### Addendum 1

Project: Relocate PCT and Convert File Room Space

Project No. 568-12-114

Solicitation No. VA26313R0076

Proposal Due Date: COB December 10, 2011

Item 1. The attached drawings, SB001, SB101 and SF101 were not included in the original published Solicitation, these drawings, by this Addendum, shall be included and become a part of the Contract Documents for this Project.

Item 2. The Proposal Due Date and time **shall not** change due to this Addendum

DESIGN LIVE LOADS CORRIDORS OFFICES SIDEWALKS CEILING DESIGN WIND LOAD DESIGN SEISMIC LOAD MATERIAL DATA: CEMENT TYPE AGGREGATES METAL DECKING STRUCTURAL NOTES: ROOF DIAPHRAGM: METAL ROOF DECKING FROM SAID ACCEPTANCE. INSTALLATION. PROVIDE ADEQUATE STRUCTURAL FRAMING AS APPROVED BY THE ENGINEER FOR AND LOCATIONS OF SLAB DROPS, MASONRY CONTROL JOINTS, AND WALL OPENING CONSULTANTS:

VA FORM 08-6231

APPLICABLE CODES AND STANDARDS - 2006 INTERNATIONAL BUILDING CODE A.C.I. 318–05 - A.I.S.C. STEEL CONSTRUCTION MANUAL - 13TH EDITION A.W.S. D1.1, D1.3, D1.4 - A.I.S.I. NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS - 2001 EDITION OFFICE LOBBIES PARTITIONS (LATERAL LOAD) RAILING (LATERAL LOAD AT TOP) 50 PLF GROUND SNOW LOAD ROOF SNOW LOAD SEE PLAN DRIFT SNOW LOAD ROOF LIVE LOAD (MIN) BASIC WIND SPEED EXPOSURE CLASSIFICATION IMPORTANCE FACTOR SEISMIC DESIGN CATEGORY SEISMIC USE GROUP SPECTRAL RESPONSE COEFFICIENT Ss = 0.149 SPECTRAL RESPONSE COEFFICIENT  $S_1 = 0.042$ SITE CLASSIFICATION MAXIMUM ALLOWABLE SOIL BEARING CAPACITY 1500 PSF CONCRETE AND REINFORCING CONCRETE STRENGTH (f'c @ 28 DAYS) FOOTINGS, GRADE BEAMS 3000 PSI CONC. SLAB ON MTL. DECK CONC. NOT SPECIFIED 3000 PSI GRADE SUPPORTED SLABS 4000 PSI PORTLAND TYPE I REGULAR WT. HARDROCK TYPE - ASTM REINFORCING STEEL ASTM A615, GRADE 60 REINFORCING STEEL ASTM A706, WELDABLE WELDED WIRE FABRIC ASTM A185 PREFORMED EXPN. JT. (1/2") ASTM D1751 STRUCTURAL STEEL (WIDE FLANGES) ASTM A992, GRADE 50 STRUCTURAL STEEL (ALL OTHER SHAPES) ASTM A36 STRUCTURAL STEEL PIPE ASTM A53, GRADE B, SCH. 40 STRUCTURAL STEEL TUBE ASTM A500, GRADE B ANCHOR RODS ASTM F1554, GRADE 36 BOLTED CONNECTIONS ASTM A 325N WELDED CONNECTIONS

E70XX ELECTRODES WELDED CONNECTIONS (GALV. SURFACES) E6010 OR E6011 ELECTRODES HEADED CONCRETE ANCHORS (HCA) ASTM A108 GRADES C1010 THROUGH C1020 (Fu=55 KSI) DEFORMED BAR ANCHORS (DBA) ASTM A108 AND A496 (Fy=70 KSI) ASTM A611 OR ASTM A653-94 LIGHT GAGE STEEL STUDS/JOISTS ASTM A570 (G60 GALVANIZING PER ASTM A525 & C955) 12 TO 16 GAGE (PAINTED) ASTM A570 GRADE D (Fy=50 KSI) ASTM A611 GRADE C (Fy=33 KSI) 18+ GAGE (PAINTED) 12 TO 16 GAGE (GALV) ASTM A446 GRADE D (Fy=50 KSI) 18+ GAGE (GALV) ASTM A446 GRADE A (Fy=33 KSI)

ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE APPLICABLE BUILDING

THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETED STRUCTURE - APPLICATION OF ANY LOADS TO THE PARTIALLY COMPLETED STRUCTURE SHALL BE CONSIDERED BY THE CONTRACTOR AND SO INCLUDED IN THE DESIGN OF THE SHORING, BRACING, FORMWORK, AND ANY OTHER SUPPORTING ELEMENTS PROVIDED FOR CONSTRUCTION OF THE STRUCTURE. - WHERE CONSTRUCTION MATERIAL AND EQUIPMENT ARE TEMPORARILY STORED ON THE ROOF OR FLOOR FRAMING. THEY SHALL BE DISTRIBUTED SO THAT THE DESIGN LIVE LOAD AT THE LOADED AREA IS NOT EXCEEDED. - DO NOT BACKFILL AGAINST WALLS OR OTHER STRUCTURAL ELEMENTS UNTIL SUCH ELEMENTS HAVE REACHED THEIR INTENDED STRENGTH, HAVE BEEN ADEQUATELY BRACED, AND/OR HAVE OTHER INTEGRAL STRUCTURAL ELEMENTS IN PLACE WHICH ARE INTENDED TO RESIST THE LATERAL EARTH

LATERAL LOAD RESISTING SYSTEM: ALL LATERAL LOAD RESISTANCE AND STABILITY IN THE COMPLETED STRUCTURE IS PROVIDED BY: N-S DIRECTION: STEEL MOMENT FRAMES E-W DIRECTION: STEEL MOMENT FRAMES

STEEL STABILITY: STRUCTURAL STEEL FRAMING INDICATED IN THESE PLANS REQUIRES INTERACTION WITH NON-STRUCTURAL STEEL ELEMENTS FOR STRENGTH AND/OR STABILITY. SEE PLANS FOR SPECIFIC LOCATIONS OF THESE NON-STRUCTURAL STEEL ELEMENTS WHICH ARE LISTED BELOW: METAL ROOF DECKING

DETAILS ON THE DRAWINGS INDICATED AS "TYPICAL" APPLY IN ALL AREAS WHERE CONDITIONS SIMILAR TO THE DETAIL OCCUR.

THE STRUCTURAL DRAWINGS ARE NOT INTENDED FOR USE AS SHOP ERECTION DRAWINGS. REPRODUCTION OF THESE DRAWINGS IN LIEU OF PREPARATION OF SHOP ERECTION DRAWINGS SIGNIFIES THE USERS' ACCEPTANCE THAT ALL INFORMATION SHOWN IS CORRECT AND APPROPRIATE FOR SHOP DRAWINGS AND THAT THE USER WILL BE FULLY RESPONSIBLE FOR EXPENSES THAT MAY OCCUR

UNLESS SPECIFICALLY NOTED, THERE ARE NO PROVISIONS MADE FOR FUTURE FLOORS, ROOFS, OR OTHER LOADS.

# COORDINATION/VERIFICATION:

CHECK AND VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH ANY PHASE OF THE

ANY PROPRIETARY STRUCTURAL SYSTEMS THAT ARE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE. AND ERECTION IN ACCORDANCE WITH THE INSTRUCTIONS PREPARED BY THE SUPPLIER.

CROSS REFERENCE STRUCTURAL DRAWINGS WITH MECHANICAL AND ELECTRICAL DRAWINGS, AND WITH THE ACTUAL EQUIPMENT SUPPLIED TO THE PROJECT, FOR THE LOCATION AND SIZE OF ALL SLAB OPENINGS, SLEEVES, INSERTS, FLOOR DEPRESSIONS, BLOCK-OUTS, CURBS, ANCHORS, BOLTS, ETC. REQUIRED FOR

ALL REQUIRED MECHANICAL OPENINGS THROUGH SLABS. WALLS, FLOOR DECK, ETC.. AND SUPPORT OF ALL MECHANICAL FOUIPMENT. OPENINGS SHALL NOT BE PERMITTED THROUGH BEAMS UNLESS SPECIFICALLY DETAILED BY THE ENGINEER. REFER TO ARCHITECTURAL DRAWINGS FOR ALL SURFACE FINISHES, DIMENSIONS,

### CONCRETE/REINFORCING:

CONCRETE BATCH DESIGN(S) SHALL BE PROPORTIONED AND PRODUCED IN ACCORDANCE WITH A.C.I. 318, IN PARTICULAR CHAPTER 5, AND A.C.I. 301. MIX AND DELIVER IN ACCORDANCE WITH ASTM C94. SLUMP REQUIREMENTS SLOPING SURFACES MIN. 1 IN. / MAX. 4 IN. FOUNDATIONS CONCRETE W/ PLASTICIZERS MAX. 8 IN. MIN. 1 IN. / MAX. 5 IN. OTHER CONCRETE

CONCRETE EXPOSED TO WEATHER - 5% AIR ENTRAINMENT ADMIXTURES SUBMIT AS REQUIRED FOR APPROVAL FLY ASH MAX. 20% OF CEMENT CONTENT

CONSTRUCTION JOINTS MUST HAVE PRIOR REVIEW BY THE ENGINEER. ALL CONTINUOUS REINFORCING SHALL BE CARRIED THROUGH THE JOINT. SPANNING MEMBERS NEAR MIDSPAN

CONCRETE TO CONCRETE COLD JOINTS - PROVIDE 1/4" INTENTIONALLY ROUGHENED SURFACE AT ALL HORIZONTAL JOINTS.

EXPOSED CORNERS: PROVIDE A 3/4" CHAMFER AT ALL EXPOSED CONCRETE

CURING: CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF SEVEN DAYS AFTER ITS PLACEMENT. IF FORMWORK IS REMOVED PRIOR TO SEVEN DAYS, APPLY MOIST CURING TO NEWLY EXPOSED SURFACES. APPROVED CURING COMPOUNDS MAY BE USED IN LIEU OF MOIST CURING.

MIDWAY BETWEEN COLUMNS

DO NOT ALIGN WITH FOOTING JOINTS

REINFORCING BAR WEI DING: ABSOLUTELY NO WEI DING OF REINFORCING BARS OR TORCHING TO BEND REINFORCING BARS SHALL BE ALLOWED WITHOUT THE SPECIFIC APPROVAL OF THE ENGINEER.

MINIMUM CONCRETE CLEAR COVER: TOP FOOTING BOTTOM SIDES 1-1/2" FOUND. WALLS TOP BOTTOM SIDES 1-1/2" GIRDERS/BEAMS/JOISTS TOP 1-1/2" GRADE BEAMS BOTTOM EXTERIOR WALLS EXTERIOR FACE INTERIOR FACE INTERIOR WALLS 3/4" (TOL. = -0 + 1/4) INTERIOR SLABS TOP 1-1/2" (2" IF #6 BAR OR EXTERIOR SLABS TOP EXTERIOR INTERIOR

OTHER COVER REQUIREMENTS SHALL BE AS NOTED IN APPLICABLE DETAILS. BAR SUPPORT ACCESSORIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST

A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES BEAM REINFORCING ON BAR BOLSTERS @ 4 FT. O.C. MAX. SLAB REINFORCING ON BAR BOLSTERS @ 4 FT. O.C. MAX. CONC. EXPOSED TO VIEW PLASTIC COATED OR GALV. LEGS CONCRETE W/ SANDBLAST FINISH - STAINLESS STEEL SUPPORTS NO ROCKS, CLAY TILE, OR CLAY BRICK SHALL BE USED TO SUPPORT

MECHANICAL AND ELECTRICAL CONDUIT IN SUSPENDED SLABS SHALL RUN UNDER TOP LAYER OF SLAB REINFORCING. PROVIDE A MINIMUM OF 1-1/2" CLEAR BETWEEN CONDUITS AND BETWEEN REINFORCING AND ADJACENT CONDUITS PARALLEL TO REINFORCING. IF MAXIMUM SIZE OF CONDUIT EXCEEDS ONE THIRD OF THE SLAB DEPTH, CONTACT ENGINEER FOR ADDITIONAL FRAMING OR REINFORCING.

NON-METALLIC, SHRINKAGE-RESISTANT GROUT PRE-MIXED, NON-METALLIC, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING SILICA SANDS, PORTLAND CEMENT, SHRINKAGE-COMPENSATING AGENTS, PLASTICIZING AND WATER-REDUCING

MINIMUM COMPRESSIVE STRENGTH @ 48 HOURS 2400 PSI MINIMUM COMPRESSIVE STRENGTH @ 28 DAYS ACCEPTABLE PRODUCTS: CRYSTEX; L&M CONSTRUCTION CHEM. MASTERFLOW 713; MASTER BUILDERS STAR GROUT; U.S. GROUT CORP.

UPCOM: UPCO CHEM. DIVISION, USM CORP.

REINFORCING SHOP DRAWINGS: REINFORCING SUPPLIER SHALL PROVIDE COMPLETE PLACEMENT AND FABRICATION DRAWINGS FOR ALL REINFORCING INCLUDING THE LOCATION AND SIZE OF ALL ACCESSORIES AND SUPPORTS.

VOID FILL: FOAM MATERIAL USED FOR VOID FILL SHALL BE RIGID CELLULAR, EXTRUDED POLYSTYRENE INSULATION COMPLYING WITH ASTM C578, TYPE IV, 25 PSI COMPRESSIVE STRENGTH MINIMUM.

REINFORCING.

BASIS OF DESIGN: THE FOUNDATION SYSTEM DESIGN IS BASED ON THE PRESUMPTIVE LOAD-BEARING VALUES OF SOILS RECOMMENDED IN THE INTERNATIONAL BUILDING CODE (IBC).

SPREAD FOOTINGS SHALL BE PLACED ON NEAT, CLEAN AND DRY EXCAVATIONS. EXTREME CARE SHALL BE TAKEN WHEN EXCAVATING NEAR THE BEARING SURFACE. FOOT TRAFFIC SHALL BE KEPT TO A MINIMUM NECESSARY TO PLACE THE FOOTING REINFORCEMENT AND CONCRETE.

THE CONTRACTOR SHALL PROVIDE FOR ADEQUATE DRAINAGE OF SURFACE WATER AWAY FROM THE STRUCTURE AND EXCAVATED AREAS DURING CONSTRUCTION. THIS INCLUDED NECESSARY PUMPING, TRENCHING, BACKFILL AND/OR DIKE CONSTRUCTION.

PREFORMED PLASTIC WATERSTOP: PRODUCED FROM BLENDS OF REFINED HYDROCARBON RESINS AND PLASTICIZING COMPOUNDS REINFORCED WITH INERT MINERAL FILLER, AND SHALL CONTAIN NO SOLVENTS, IRRITATING FUMES OR OBNOXIOUS ODORS, COMPLYING WITH FEDERAL SPECIFICATION SS-S210A.

# GRADE SUPPORTED SLABS:

REINFORCED CONCRETE SLAB ON GRADE SLAB THICKNESS: SEE PLAN

> REINFORCING: SEE PLAN VAPOR BARRIER: SEE PLAN

GRANULAR SUBBASE UNDER SLAB-ON-GRADE: MINIMUM THICKNESS: SEE PLAN COMPACTION 95% (+/-2%) GRADATION REQUIREMENTS: 100% PASSING THE 3/4" SIEVE LESS THAN 15% PASSING THE 100 SIEVE

SELECT FILL, WHERE REQUIRED TO ACHIEVE FINAL GRADE; CLEAN, INORGANIC, LOW-PLASTICITY SILT OR LEAN CLAY WITH THE FOLLOWING PROPERTIES: MAXIMUM LIQUID LIMIT (LL) 5 TO 18 PLASTICITY INDEX (PI) RANGE MOISTURE CONTENT (% OF OPTIMUM) +3%, -2%MAXIMUM LOOSE LIFT

LESS THAN 2% PASSING THE 200 SIEVE

COMPACTION 95% (+/-2%) EXISTING SUBGRADE: HEAVILY ROOT INFESTED TOPSOIL, PAVING, AND DEBRIS SHOULD BE STRIPPED AND DISCARDED. REMAINING EARTH SHALL BE SCARIFIED TO A DEPTH OF 12" AND RECOMPACTED TO AT LEAST 95% STANDARD PROCTOR DENSITY.

SLAB SURFACE SEALER: SEALHARD BY L & M CONSTRUCTION CHEMICALS INC., OR EQUAL. APPLY AT MANUFACTURERS RECOMMENDED RATE.

COMPACTION (% OF MAXIMUM DRY DENSITY) SHALL BE DETERMINED USING ASTM D-698 STANDARD PROCTOR TEST

CRACK CONTROL JOINTS (WHETHER CONSTRUCTION JOINTS OR SAWED JOINTS) IN SLABS ON GRADE SHALL OCCUR AS SHOWN AND ACROSS ALL DOOR OPENINGS. LOCATE JOINTS AT RE-ENTRANT CORNERS OF SLABS. MAXIMUM SPACING OF CONTROL JOINTS: 12 FEET

CURING: CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF SEVEN DAYS AFTER ITS PLACEMENT. APPROVED CURING COMPOUNDS MAY BE USED IN LIEU OF MOIST CURING. CURING COMPOUNDS SHALL BE NON-RESIDUAL TYPE AND COMPATIBLE WITH SPECIFIED SEALER.

SEAL ALL EXPOSED CONSTRUCTION/CRACK CONTROL JOINTS. SEALANT: ASTM D1190; HOT APPLIED RUBBER, SYNTHETIC RUBBER, ASPHALT, OR POLYMER BASED ASPHALT COMPOUND.

### STRUCTURAL STEEL:

BEARING CONNECTIONS: UNLESS OTHERWISE NOTED, ALL BEAM CONNECTIONS SHALL BE SIMPLE FRAMED SHEAR BEARING CONNECTIONS IN ACCORDANCE WITH THE AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING A.S.T.M. A325 OR A490 BOLTS."

DESIGN OF CONNECTIONS: BEAM CONNECTIONS SHALL BE AS DETAILED ON THE PLANS. ALTERNATIVE CONNECTIONS, DESIGNED BY A LISCENSED ENGINEER FOR THE FABRICATOR, MAY BE UTILIZED PROVIDED THE ALTERNATIVE CONNECTION PROVIDES THE SAME LOAD CARRYING CAPACITY OF THE ORIGINAL DESIGN.

STEEL PROTECTION: ALL STRUCTURAL STEEL SHALL BE CLEANED PER SSPC SP-2 HAND TOOL CLEANING OR SP-3 POWER TOOL CLEANING AND SHOP PAINTED WITH ONE COAT OF RED OXIDE PRIMER (PAINT TYPE SSPC-PAINT 13). TOUCH UP SCARRED AREAS WITH THE SAME PAINT AFTER ERECTION.

STEEL PROTECTION: WHERE STEEL IS INDICATED AS GALVANIZED, PROVIDE HOT-DIPPED GALVANIZED SURFACE PER ASTM A525, CLASS G90. WHERE WELDING OR OTHER CONSTRUCTION OPERATIONS DAMAGE GALVANIZING, PROVIDE ZINC CHROMATE-TYPE TOUCH UP PAINT TO DAMAGED AREA.

SHOP DRAWINGS: STEEL FABRICATOR SHALL PROVIDE COMPLETE ERECTION AND FABRICATION DRAWINGS SHOWING ALL MEMBERS AND CONNECTIONS.

WELDING: ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS PER AWS STANDARD QUALIFICATION PROCEDURES. COMPLETE PENETRATION WELDS: COMPLETE PENETRATION WELDED CONNECTIONS

SHALL BE PROVIDED WHERE INDICATED. A MINIMUM OF 10% OF ALL FIELD AND SHOP FULL PENETRATION WELDS SHALL BE TESTED BY X-RAY OR ULTRASONIC PROCEDURES. (SEE SPECIFICATIONS FOR OTHER TESTING.)

# METAL DECKING:

DECK GOVERNING CRITERIA: THE DESIGN, FABRICATION, AND ERECTION OF METAL DECKING SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF THE SDI SPECIFICATIONS AND THE SDI DIAPHRAGN DESIGN MANUAL.

DECK ENDS: MAY BE BUTTED OR LAPPED OVER SUPPORTS. LAPS SHALL BE A MINIMUM OF 3" ROOF DECK CONNECTIONS: ROOF DECKING SHALL BE CONNECTED TO THE

STRUCTURE AS INDICATED IN THE ROOF DECK SCHEDULE. MAINTAIN OVERALL STRUCTURAL BRACING UNTIL ALL DECKING HAS BEEN INSTALLED. DECK PROTECTION: PAINTED DECKING SHALL BE COVERED WITH A WATERPROOF

COVERING DURING SHIPPING AND SHALL BE STORED OFF THE GROUND AND COVERED WITHA WATERPROOF COVERING WHILE ON SITE.

HEADED CONCRETE ANCHORS (HCA): AUTOMATICALLY END WELDED IN THE SHOP OR FIELD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ANCHOR WELDS SHALL BE TESTED PER AWS SECTION 7.7.

POST-INSTALLED MECHANICAL ANCHORS: SEE SCHEDULE INSTALL USING MINIMUM TORQUE, EMBEDMENTS, EDGE DISTANCES AND SPACING (UNLESS OTHERWISE NOTED) AS RECOMMENDED BY THE ANCHOR MANUFACTURER. POST-INSTALLED ADHESIVE ANCHORS: SEE SCHEDULE

INSTALLATION TO MEET MANUFACTURER'S RECOMMENDATIONS (UNLESS NOTED OTHERWISE) MIN. EMBEDMENTS, EDGE DISTANCES, SPACING, PROCEDURES, AND CURING TIME PRIOR TO LOADING.

POST-INSTALLED ANCHORS SHALL BE LOCATED PER THE DETAILS. AVOID CUTTING OR DAMAGING EXISTING REINFORCING. SHOULD LOCATIONS OF DRILLED HOLES BE FOUND DIRECTLY ALIGNED WITH REINFORCING BARS, NOTIFY

POST-INSTALLED ANCHORS LOCATED IN PRESTRESSED OR POST-TENSIONED MEMBERS SHALL NOT BE DRILLED UNTIL EXACT LOCATIONS OF TENDONS OR STRANDS ARE DETERMINED USING NON-DESTRUCTIVE METHODS.

ENGINEER FOR NECESSARY ADJUSTMENTS TO THE DESIGN.

STEEL CONSTRUCTION CONTINUOUS | PERIODIC MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS: A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN APPROVED CONSTRUCTION DOCUMENTS B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. INSPECTION OF HIGH-STRENGTH BOLTING: A. BEARING-TYPE CONNECTIONS. MATERIAL VERIFICATION OF STRUCTURAL STEEL: A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION B. MANUFACTURER'S CERTIFIED MILL REPORTS. MATERIAL VERIFICATION OF WELD FILLER MATERIALS: A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS. B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED INSPECTION OF WELDING: A. STRUCTURAL STEEL: 1. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS. 2. MULTIPASS FILLET WELDS. Χ SINGLE-PASS FILLET WELDS > 5/16" 4. SINGLE-PASS FILLET WELDS <= 5/16" FLOOR AND ROOF DECK WELDS. B. REINFORCING STEEL: 1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706. SHEAR REINFORCEMENT. 4. OTHER REINFORCING STEEL. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS: A. DETAILS SUCH AS BRACING AND STIFFENING. B. MEMBER LOCATIONS. C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION. CONTINUOUS | PERIODIC CONCRETE CONSTRUCTION INSPECTION OF REINFORCING STEEL AND PLACEMENT. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE

AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND

VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE

VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF

PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED

INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.

SPECIAL INSPECTIONS SHALL BE PROVIDED FOR THE WORK IN ACCORDANCE WITH IBC CHAPTER 17. CONTRACTOR SHALL NOTIFY AND ACCOMMODATE

THE APPLICABLE INSPECTOR DURING APPROPRIATE PHASES OF THE WORK AS REQUIRED FOR EACH TYPE OF INSPECTION.

SPECIAL INSPECTIONS (IBC 2006)

	GRAD	DE BEA	M FOOTING SCHEDULE	
MARK	WIDTH	DEPTH	LONGITUDINAL REINFORCING	STIRRUPS
GB1-4	1'-4"	4'-0"	(2) #6 BARS BOTT., (2) #6 BARS TOP	#4 @ 24" OC

PAD FOOTING SCHEDULE					
MARK PLAN SIZE DEPTH REINFORCING					
3'-0" x 3'-0"	4'-0"	(4) #6 BARS EA. WAY, TOP & BOTTOM			
	PLAN SIZE	PLAN SIZE DEPTH			

METAL DECKING SCHEDULE										
LOCATION	DEPTH (IN.)	GAGE	MIN. lx (IN.4/FT)	MIN. Sx (IN.3/FT)	FINISH	CONN. TO SUPPORTING MEMBERS (5/8" DIA. PUDDLE WELD)	SIDE LAPS (#10 SCREWS)	REMARKS		
VESTIBULE ROOF	1 1/2	22	lp = 0.155 ln = 0.183	Sp = 0.186 Sn = 0.192	GALVANIZED	36" DECK WIDTH	1 SIDE LAP FASTENERS #10 TEK SCREWS — EA. JOIST SPACE	TYPE "B"		

LOADS HAVE BEEN INCREASED.

PROPERLY.

VERIFYING USE OF REQUIRED DESIGN MIX.

AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF CONCRETE.

PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.

INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.

AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.

VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.

VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.

INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.

NOTES: GALVANIZING SHALL CONFORM TO ASTM A525, CLASS G-60. SUPPORT CONNECTIONS SHALL BE MADE WITH 5/8" WELDS (TYP. U.N.O.).

CHITECT/E	NGINEERS	•		

SCHEMMER ARCHITECTS | ENGINEERS | PLANNERS

Drawing Title GENERAL NOTES AND SCHEDULES Project Title Department of Veterans Affairs Medical Center - Convert File Room Bldg 113 + Relocate PCT to Bldg 148

Ft. Meade, South Dakota

Project Number Office of 568-12-114 Construction **Building Number** 113 + 148 and Facilities Drawing Number Management

CONTINUOUS | PERIODIC

KMW JJH 10/22/2012

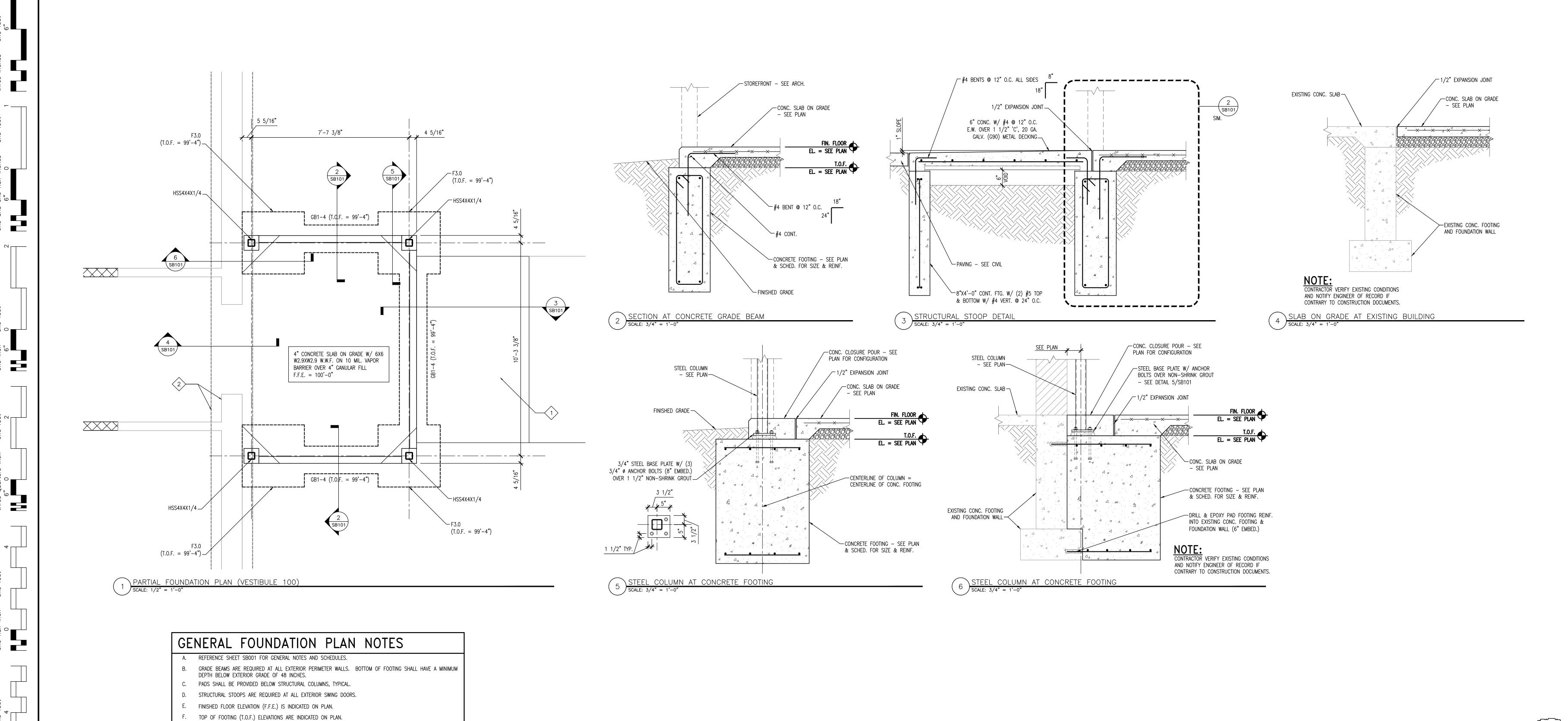
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TSA PROJECT NO: 06054,011

Approved: Project Director

**SB001** Dwg. **24** of 37

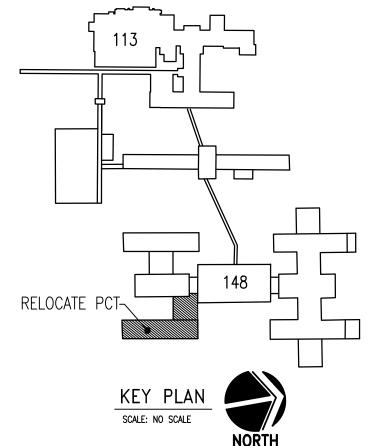
Department of Veterans Affairs



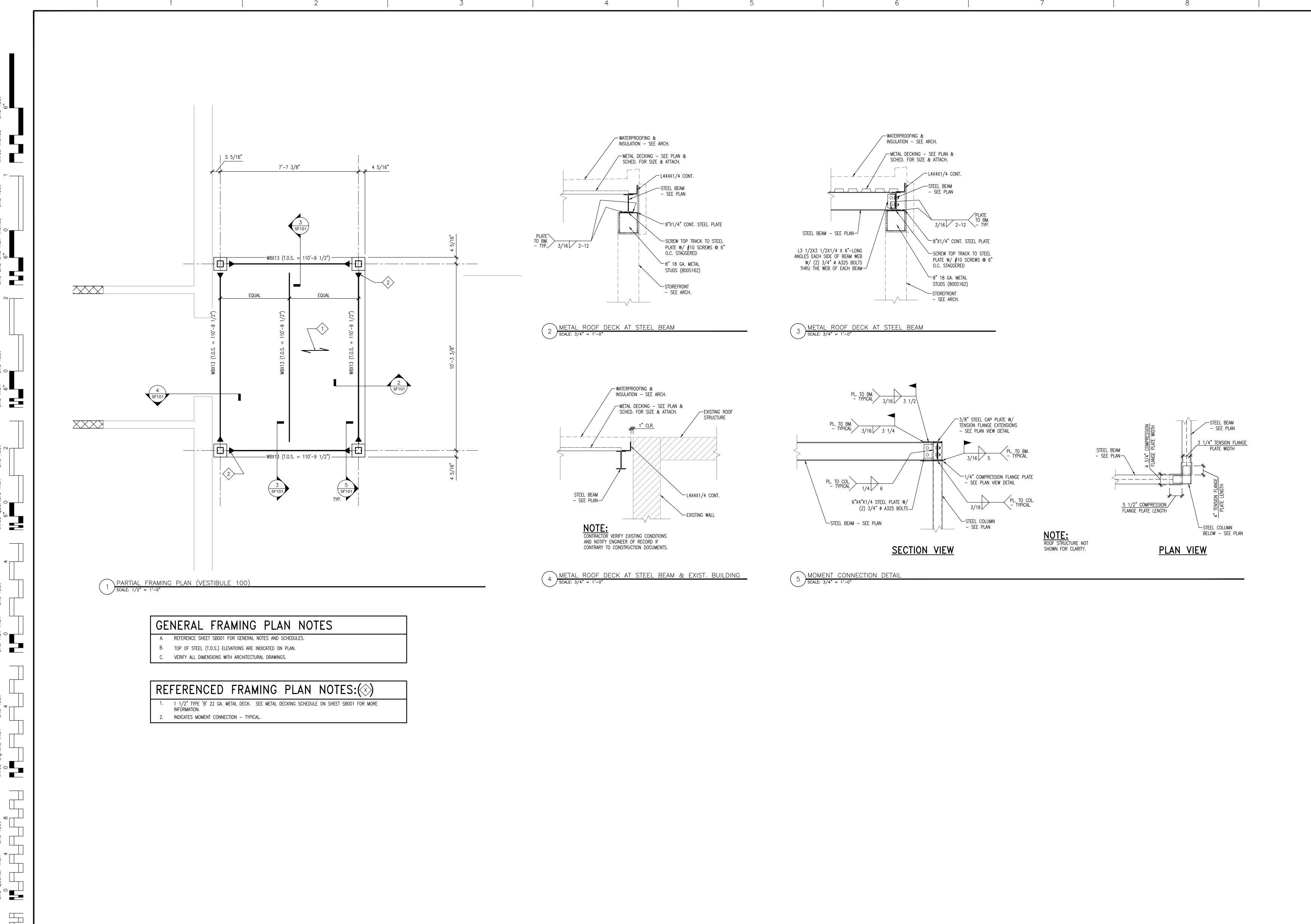
# REFERENCED FOUNDATION PLAN NOTES:(🛞)

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

STRUCTURAL STOOP — SEE DETAIL INDICATED ON PLAN. REFERENCE ARCHITECTURAL DRAWINGS FOR STOOP 2. EXISTING FOUNDATION WALL AND FOOTING.



	CONSULTANTS:	ARCHITECT/ENGINEERS:	VESTIBULE FOUNDATION PLAN AND DETAILS	Project Title Department of Medical Center		airs	Project Number 568-12-114	Office of Construction
		SCH=MM=R		Bldg 113 + Relo	cate PCT to	Bldg 148	113 + 148	and Facilities
		ARCHITECTS   ENGINEERS   PLANNERS	Approved: Project Director	Ft. Meade, Sou	7		Drawing Number SB101	Management
Revisions: Date		TSA PROJECT NO: 06054,011		10/22/2012	Checked <b>JJH</b>	Drawn KMW	Dwg. <b>25</b> of 37	Department of Veterans Affairs



CONSULTANTS:	ARCHITECT/ENGINEERS:	Drawing Title  VESTIBULE FRAMING PLAN AND DETAILS	Project Title  Department of Veterans Affairs		Project Number 568-12-114  Building Number 113 + 148  Drawing Number  SF101	Office of Construction and Facilities Manageme	
	CCUERARAED		Medical Center - Convert File Room Bldg 113 + Relocate PCT to Bldg 148				
	SCHEMMER  ARCHITECTS   ENGINEERS   PLANNERS	Approved: Project Director	Location Ft. Meade, South Dakota				

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KEY PLAN

SCALE: NO SCALE