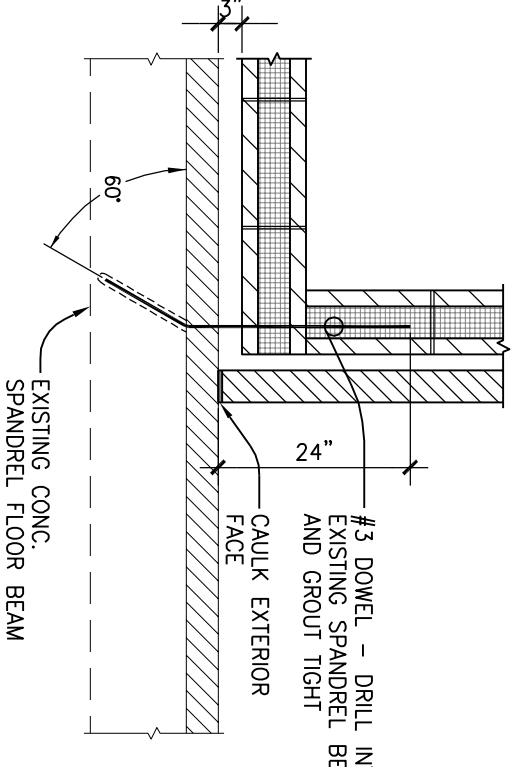
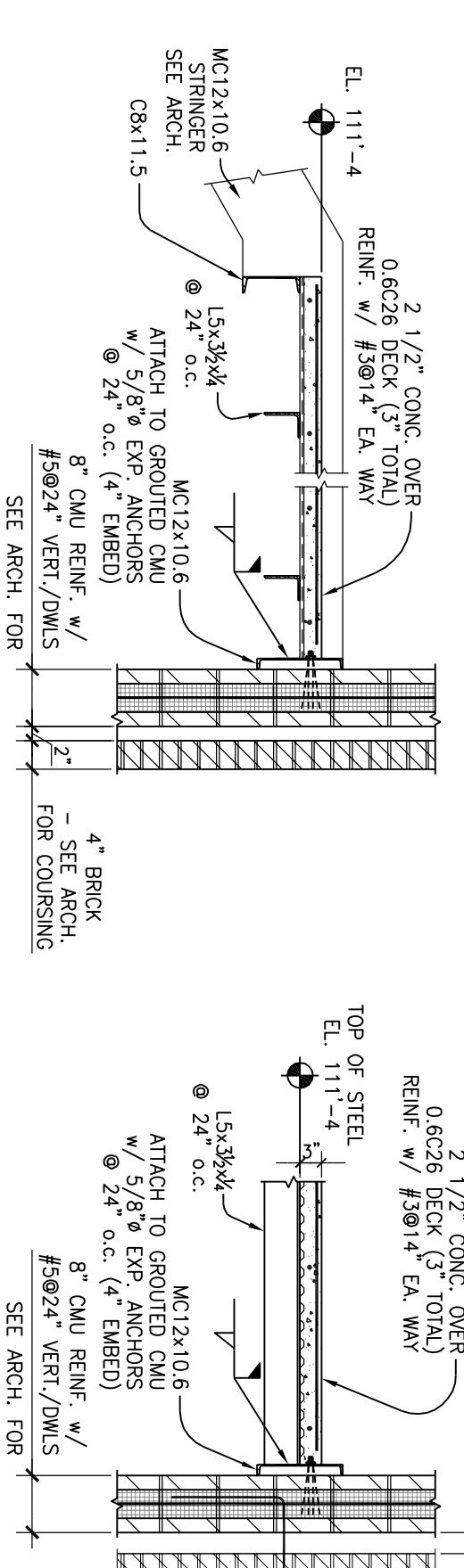
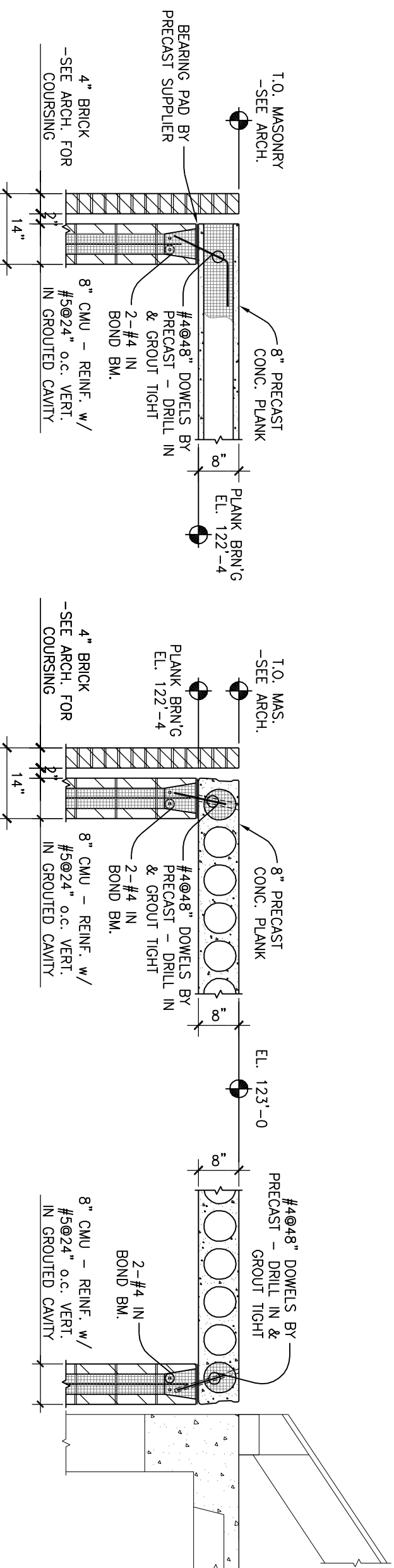
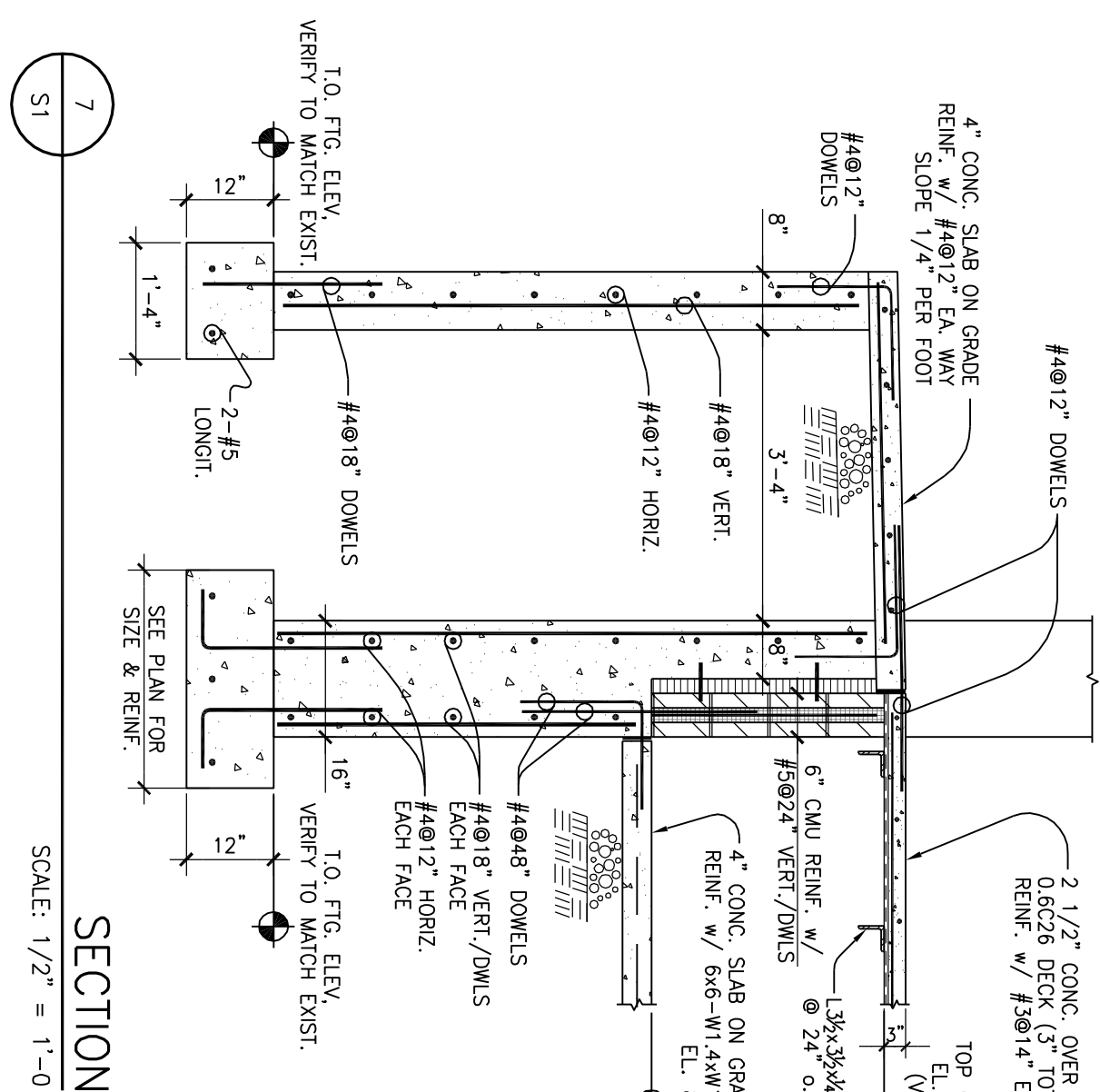


## GENERAL STRUCTURAL NOTES

- |     |   |
|-----|---|
| 1.  | Design Codes Used:<br>BSI<br>BSI 8006<br>ASCE<br>Eurocode   |
| 2.  | Design Life: 100 years<br>Refr Snow Load:<br>$P_s = 50 \text{ PSF} + D/Ht$<br>$P_g = 40 \text{ PSF}$<br>$S_e = 11$<br>$S_o = 11$<br>Site-<br>Wind<br>$C_t = 1.0$<br>$I_w = 1.15$<br>90 mph Basic Wind Speed<br>Intervl Pressure Coefficient 10.18   |
| 3.  | Design Strengths Used:<br>Concrete<br>- Slabs on Grade<br>- Columns<br>- Exterior exposed<br>Steel W Shapes<br>- Tubes<br>- Angles, Channels, Bars<br>Reinforcing Steel<br>Soil Bearing Pressure<br>Footings<br>All walls and piers to center on footing unless otherwise noted.<br>All footing elevations are given to the top of footings.<br>Foundation construction joints to be as follows:  |
| 4.  | FOOTINGS to rest on undisturbed soil or engineered substrate.<br>ALL WALLS AND PIERS TO CENTER ON FOOTING UNLESS OTHERWISE NOTED.<br>ALL FOOTING ELEVATIONS ARE GIVEN TO THE TOP OF FOOTINGS.<br>FOUNDATION CONSTRUCTION JOINTS TO BE AS FOLLOWS:   |
| 5.  | OPENINGS IN A concrete FOUNDATION WALLS shall be reinforced with steel reinforcement extending 2'-0" past each face of the opening unless otherwise noted.  |
| 6.  | CONCRETE CONFINEMENT for reinforcing shall be as follows:<br>Footings<br>Columns<br>Walls<br>Slabs on grade<br>Structural Slobs<br>PROVIDE BAR SUPPORTS AND SPACERS in accordance with the ACI detailing Manual.  |
| 7.  | REINFORCING STEEL to be bent and placed in accordance with ACI Code. All spaces to be 30 bar diameters.   |
| 8.  | PROVIDE 25 #steel tie of L 3 x 7/23 1/2x5/16 to be used at the Architect/Drywall's discretion.  |
| 9.  | MASONRY TO CONSIST OF ASTM C90 Hollow Clay, Grade N, nominal weight, solid-drum shell units conforming to ASTM S2, consisting of staggered longitudinal rows with cross void width (interior of ribs) not exceeding 16" o.c., cold-water shelf units conforming to ASTM S2, consisting of staggered longitudinal rows with cross void width (interior of ribs) not exceeding 16" o.c., all masonry units formed from 1 1/4" square rods of 1/4" points and 5-square rods at 1/4" points. Provide full joint coats of Masonry Mortar of every 16" in wall thickness. |
| 10. | MORTAR TO BE Type S or Type M below grade and Type N elsewhere.   |
| 11. | PROVIDE TEMPORARY BRACING during masonry erection. Monitor in place until Bracing Structure provides permanent bracing.   |
| 12. | At MASONRY CONTROL Joints DO NOT continue horizontal joint reinforcement across control joints. In EXTERIOR Walls provide vertical joint panel at 24' feet on center maximum. INTERIOR Walls provide vertical joint panel at 30' feet on center maximum.  |
| 13. | All STRUCTURAL STEEL will be fabricated and welded in accordance with the AISC Code. Connections not detailed are to be designed by the fabricator in accordance with the AISC Fabricator of Structural Steel Detailing Codebook. Welds shall comply with AWS D1.1 and be tested with 1/2" thick compression test strength bolts (ASTM A325).   |
| 14. | SEE MECHANICAL, ELECTRICAL & PLUMBING DRAWINGS for all ratings and inserts not shown on the structural drawings. All opening sizes and locations to be verified with mechanical and electrical contractors.   |
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DETAIL  
1/2" = 1'-0"

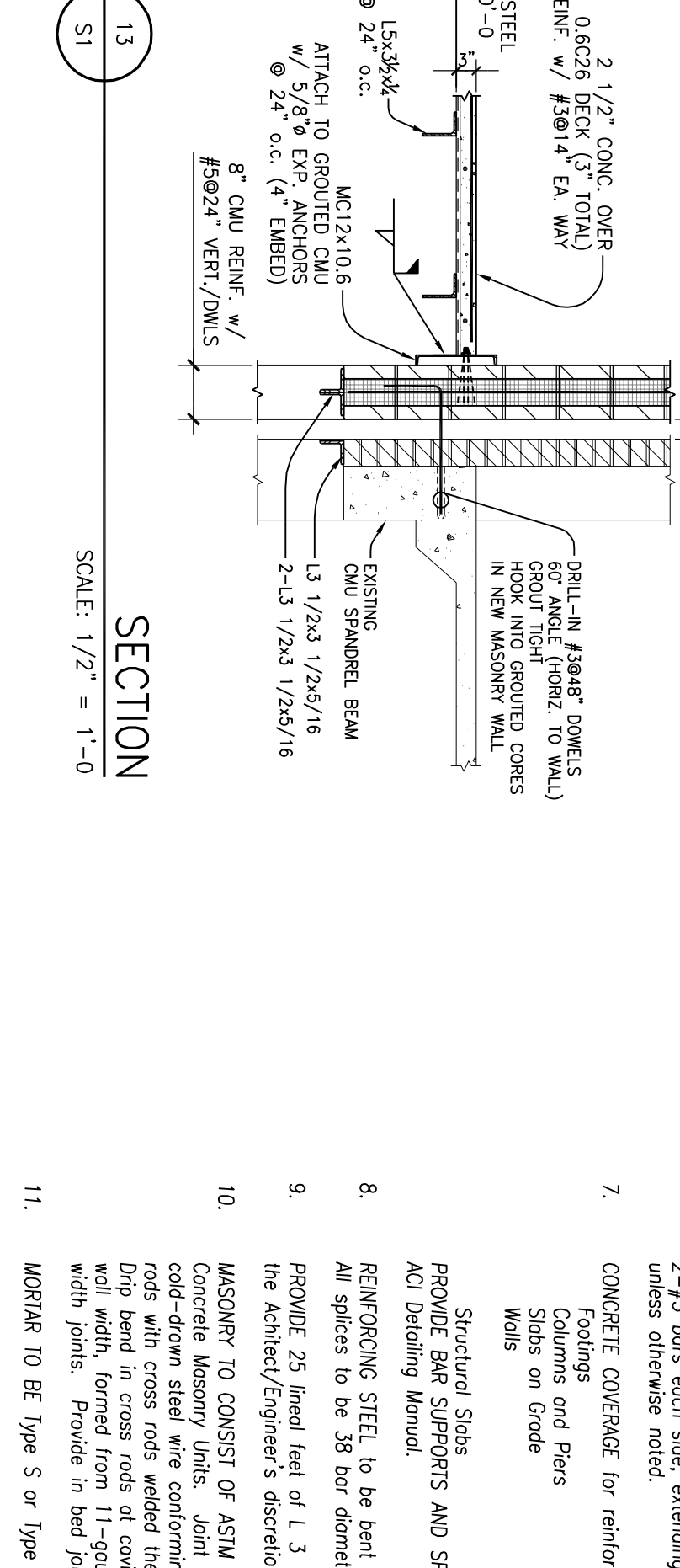
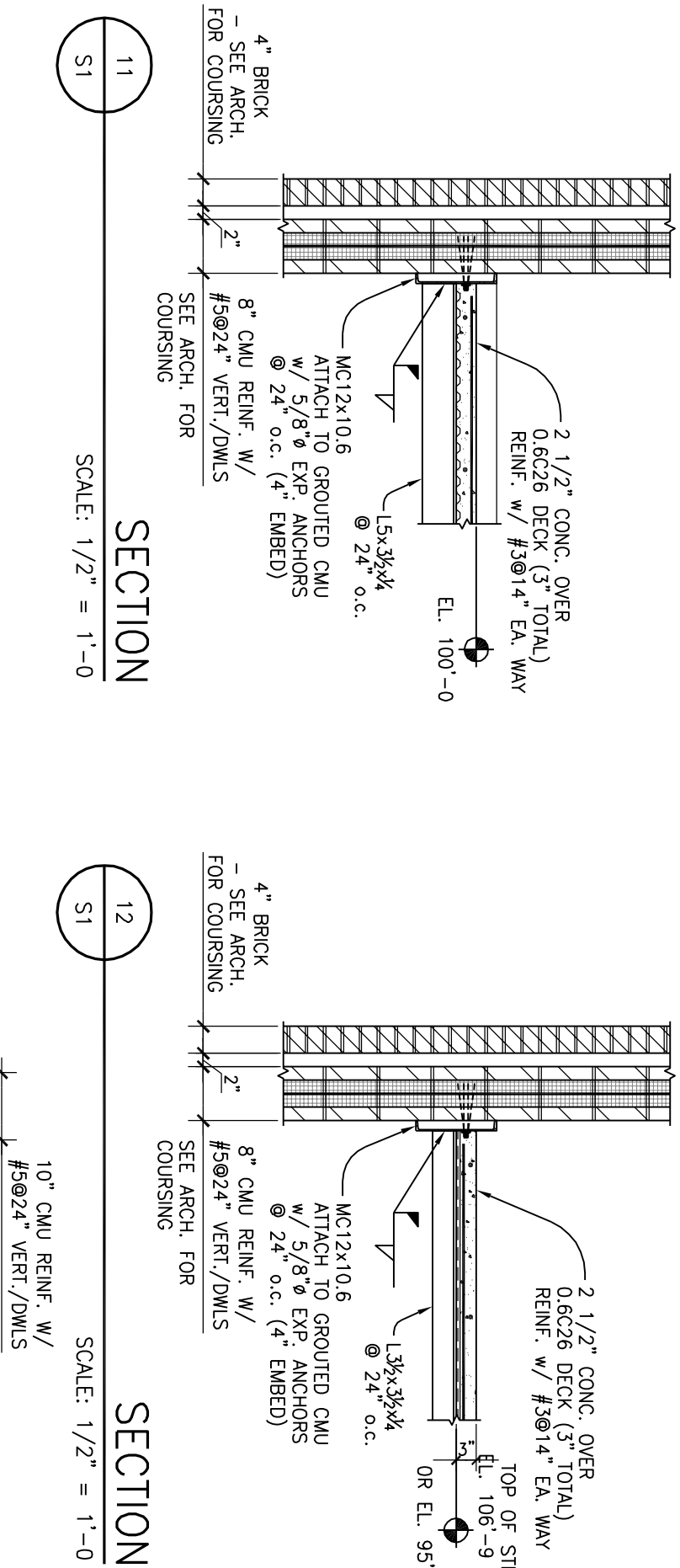
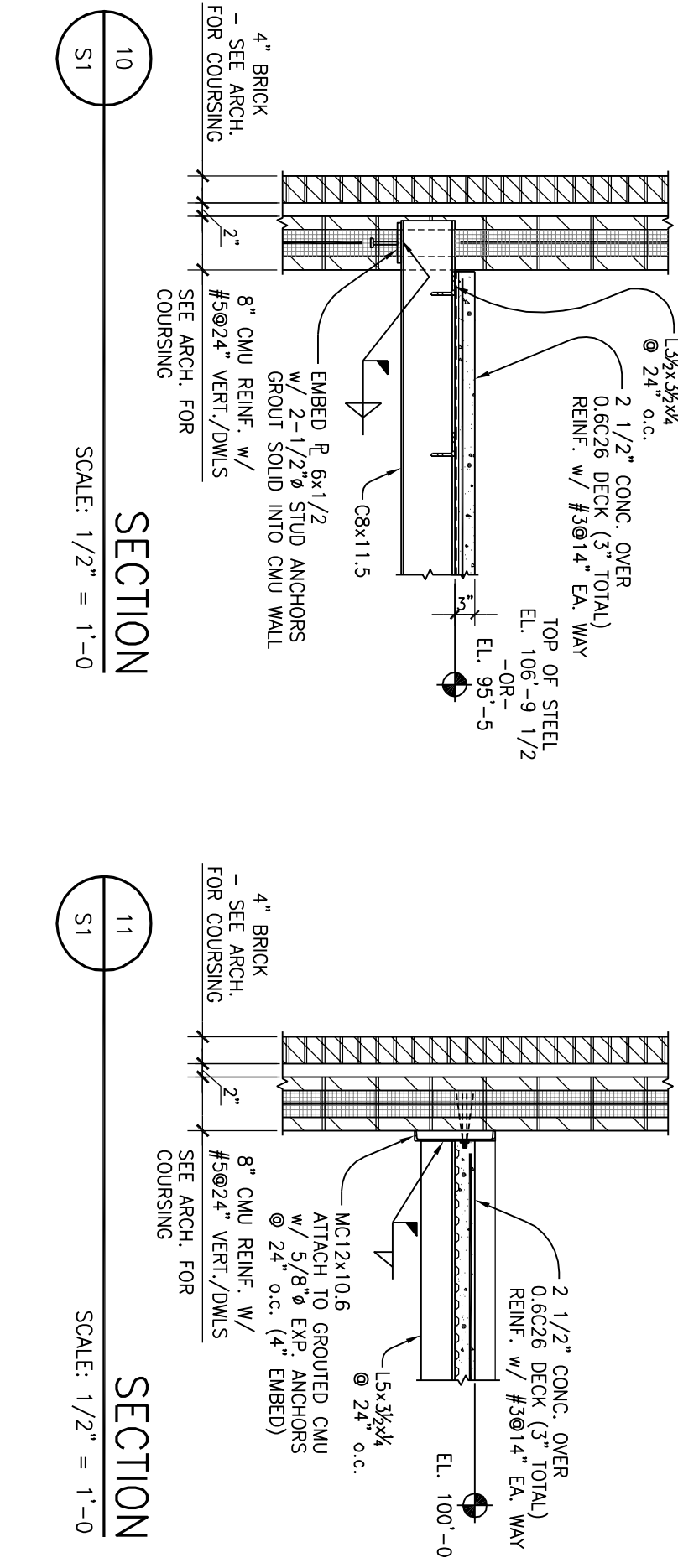
**SECTION**  
**1/2" = 1'-0"**

## SECTION

## SECTION

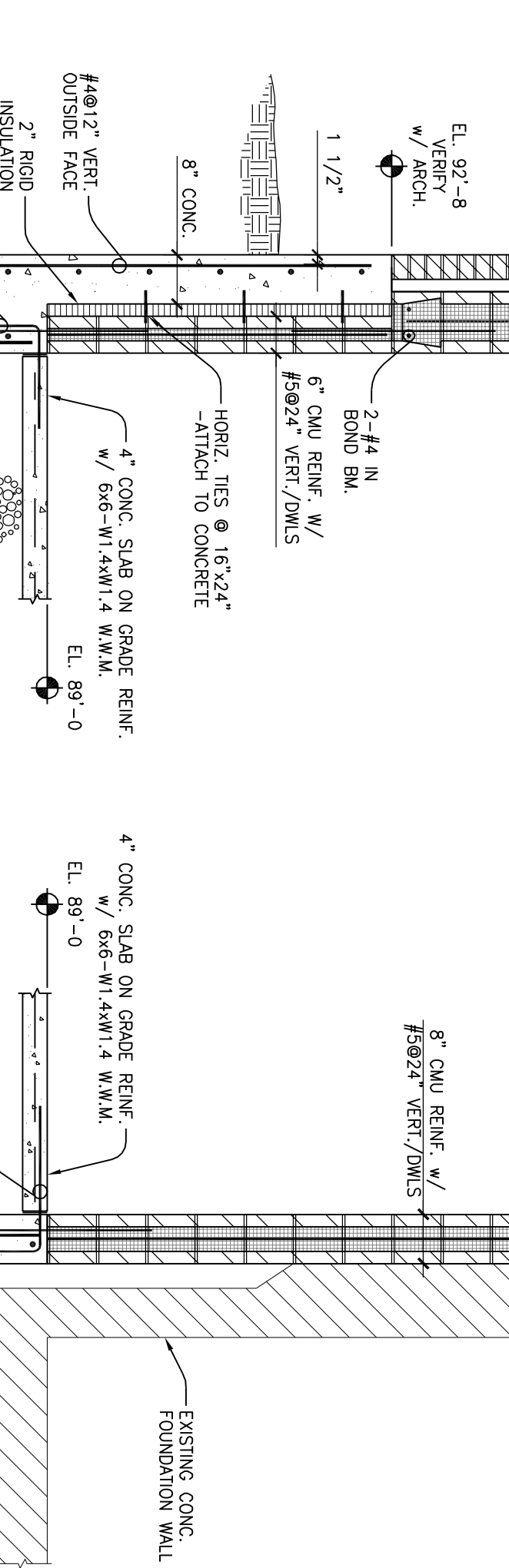
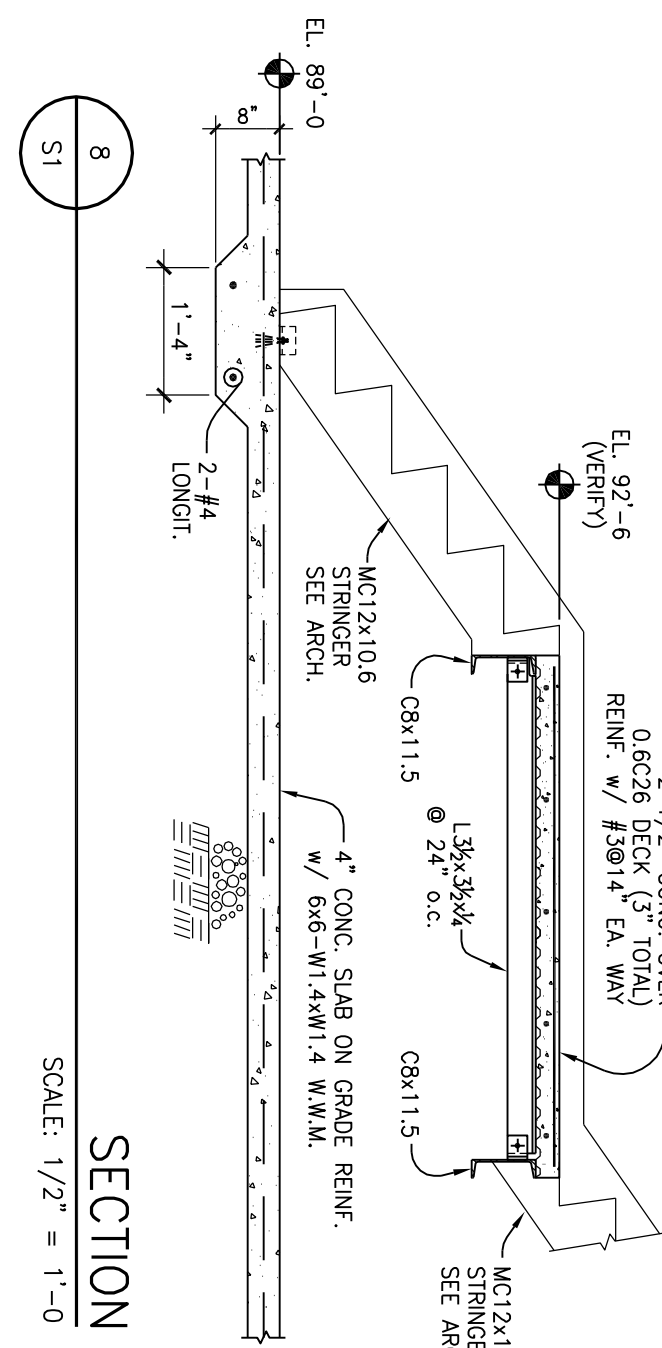
**SECTION**  
**1/2" = 1'-0"**

## SECTION



## SECTION

2 1/2" CON  
0.6C26 DECK (3  
REINF. w/ #3@14"



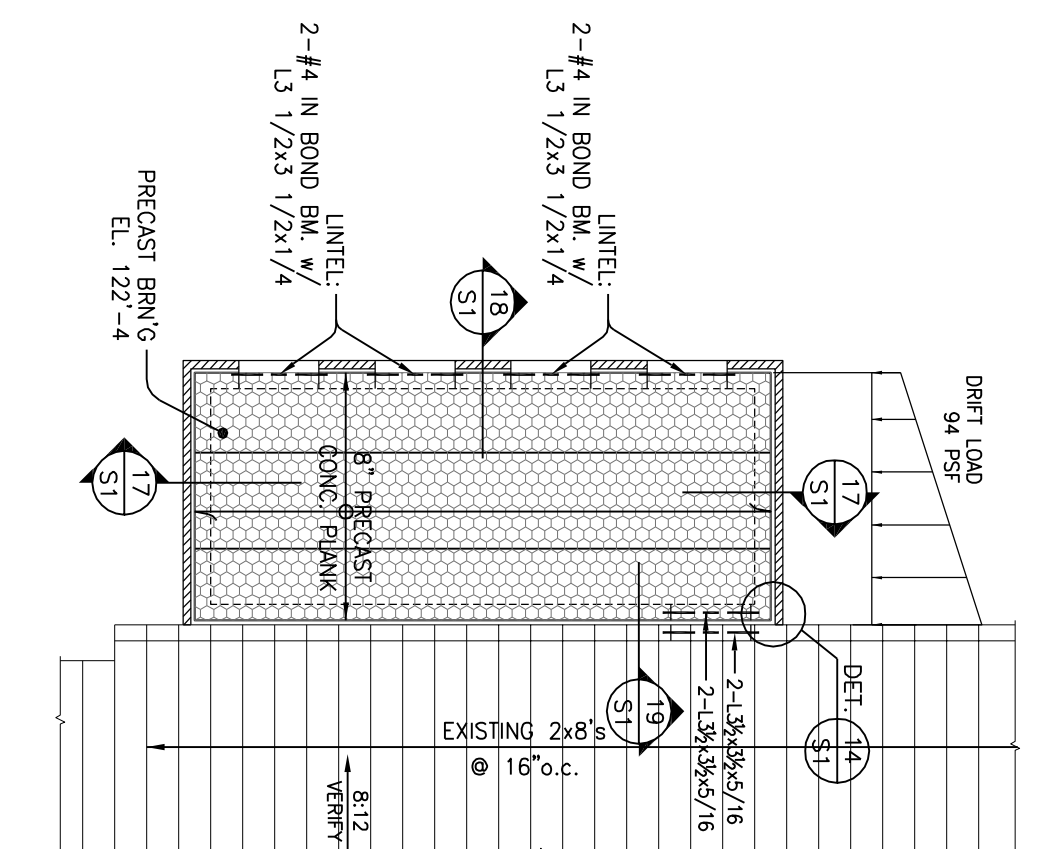
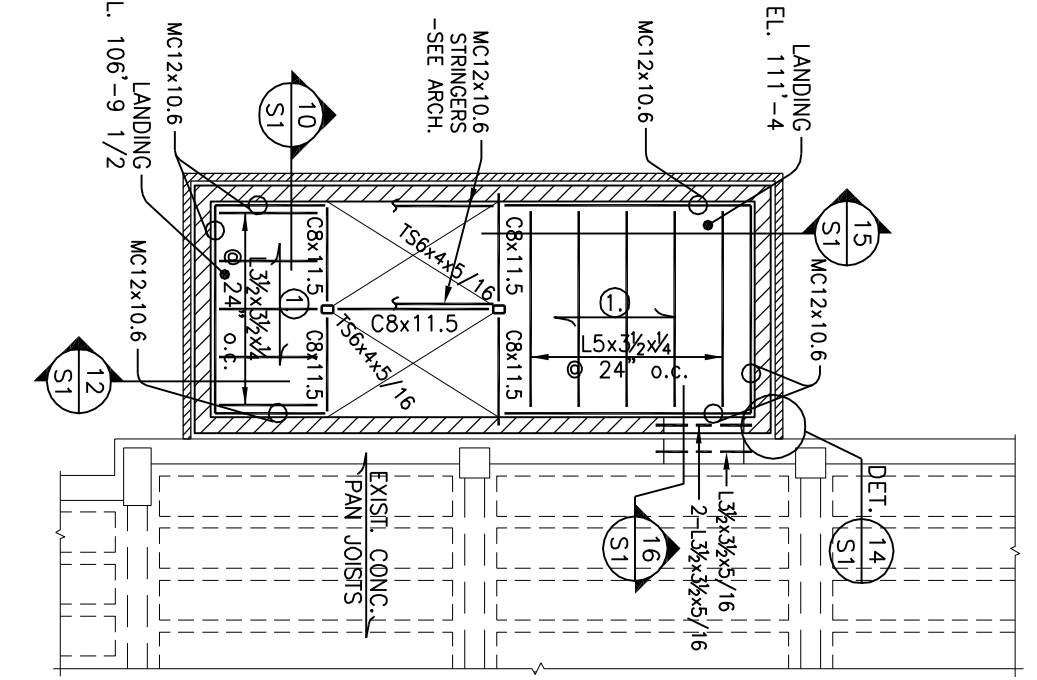
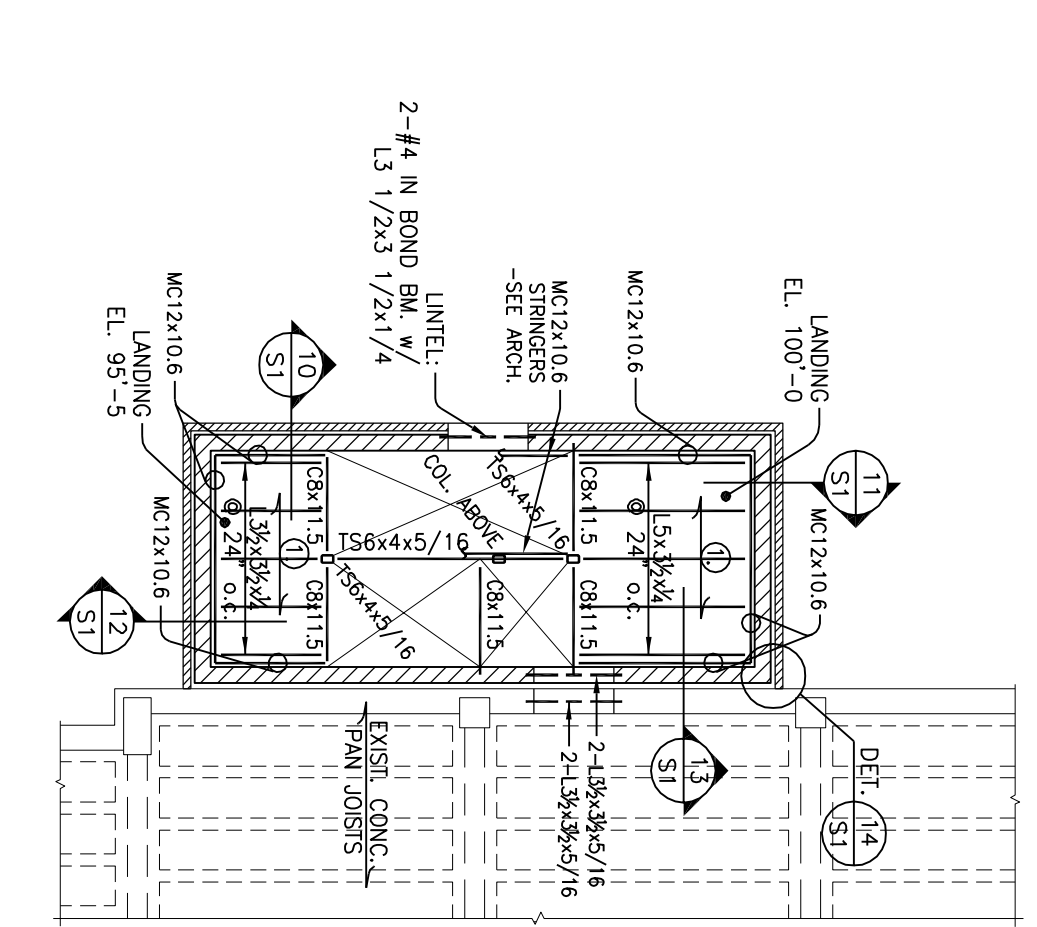
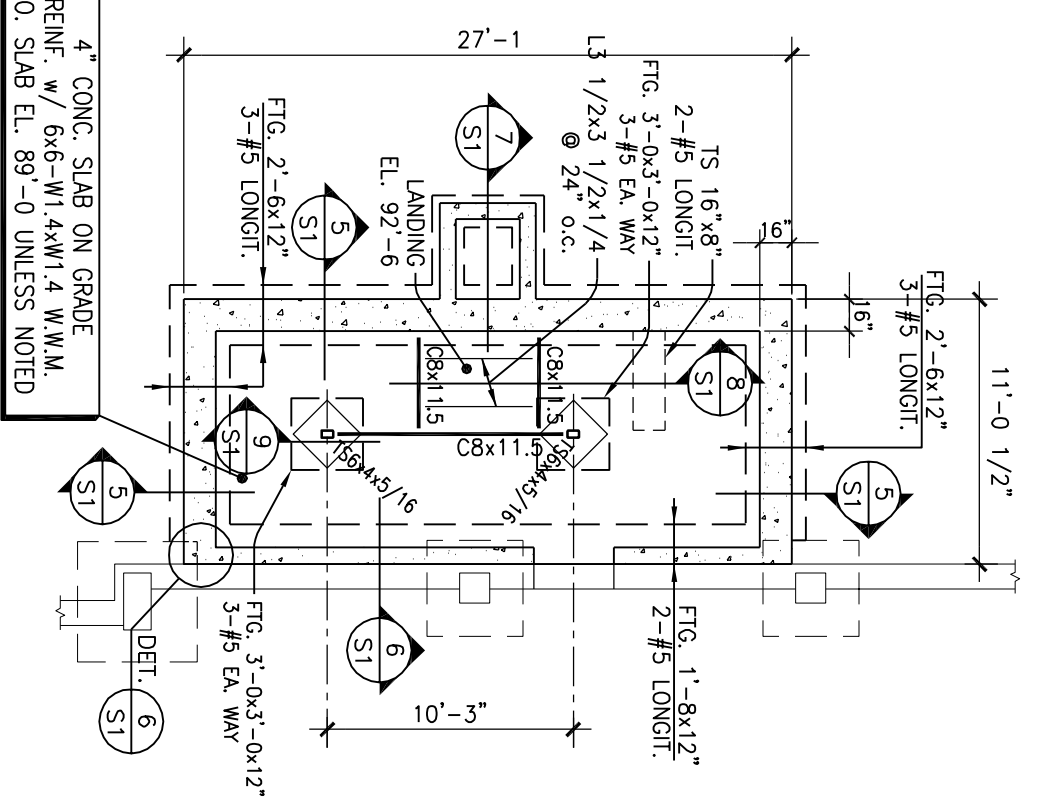
## SECTION

3  
S1

DETAIL  
1/2" = 1'-0"

SECTION  
1/2" = 1'-0"

SECTION  
SCALE: 1/2" = 1'-0"



# FOUNDATION PLAN

# 1st FLOOR FRAMING PLAN

## 2nd FLOOR FRAMING PLAN

# ROOF FRAMING PLAN

[illegible]

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision, and that I am a duly Licensed Professional Engineer under the laws of the state of Minnesota.

Robert P. Dura  
Date 2/21/2013 Lic. No. 24625

Lic. No. 24625