	-
D3	B	1 1/2"	22	NO	0.175	0.187	0.198	-	-	-	1/2" PUDDLE WELD AT ALL FLUTES	1/2" PUDDLE WELD @ 6" O.C.	1/2" PUDDLE WELD AT ALL FLUTES	1/2" PUDDLE WELD @ 6" O.C.	TOP SEAM WELD @ 12" O.C.	PRE-FINISHED DECK PER B1/A5-604

NOTE:
SPLICE ALL TYPICAL AND ADDITIONAL SLAB REINFORCEMENT WITH
TYPE 2 MECHANICAL SPLICES.

A3 METAL DECK SCHEDULE

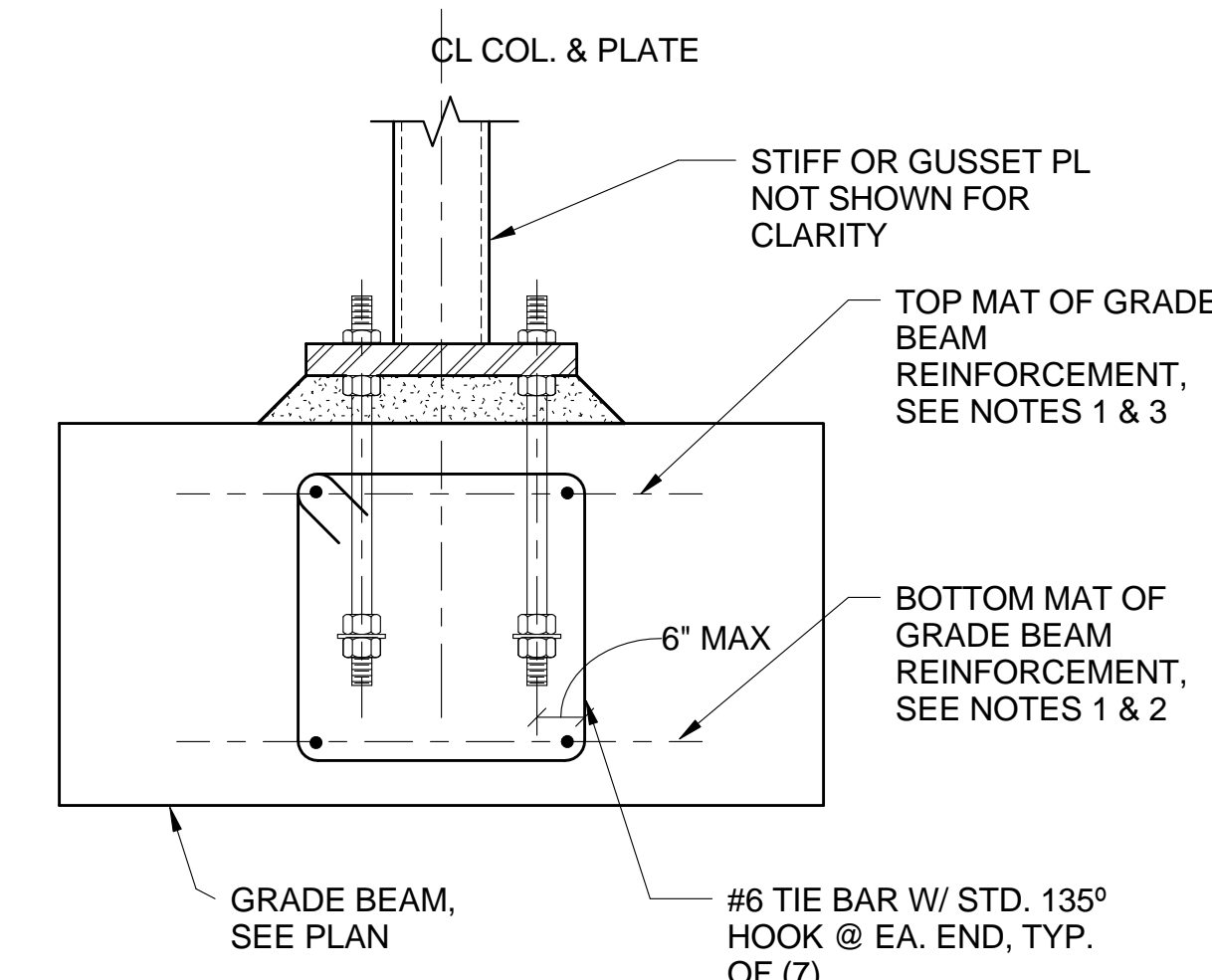
$$1'' = 1' - 0''$$


(B3) COLUMN BASE PLATE FOR 2MC9

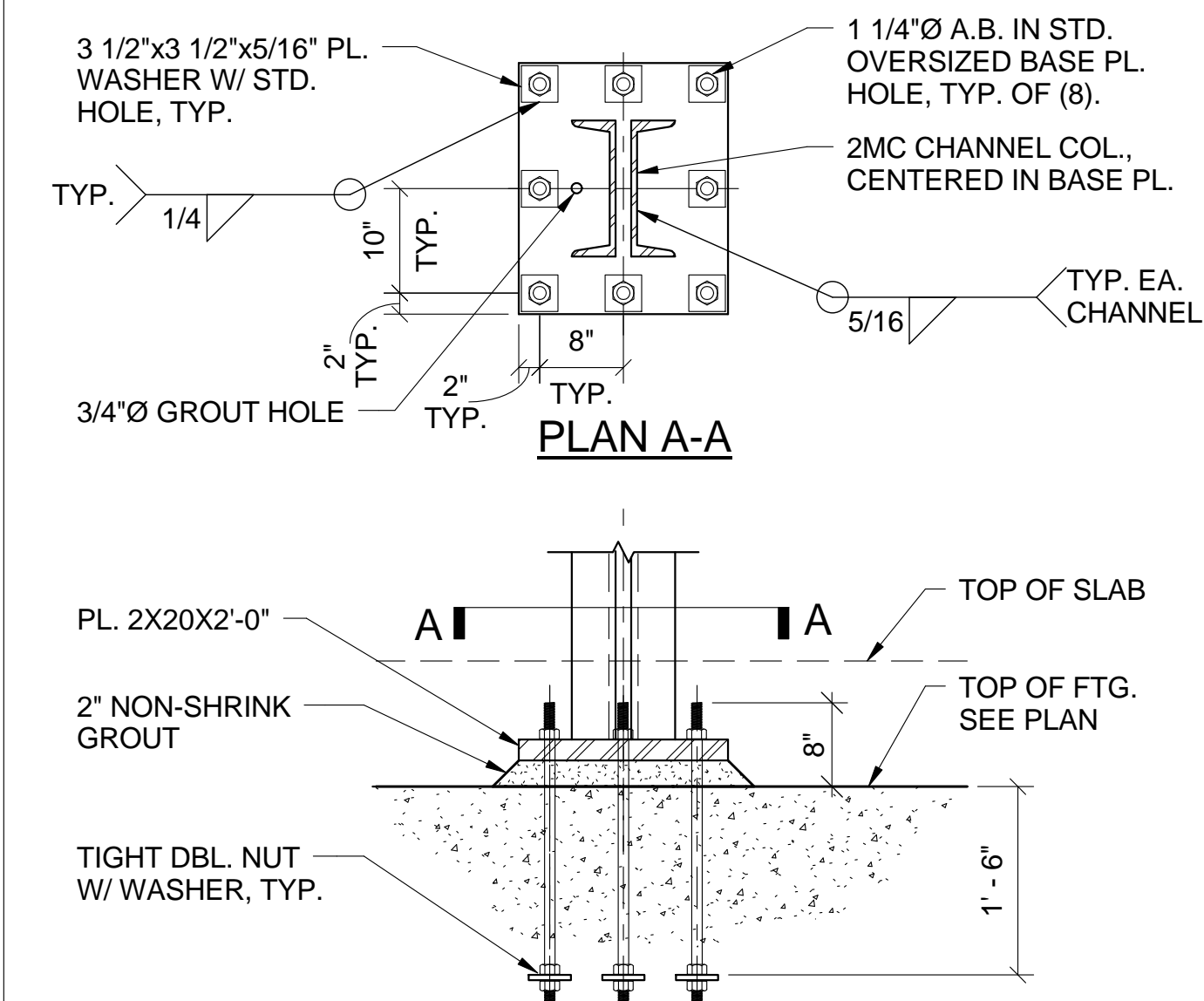
$$\frac{3}{4}'' = 1'-0''$$

NOTES:

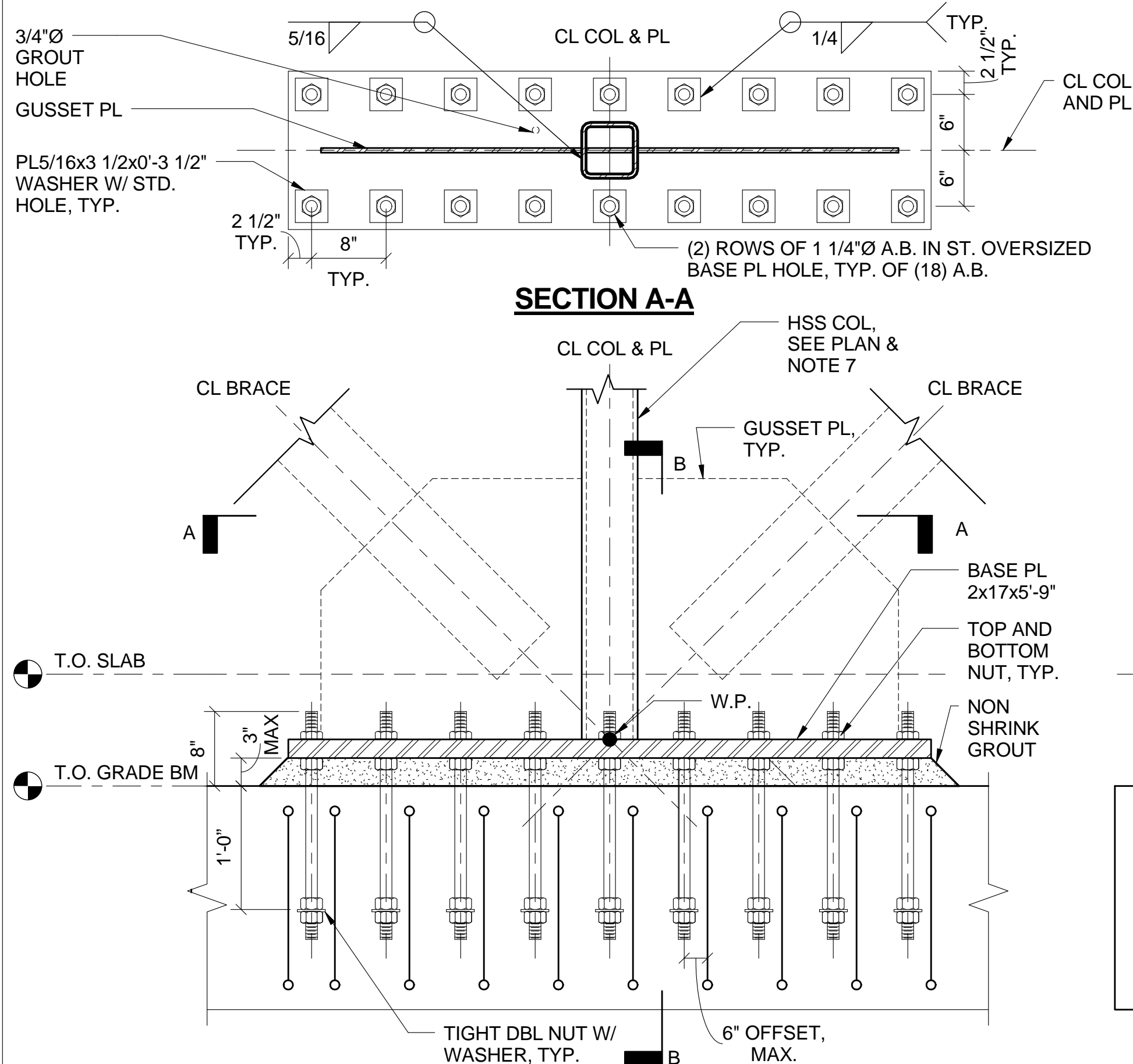
1. SEE GRADE BEAM SCHEDULE FOR GRADE BEAM REINFORCEMENT, NOT SHOWN FOR CLARITY.
2. ALIGN (2) BARS OF BOTTOM MAT LONGITUDINAL GRADE BEAM REINFORCEMENT TO HOOK #6 TIE AROUND.
3. ALIGN (2) BARS OF TOP MAT LONGITUDINAL GRADE BEAM REINFORCEMENT TO HOOK #6 TIE AROUND.
4. BRB AND GUSSET PLATE SHOWN DASHED FOR CLARITY.
5. BASE PLATE LENGTHS ARE BASED ON PRELIMINARY GUSSET PLATE SIZES. NOTIFY THE RESIDENT ENGINEER IF FINAL GUSSET PLATE SIZES DO NOT COORDINATE WITH THE INFORMATION GIVEN HEREIN.
6. SEE NOTE 2 OF DETAIL B4/SB-501.
7. GRADE OMITTED AT SIMILAR CONDITION, CENTERLINE OF PLATE EQUALS CENTERLINE OF COLUMN AT SIMILAR CONDITION.
8. SEE DETAIL D3/SF-502 FOR HSS SLOT PROFILE REQUIREMENTS.



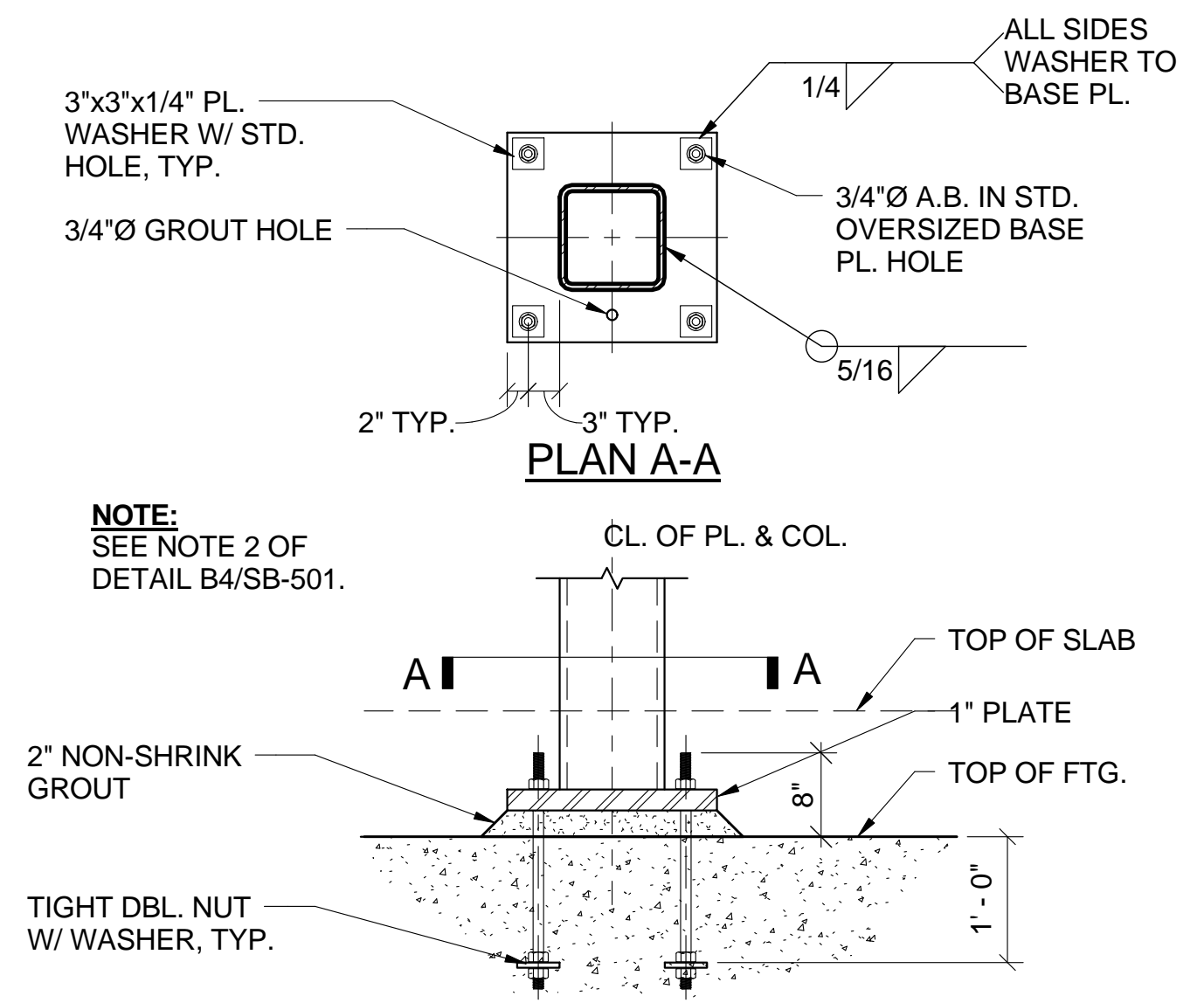
SECTION B-B (TYP.)



B4 COLUMN BASE PLATE FOR 2MC13

$$\frac{3}{4}'' = 1'-0''$$


D4 DOUBLE-SIDED BRB BASE
PLATE CONNECTION

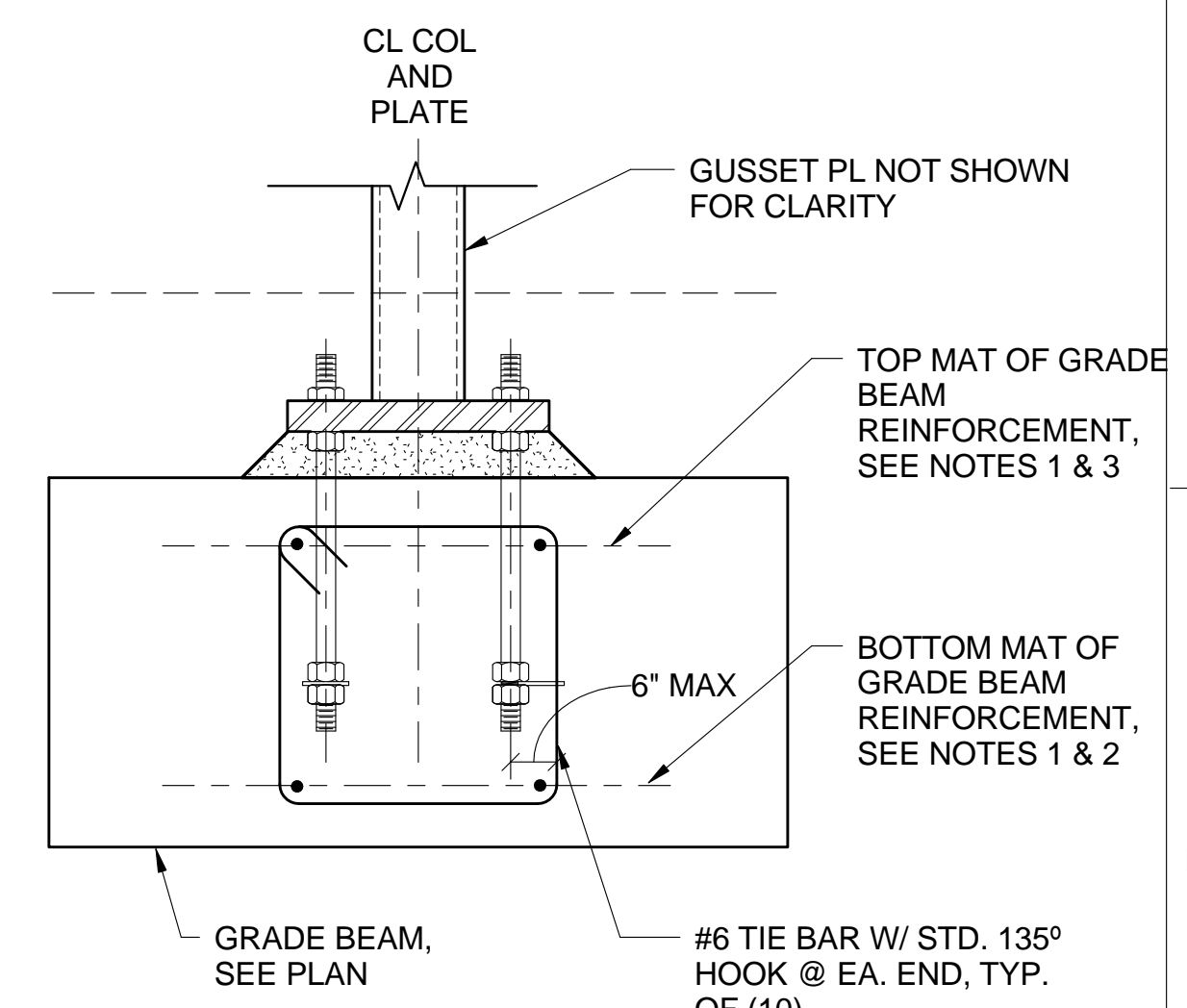
$$1'' = 1' - 0''$$


B5 GRAVITY COLUMN BASE PLATE
FOR HSS

3/4" = 1'-0"

NOTES:

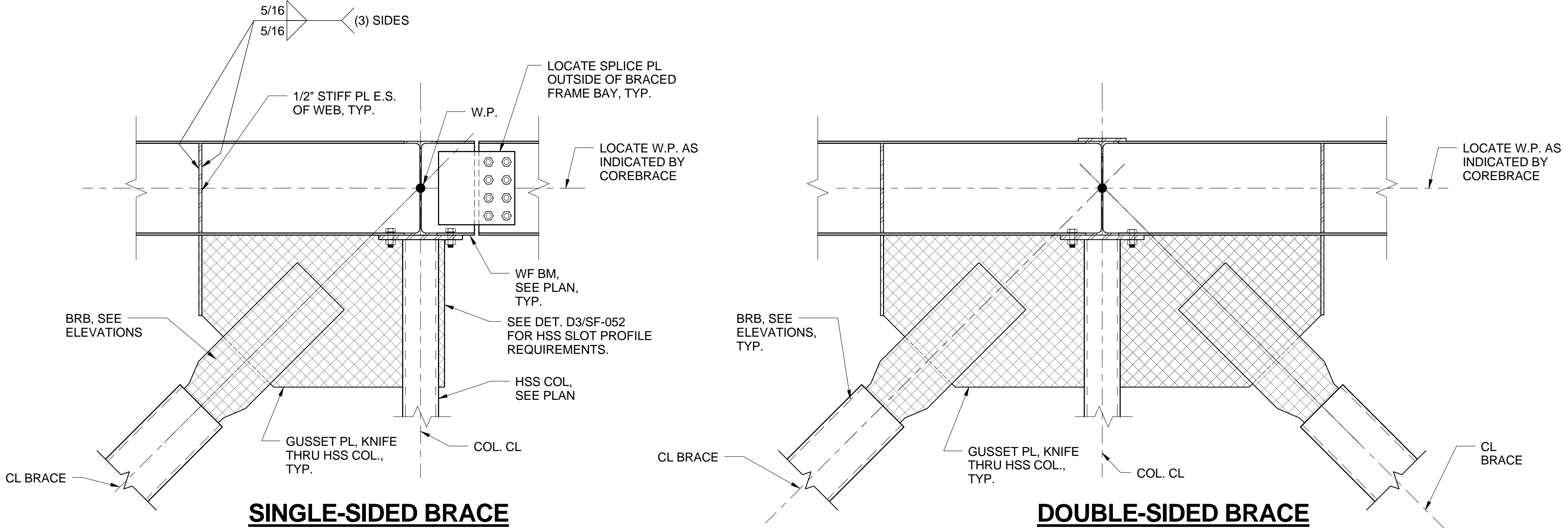
- NOTES:**
1. SEE GRADE BEAM SCHEDULE FOR GRADE BEAM REINFORCEMENT. NOT SHOWN FOR CLARITY.
 2. ALIGN (2) BARS OF BOTTOM MAT LONGITUDINAL GRADE BEAM REINFORCEMENT TO HOOK #6 TIE AROUND.
 3. ALIGN (2) BARS OF TOP MAT LONGITUDINAL GRADIBEAM REINFORCEMENT TO HOOK #6 TIE AROUND.
 4. BRB AND GUSSET PLATE SHOWN DASHED FOR CLARITY.
 5. BASE PLATE LENGTHS ARE BASED ON PRELIMINARY GUSSET PLATE SIZES. NOTIFY THE RESIDENT ENGINEER IF FINAL GUSSET PLATE SIZES DO NO COORDINATE WITH THE INFORMATION GIVEN HEREIN.
 6. SEE NOTE 2 OF DETAIL B4/SB-501.
 7. SEE DETAIL D3/SF-502 FOR HSS SLOT PROFILE REQUIREMENTS.



SECTION B-B

Drawing Title STEEL DETAILS	Project Title SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD			Project Number 653-322	Office of Construction and Facilities Management
	Approved: Project Director	Location ROSEBURG, OREGON			
	Date 30 APRIL 2012	Checked	Drawn	Drawing Number SF-511	

one inch = one foot
6" 0 2
three quarters inch = one foot
6" 0 2
one half inch = one foot
6" 0 2
three eighths inch = one foot
4 0 4
one quarter inch = one foot
4 0 4
one eighth inch = one foot
16 0 4 8
one sixteenth inch = one foot
16 0 4 8
three inches = one foot
6" 0 2
one and one half inches = one foot
16 0 8 16



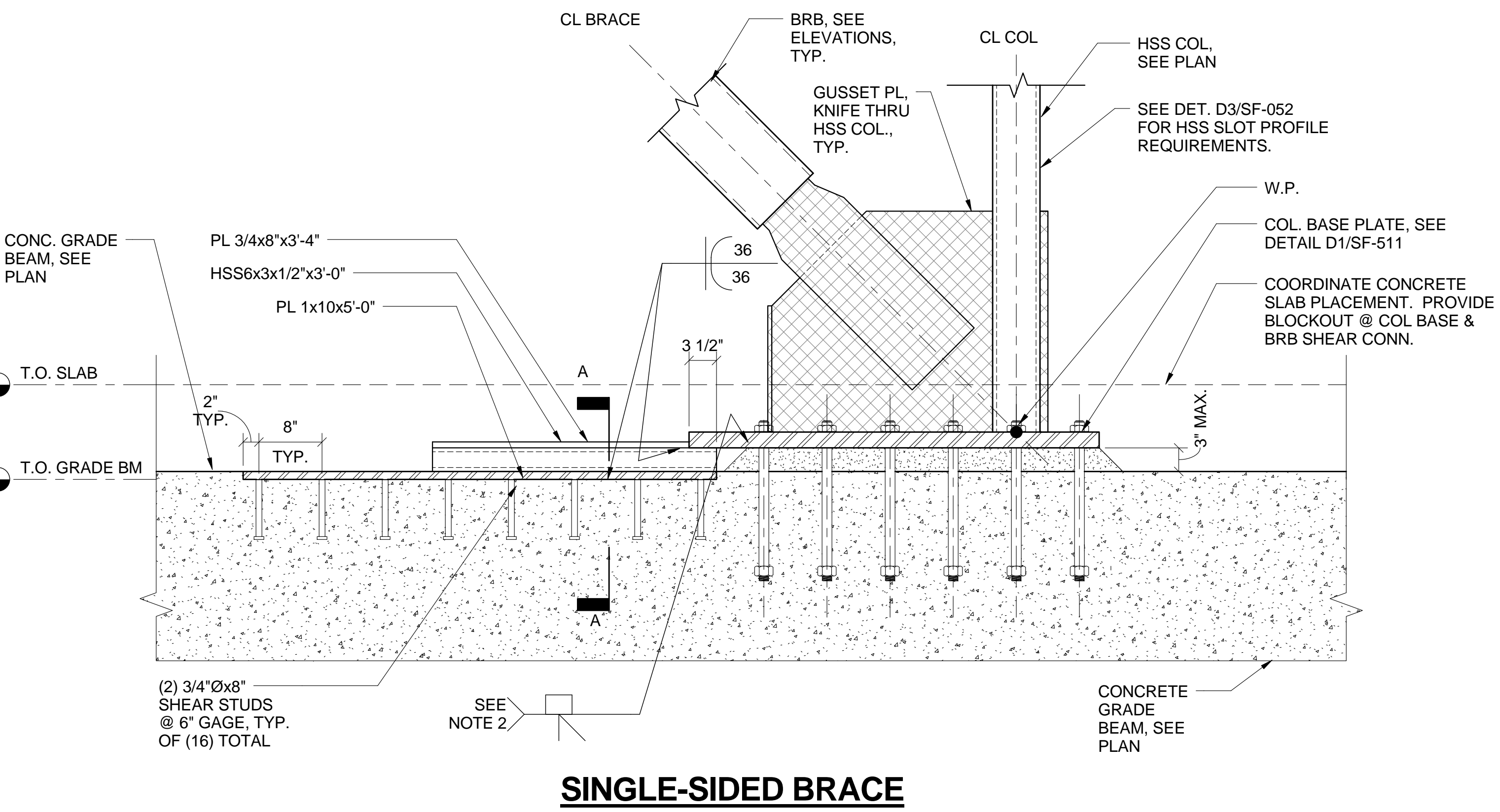
- NOTES:**
- BUCKLING RESTRAINED BRACE TO GUSSET CONNECTION, GUSSET TO BEAM/COLUMN CONNECTION, AND THE GUSSET PLATE ITSELF ARE TO BE DESIGNED BY COREBRACE. DESIGN MUST SATISFY THE CONSTRAINTS SHOWN AND IS SUBJECT TO THE REVIEW AND APPROVAL OF THE VA RESIDENT ENGINEER.
 - THE FRAMING COMPONENTS SHOWN IN THIS DETAIL IN ELEVATION ARE PART OF THE SEISMIC LOAD RESISTING SYSTEM.
 - BRB FRAME PROTECTED ZONES. WELDED, BOLTED, SCREWED, OR SHOT-IN ATTACHMENTS OF ANY TYPE, BY ANY TRADE, ARE PROHIBITED WITHIN THE PROTECTED ZONES. PROTECTED ZONE ALSO INCLUDES CORE PLATE WITHIN CASING.
 - ATTACHMENTS TO THE BRB CASING, OTHER THAN THOSE SHOWN IN THE DRAWINGS, ARE NOT ALLOWED WITHOUT APPROVAL OF THE ENGINEER OF RECORD AND COREBRACE.

BUCKLING - RESTRAINED BRACE SCHEDULE				
				CASING
BRACE MARK	STEEL CORE AREA (sq. in)	STIFFNESS RATIO K _f	MINIMUM STROKE (INCHES)	MAXIMUM HSS SIZE (inches)
B-1	2.0	1.6	4"	8x8
B-2	3.5	1.6	4"	8x8

- NOTES:**
- BRB MANUFACTURER SHALL DESIGN BRB AND GUSSET PLATE CONNECTIONS IN ACCORDANCE WITH AISC 341-05.
 - BRB MANUFACTURER SHALL PROVIDE THE STRUCTURAL ENGINEER OF RECORD WITH TEST RESULTS FOR SIMILAR BRACES IN ACCORDANCE WITH AISC 341-05.
 - THE BRB COMPRESSION STRENGTH ADJUSTMENT FACTOR, BETA, SHALL NOT EXCEED 1.15, THE STRAIN HARDENING ADJUSTMENT FACTOR, OMEGA, SHALL NOT EXCEED 1.40, AND THE PRODUCT OF BETA TIMES OMEGA SHALL NOT EXCEED 1.50.
 - EFFECTIVE STIFFNESS BASED ON WORK POINT TO WORK POINT LENGTH TO BE $K_f \times (\text{AREA} \times E/L)$. THE EFFECTIVE STIFFNESS OF THE SUPPLIED BRB'S SHALL BE WITHIN $\pm 2.5\%$ OF THE VALUES INDICATED IN THE SCHEDULE.
 - CORE PLATES SHALL BE MADE FROM ASTM A36 STEEL. ACTUAL YIELD STRESS OF BRB CORE PLATES SHALL BE 42 KSI ± 4 KSI.
 - HSS SHALL BE ASTM A500 GRADE B STEEL. SIZE SHOWN IS MAXIMUM. ACTUAL SIZE TO BE DETERMINED BY BRB MANUFACTURER.
 - BRB MANUFACTURER SHALL DESIGN BRACE AND END CONNECTIONS FOR HORIZONTAL OUT-OF-PLANE LOADS ON BUILDING PERIMETER AS FOLLOWS:

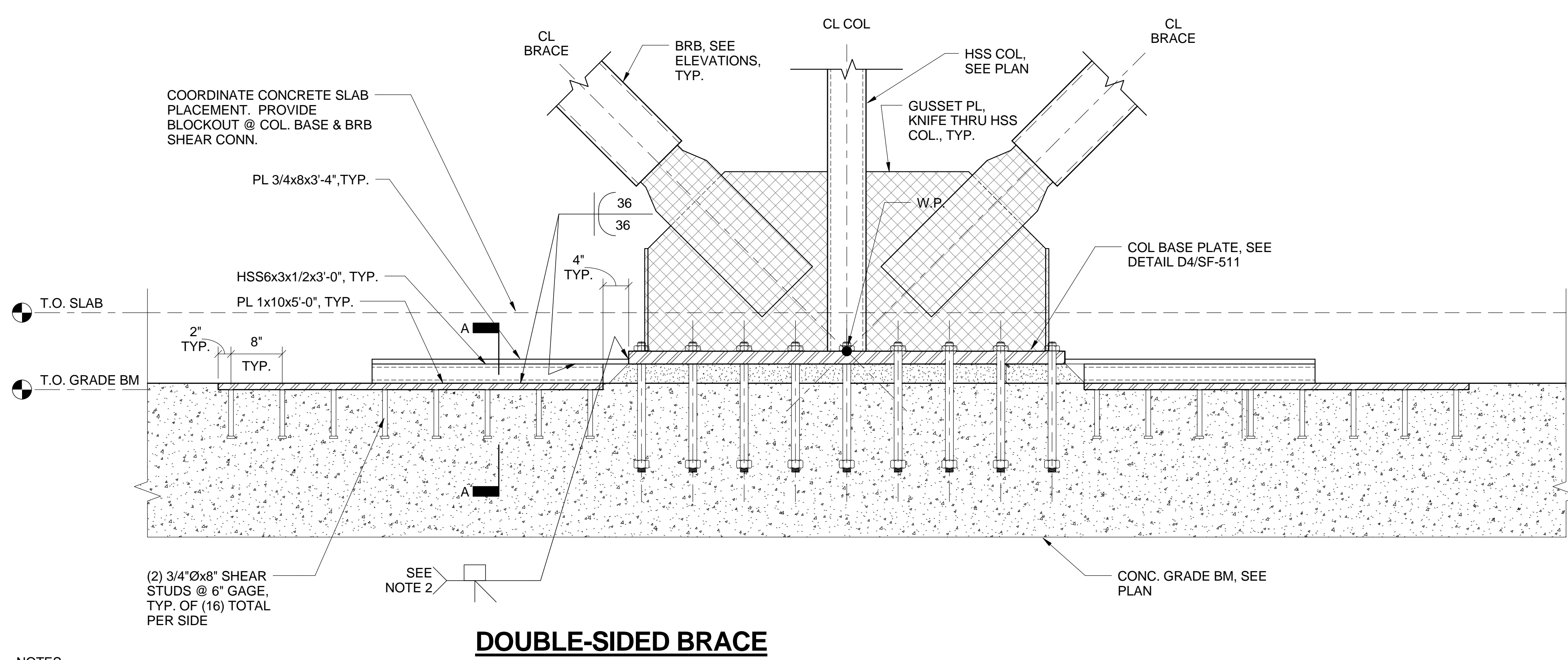
WIND = 200 PLF
SEISMIC = 240 PLF

B1 BRB TO BM CONNECTION
1" = 1'-0"



D1 BRB SHEAR TRANSFER CONNECTION
1" = 1'-0"

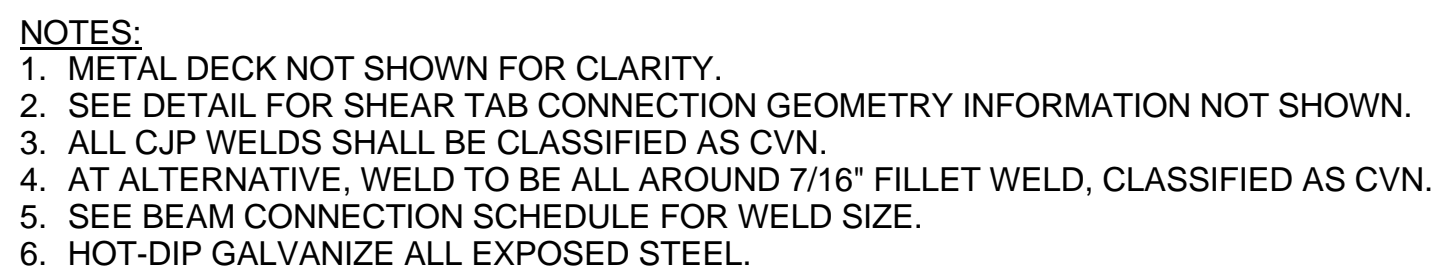
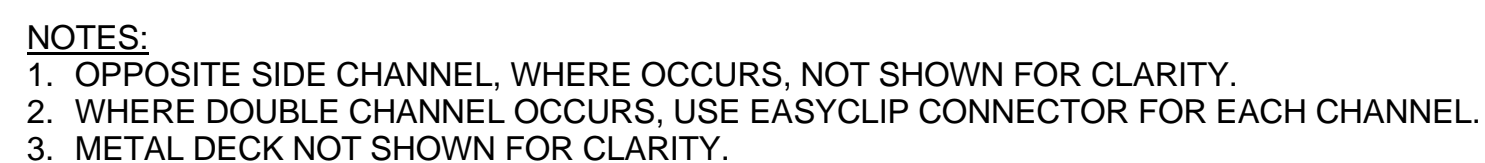
B4 BUCKLING RESTRAINED BRACE DETAIL
1" = 1'-0"



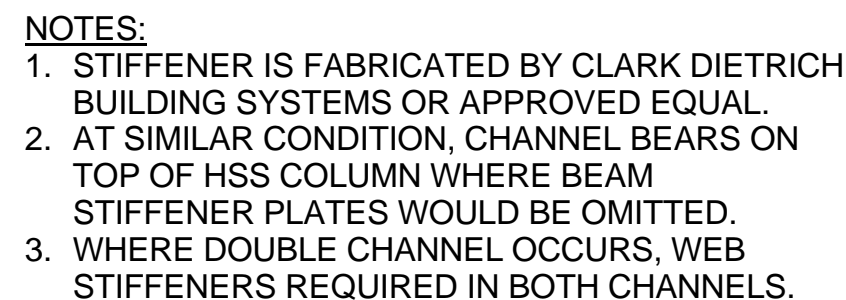
- NOTES:**
- CONCRETE SLAB/GRADE BEAM REINFORCING NOT SHOWN FOR CLARITY.
 - COMPLETE WELD PRIOR TO GROUTING BASE PLATE.
 - NOT USED
 - BRB FRAME PROTECTED ZONES. WELDED, BOLTED, SCREWED, OR SHOT-IN ATTACHMENTS OF ANY TYPE, BY ANY TRADE, ARE PROHIBITED WITHIN THE PROTECTED ZONES.
 - ATTACHMENTS TO THE BRB CASING, OTHER THAN THOSE SHOWN IN THE DRAWINGS, ARE NOT ALLOWED WITHOUT APPROVAL OF THE ENGINEER OF RECORD AND COREBRACE.
 - BUCKLING RESTRAINED BRACE TO GUSSET CONNECTION, GUSSET TO BASEPLATE CONNECTION, AND THE GUSSET PLATE ITSELF ARE TO BE DESIGNED BY COREBRACE. DESIGN MUST SATISFY THE CONSTRAINTS SHOWN AND IS SUBJECT TO THE REVIEW AND APPROVAL OF THE VA RESIDENT ENGINEER.

CONSULTANTS: 1000 SW Broadway, #1800 Portland, OR 97205 Telephone 503.223.9932 Fax 503.242.1780 Degenkolb Project No.: B0437012.00		ARCHITECT/ENGINEERS: Tina Ely architect 2915 Wingate Street / Eugene, OR 97408 541.521.2477 / Tina.Ely@comcast.net		Drawing Title STEEL DETAILS	Project Title SEISMIC REPLACEMENT BLDG 2 PHASE I MINOR ACUTE PSYCHIATRIC WARD	Project Number 653-322 Building Number 086	Office of Construction and Facilities Management 	
Revisions:	Date	Approved: Project Director		Location ROSEBURG, OREGON	Date 30 APRIL 2012	Checked	Drawn	Drawing Number SF-512

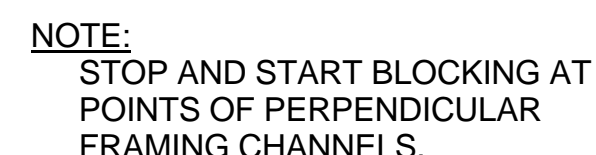



$$1\ 1/2'' = 1'-0''$$

$$3'' = 1'-0''$$


N.T.S.



A4



D4



D4

VA FORM 08-6231

Degenkolb

1000 SW Broadway, #1800
Portland, OR 97205
Telephone 503.223.9932
Fax 503.242.1780
Degenkolb Project No.: B0437012.00



Tina Ely architect
2915 Wingate Street / Eugene, OR 97408
541.521.2477 / Tina.Ely@comcast.net

Drawing Title

Approved: Project Director

Project Title
SEISMIC REPLACEMENT BLDG 2
PHASE I MINOR
ACUTE PSYCHIATRIC WARD

Location
ROSEBURG, OREGON

Date
30 APRIL 2012

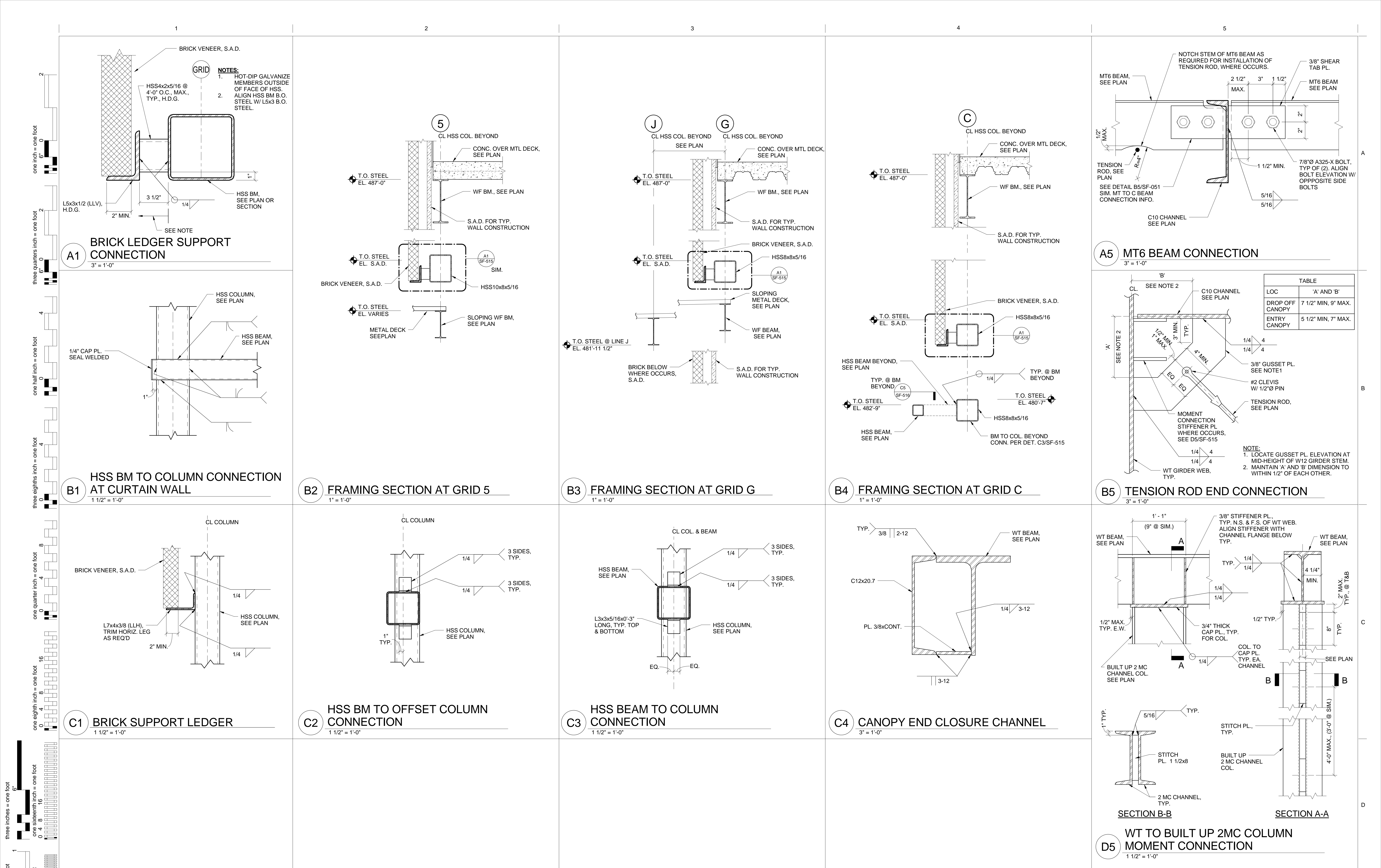
Drawn

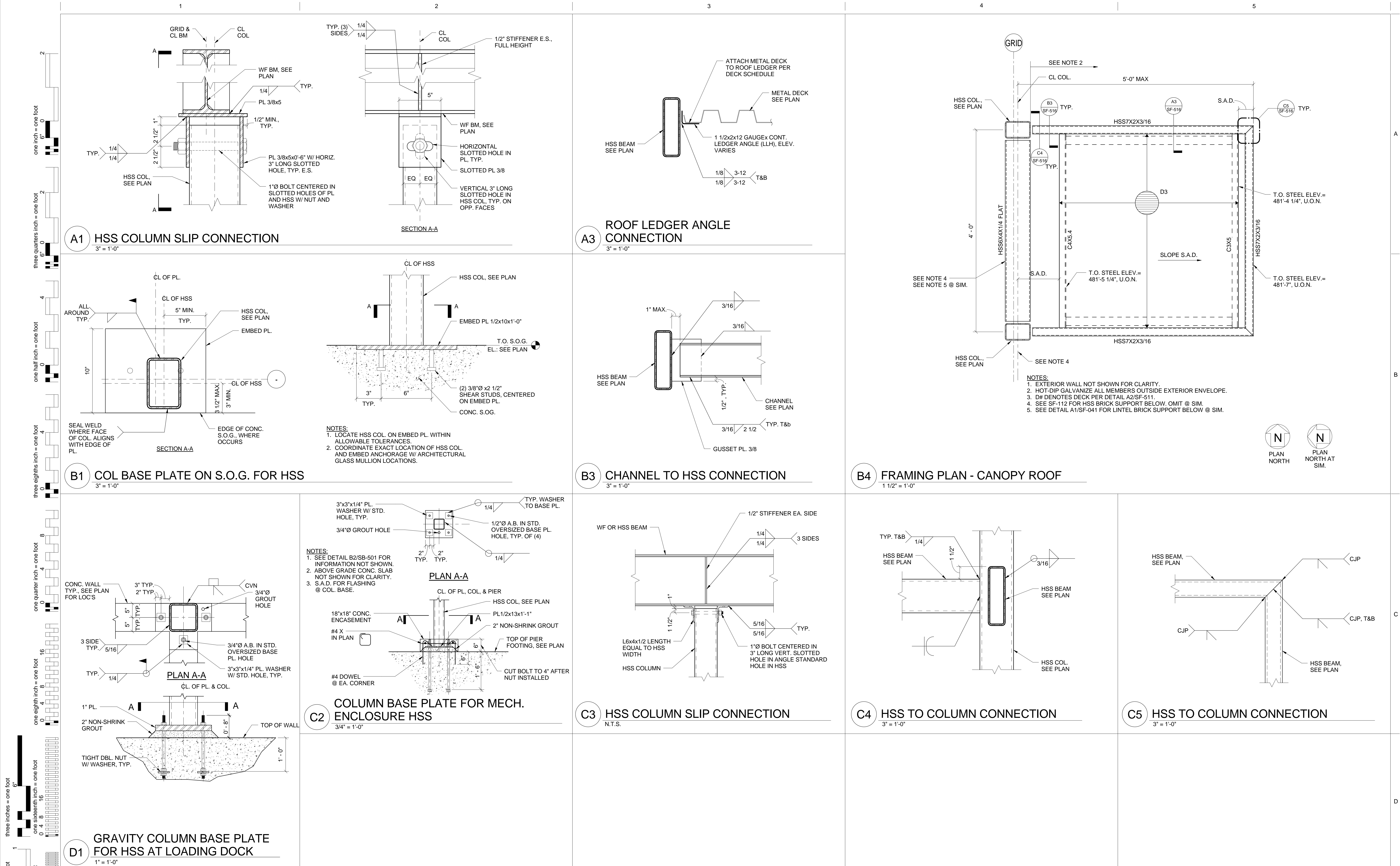
Project Number	653-322
Building Number	086

SF-514

Office of
Construction and
Facilities
Management







D1 GRAVITY COLUMN BASE PLATE FOR HSS AT LOADING DOCK
1" = 1'-0"

--	--